

تقدم لجنة EICoM الاكاديمية

تلخيص لمادة

تصميم ميكانيكي

جزيل الشكر للطالبة:

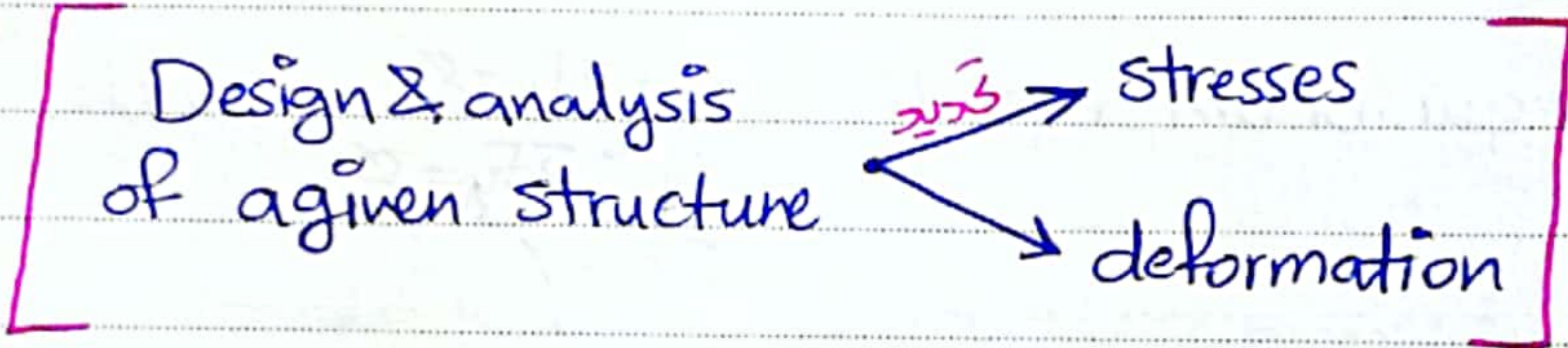
سارة أبو سارة



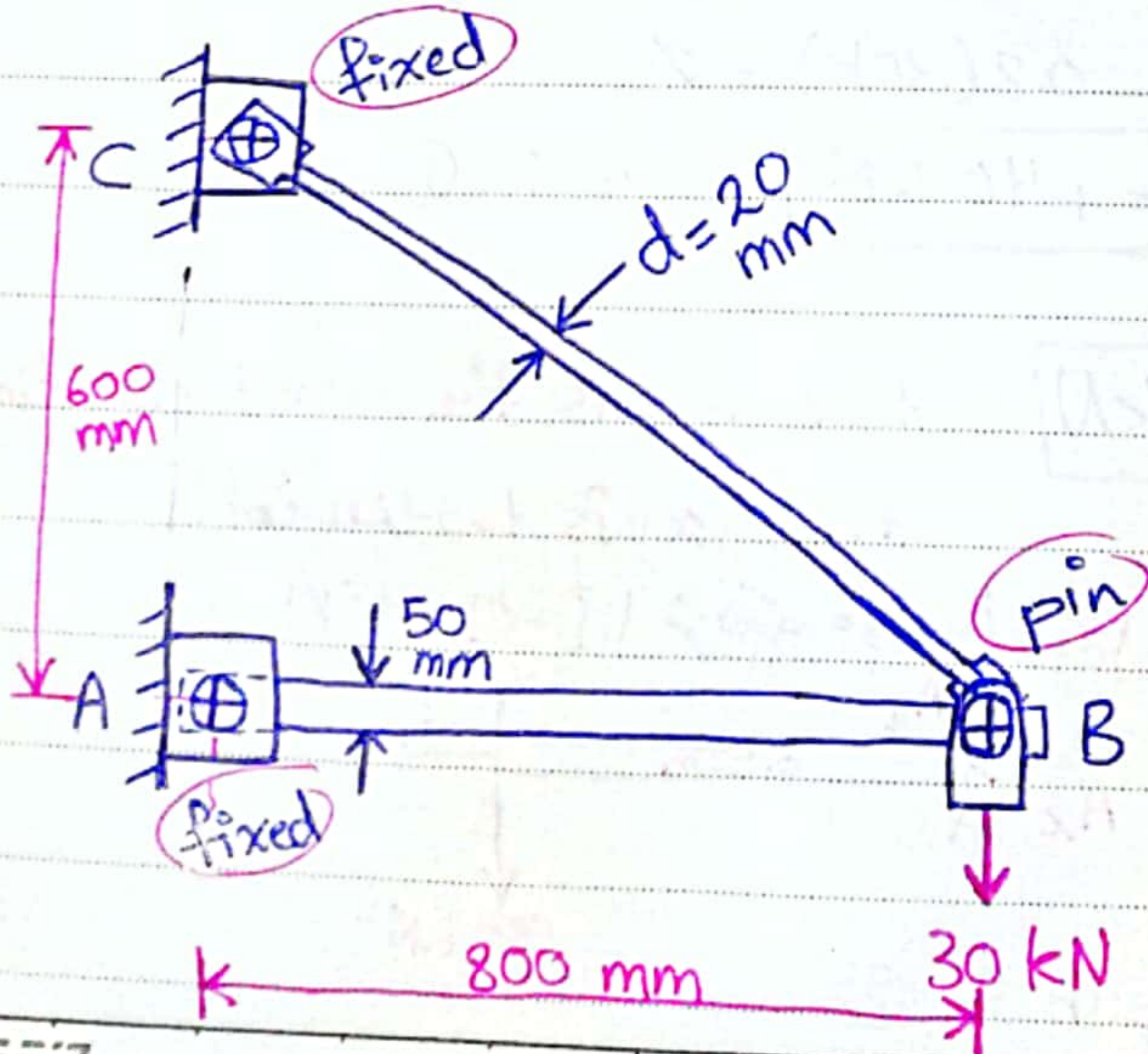
Ch.1 Concept of Stress

1.1 Introduction ∞

The main objective of the study of the mechanics is to provide the future engineer with the means of analysing and designing various machines and load-bearing structures.

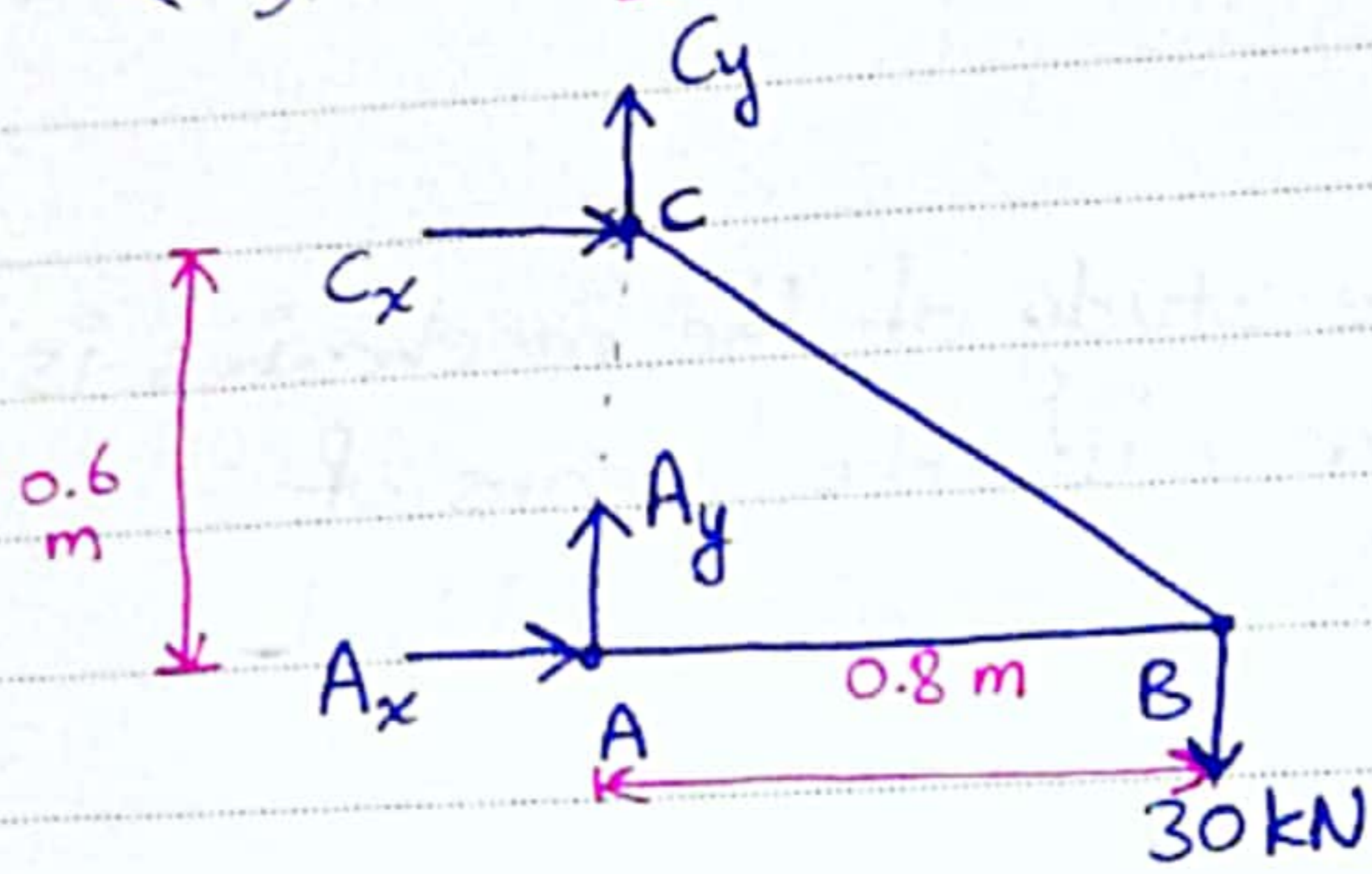


1.2 A Short Review of the methods of the Static ∞
review the basic methods of statics while determining the forces in the members of a simple structure.

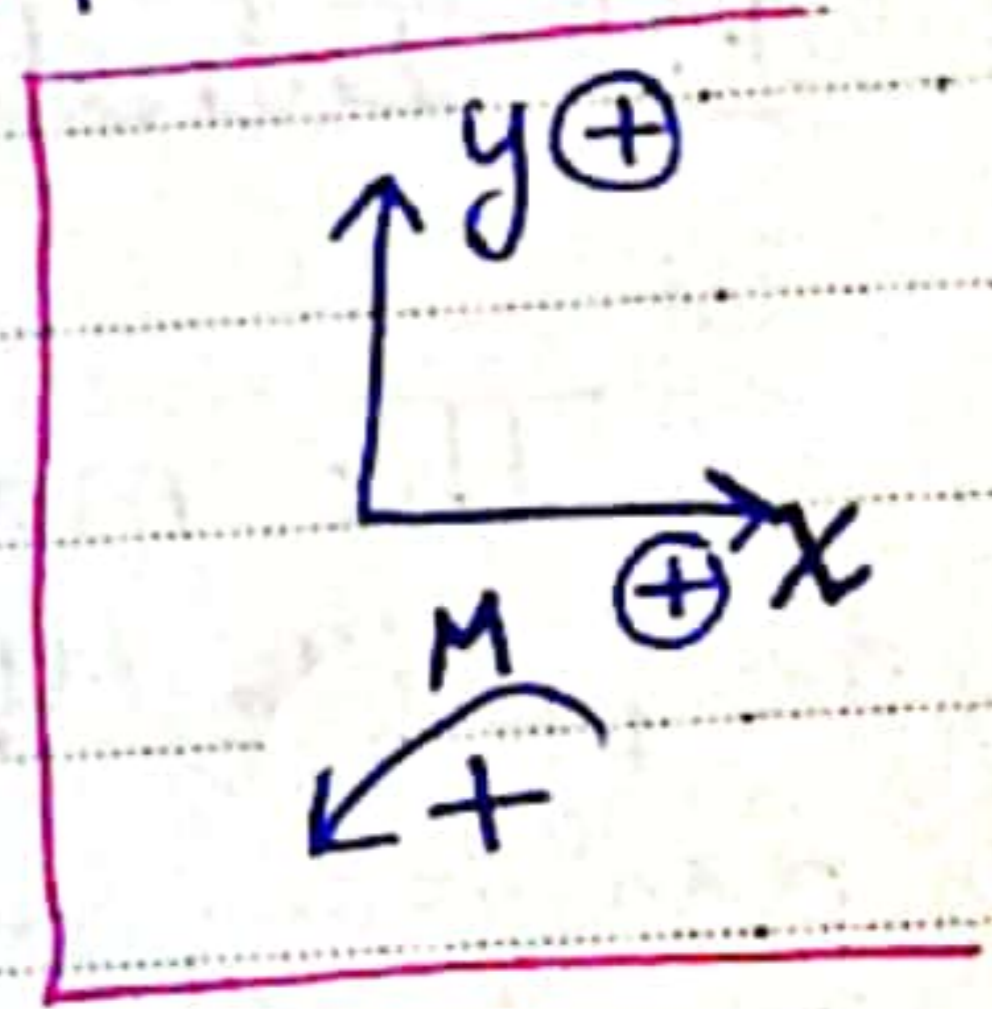


- * Boom (AB) used to support a 30 kN load that has a rectangular cross sectional area.
- * Rod (CB) has a circular cross sectional area..

* كمان دة (analysis) لل (structure) السابقة
 بتايسر FBD توضع من قبل ال [Reaction Forces] ال Static structure



4 unknowns



Using the equilibrium equations

$$\begin{aligned} \sum F_x &= 0 \\ \sum F_y &= 0 \\ \sum M &= 0 \end{aligned}$$

• $\sum F_x = 0 \Rightarrow C_x + A_x = 0 \dots (1)$

• $\sum F_y = 0 \Rightarrow C_y + A_y - 30k = 0 \dots (2)$

• $\sum M_C = 0 \Rightarrow A_x(0.6) - 0.8(30k) = 0$
 $A_x = +40 \text{ kN}$ sub in (1)

افضل طريقة
 الحركية عند
 حساب المومنت

$C_x = -40 \text{ kN}$, that means the correct direction for C_x is to the left

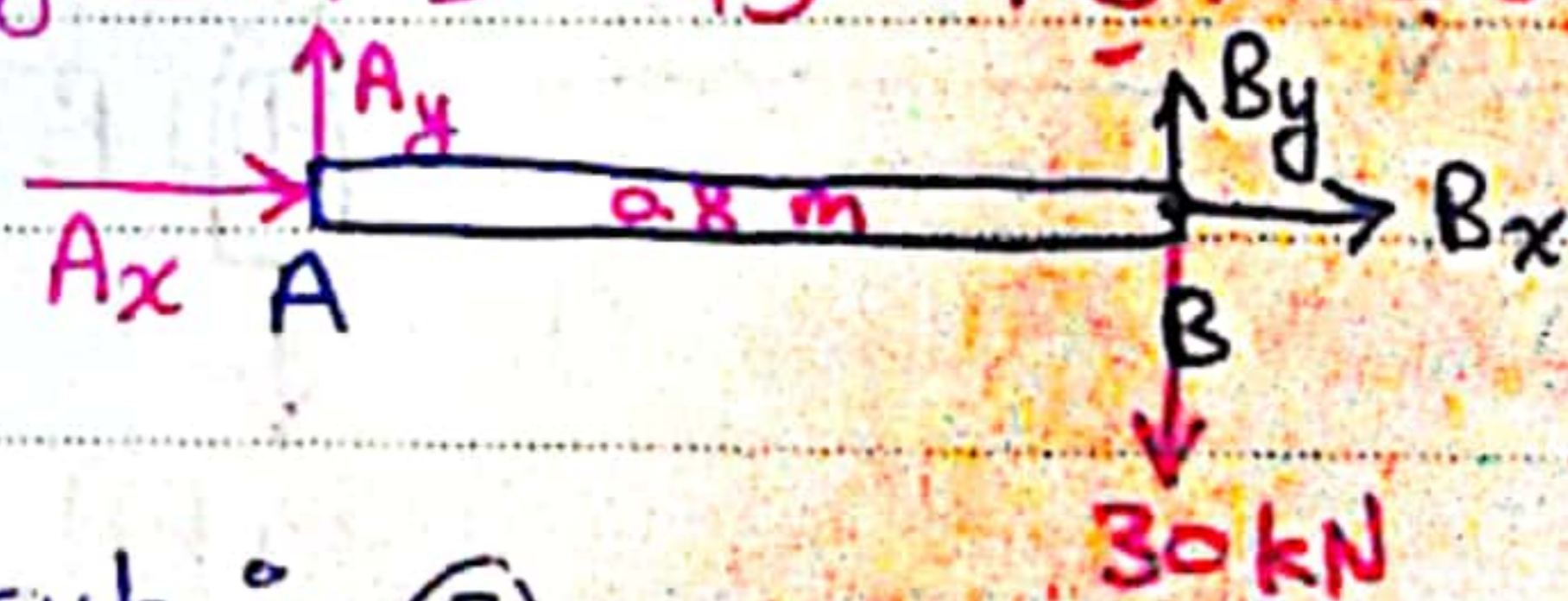
سبب الالاته الي اننا نرضيه فوق بال شكل

• $\sum M_B = 0$

$-A_y(0.8) = 0$

$A_y = 0 \text{ kN}$ sub in (2)

$C_y = 30 \text{ kN}$

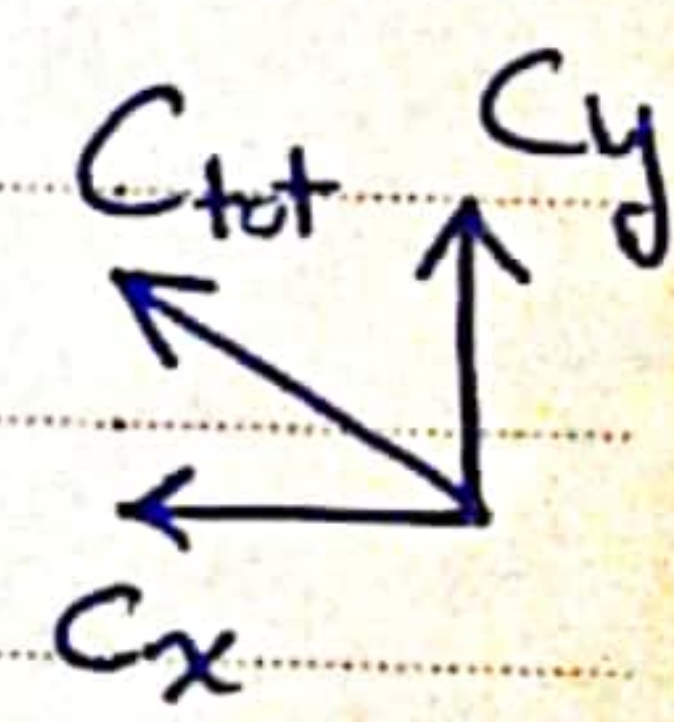


Remember \Rightarrow النقطه التي باقى عندها المومنته ما بيكون في عومنته
 للقوى الممثله فيها ، زي عند النقطه B
 القوى على B ما فاضدهم بجنب الاعتبار لما حسب عومنته المومنته

عند B

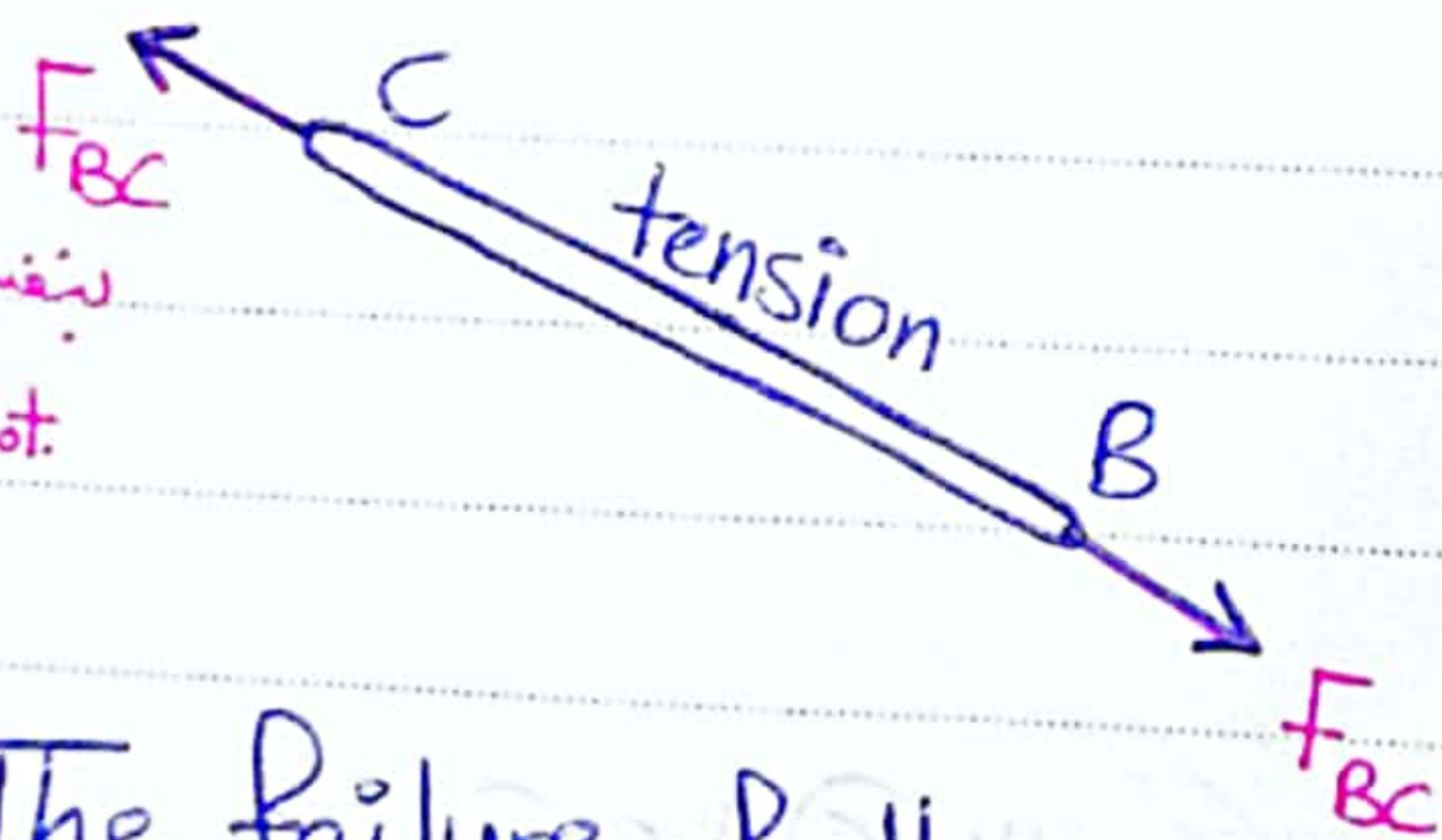
* بفصل كل member كانه وبتقل عليه للايجاد الجاهل
 والاتجاه اللي بفرضه على AB للقوى لازم تكون عكسها لما باقى عند CB
 عند النقطه B

* النقطه B هي عبارة عن (joint) بتبسط بين [2 members] فبفرضه عند النقطه pin

$$C_{tot} = \sqrt{C_x^2 + C_y^2} = \sqrt{(-40)^2 + (30)^2} = 50 \text{ kN} = F_{BC}$$




$$F_{AB} = A_x = 40 \text{ kN}$$



$$F_{BC} = C_{tot} = 50 \text{ kN}$$

* The failure of the system (depends on)

- \rightarrow Load
- \rightarrow Material
- \rightarrow Cross Sectional Area

1.3 | Stress in the members of structure ∞

The structure can be safely supported depends on →

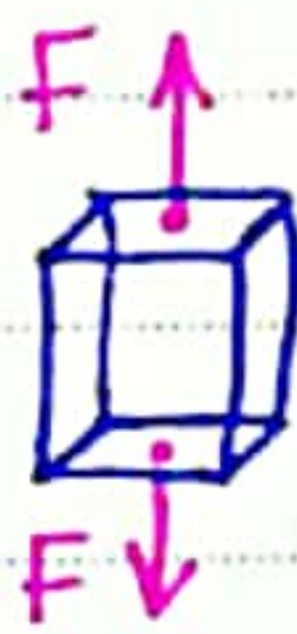
- 1- Internal Force
- 2- Cross Sectional Area
- 3- The material of which the rod is made from.

∴ Stress ≡ the force per unit area or intensity of the forces distributed over a given section.

$$\sigma = \frac{F}{A}$$

$$[\text{Pascal}] = \frac{[N]}{[m^2]}$$

the force → tension
 ↘ compression



$\sigma (+ve)$



$\sigma (-ve)$

Recall) $1 \text{ kPa} = 10^3 \text{ Pa}$

$1 \text{ MPa} = 10^6 \text{ Pa}$

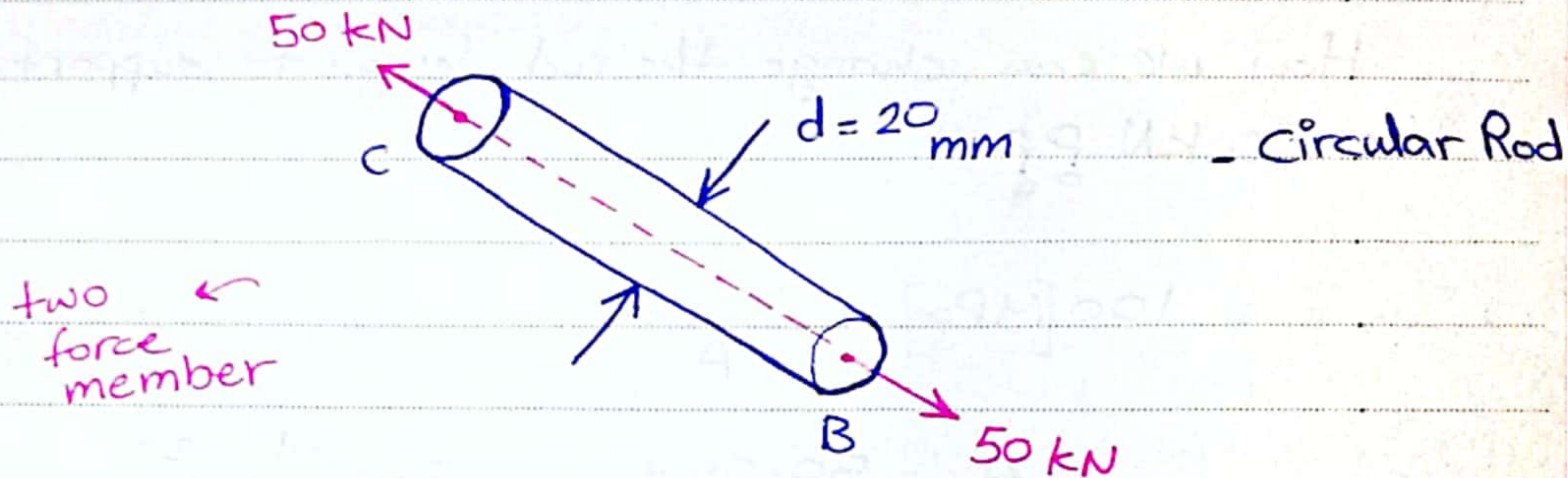
$1 \text{ GPa} = 10^9 \text{ Pa}$

عقبة الـ σ التي بوجدها (Normal stress)

$$\sigma < \sigma_{\text{allowed}}$$

1.4 | Analysis and Design

According to the previous simple structure, assume that rod (BC) is made of a steel with a max. allowable stress $[\sigma_{all} = 165 \text{ MPa}]$. Knowing that $F_{BC} = 50 \text{ kN}$



* أنا بعرف انه هاد ال (rod) يتعمل (stress) لغاية 165 MPa
فلحن اتمد اذا هو بيقدر يتعمل القوة 50 kN ولا لا، كجانب انا بسبب
قيمة ال [stress] الناتجة عن هال 50 kN المؤثرة على ال (BC Rod)!

$$\sigma = \frac{50 \text{ kN}}{\pi (0.1)^2} = +159 \text{ MPa} < \underline{165 \text{ MPa}}$$

↓
تension force

∴ اذن بيقدر ال rod يتعمل هاي القوة المؤثرة عليه $[50 \text{ kN}]$

⇒ At the same previous value of the force (50 kN),
إذا عطينا الـ [material] الخاصة بالـ "BC rod"
من (steel) لنكون حسب أمان

$$\sigma_{\text{allow}} = 100 \text{ MPa}$$

How we can change the rod design to support
the 50 kN ???

$$100 \text{ [MPa]} = \frac{50 \text{ [kN]}}{A}$$

$$A = \frac{50,000}{100 \times 10^6} = 5 \times 10^{-4} \text{ [m}^2\text{]}$$

$$5 \times 10^{-4} = \pi r^2 \Rightarrow r = 12.62 \text{ mm}$$

روح أسترى القطعة بنبدأ

$$[d = 25.2 \text{ mm}]$$

على طول 26 mm لأنه

حارج أقدار الألياف بـ (25.2) وما يقدر أقتدال 25 مبردار كمن المصنع به . .

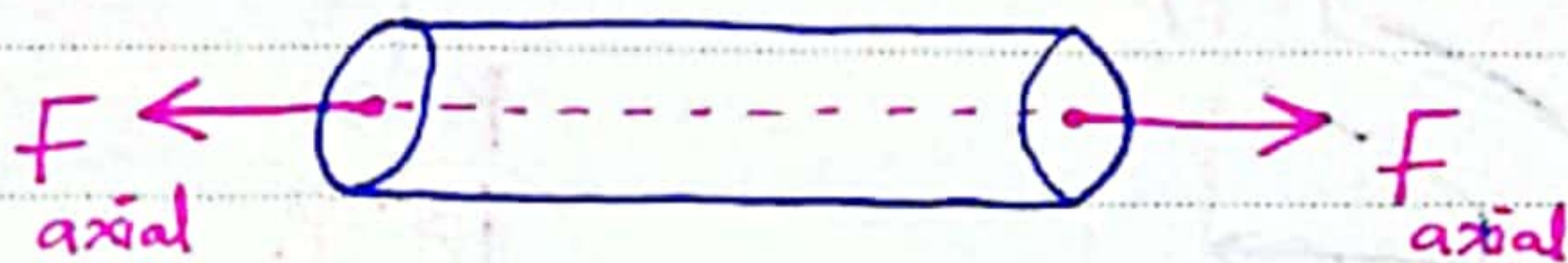
∴ we need to increase the rod diameter

26 mm or more

لأنه إذا كان أقل من ذلك سيكون (σ) أكثر من [100 MPa]

1.5 | Axial loading ; Normal Stress ∴

Axial loading → When the force directed along the (F_{axial}) axis of the rod

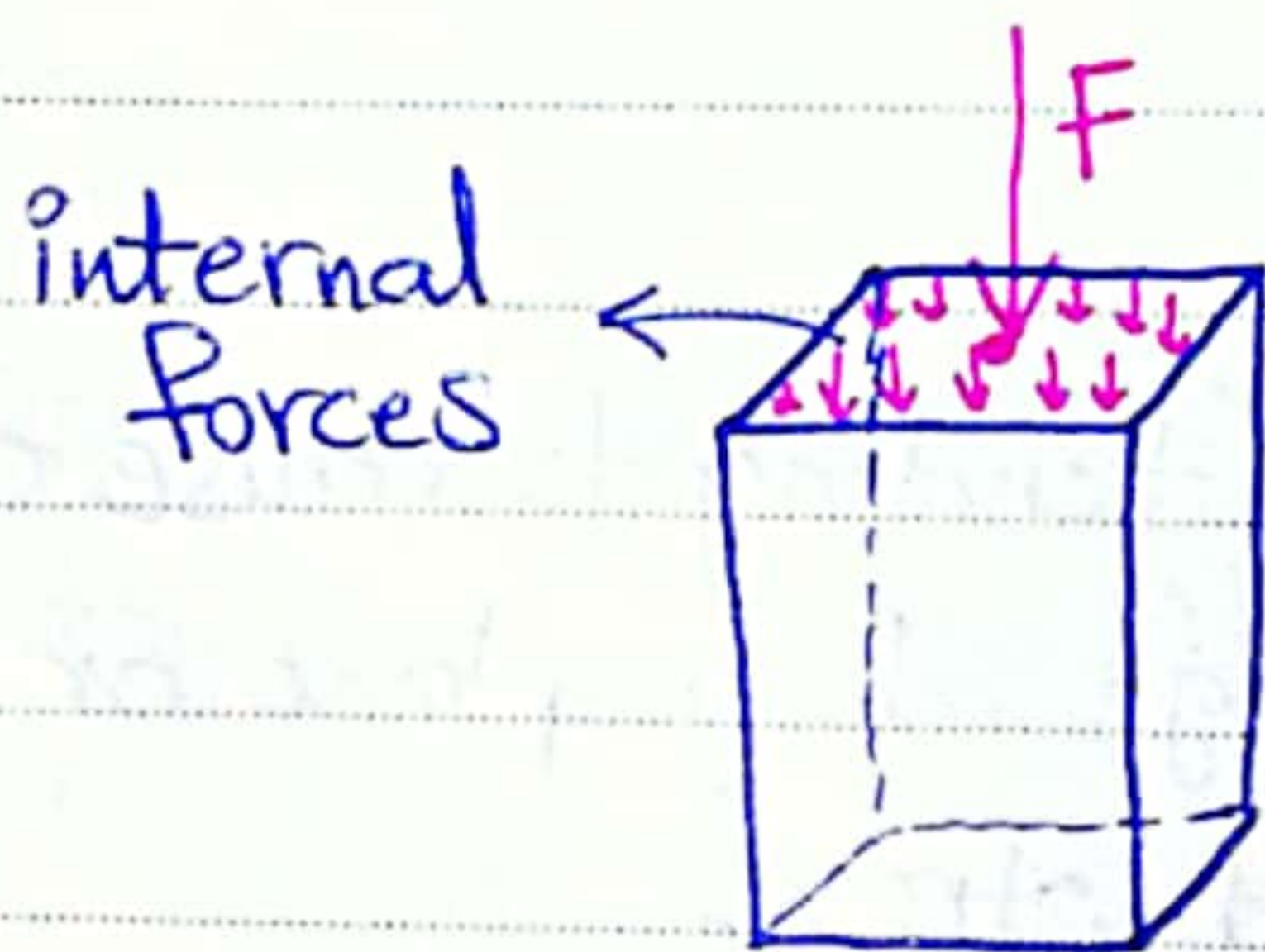


Normal stress ≡ occurs when a member is loaded by an axial force

$$\sigma_{\text{normal}} = \frac{F_{\text{axial}}}{A}$$

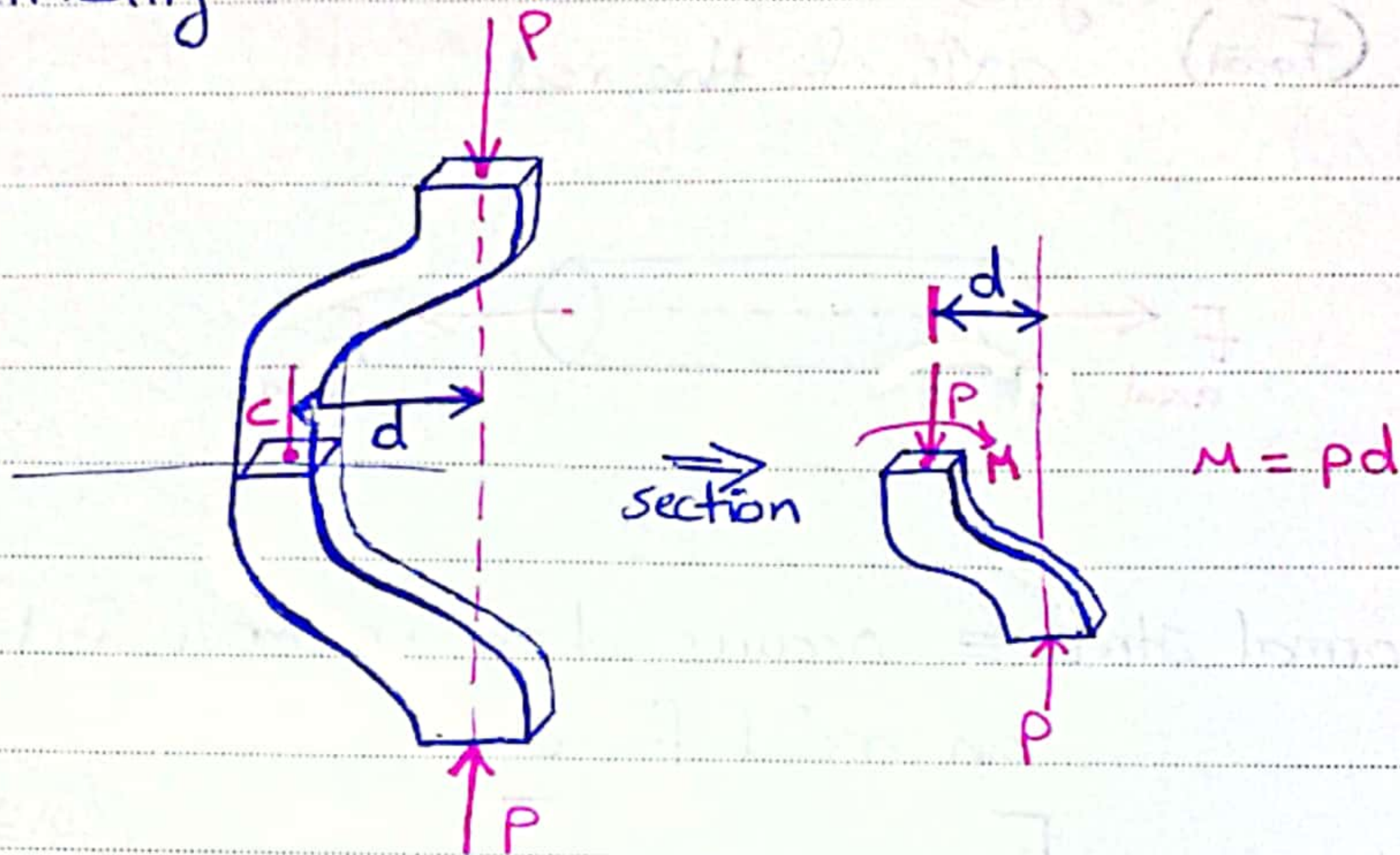
زِيَّ الِّي عَلَى ال
BC Rod

وَبِرَأْيِهَا تَبْكَونَ القُوَّةَ مَوْزَعَةً لِسُكُونِ سِتْرَمِ عَلَى الِ area



∴ a uniform distribution of stress is possible only if the line of action of the concentrated load passes through the centroid of the section considered (centric loading)

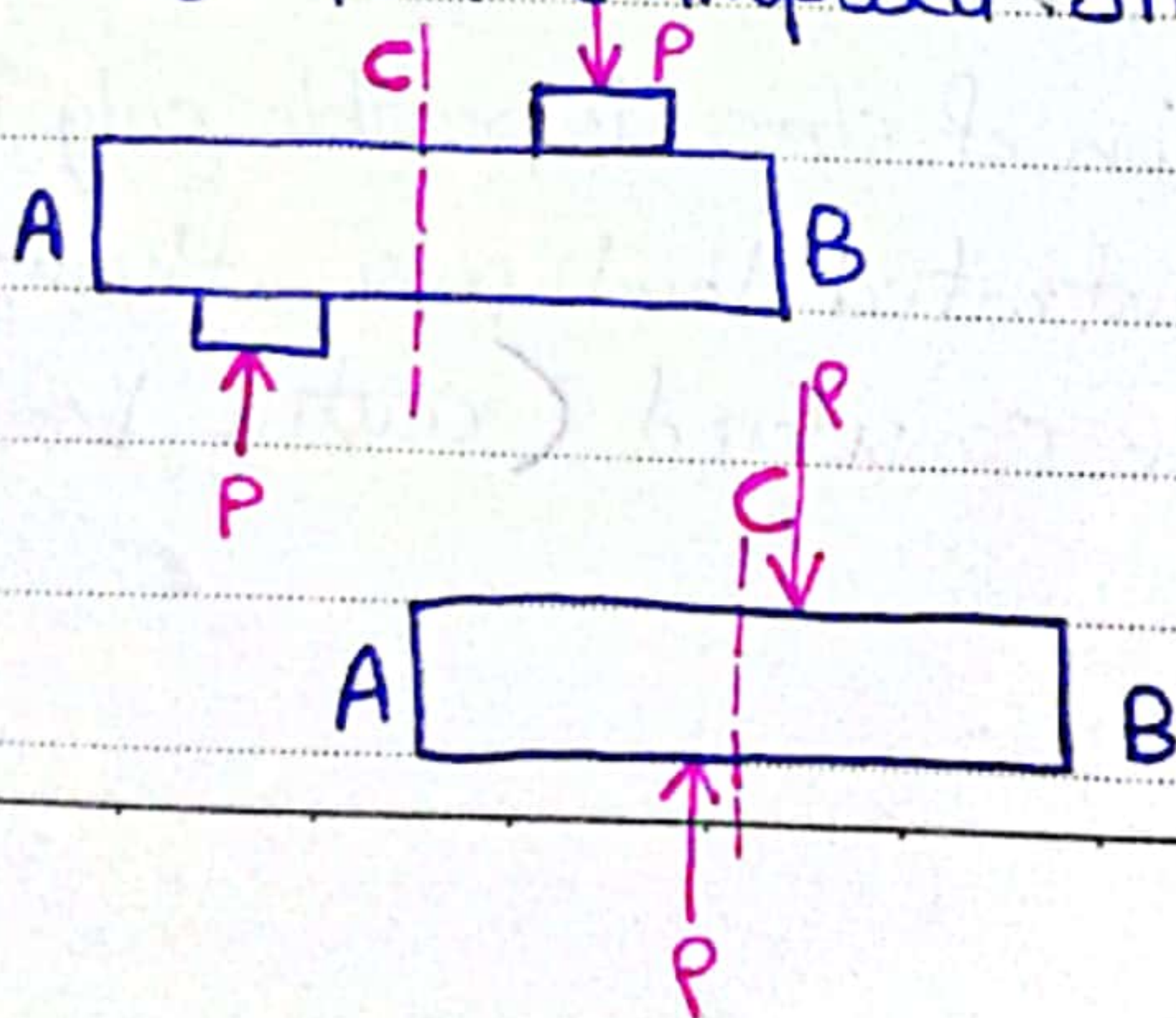
However, if a two force member is loaded axially, but eccentrically



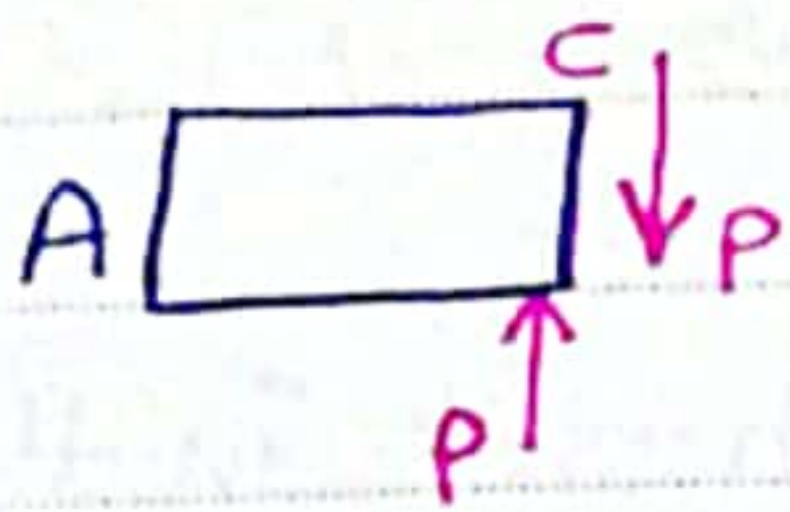
The distribution of forces, thus the corresponding distribution of stresses \Rightarrow [Non-uniform]

1.6] Shearing Stress :

Occured due to a force tending to cause deformation of a material by slippage along plane or planes parallel to the imposed stress.

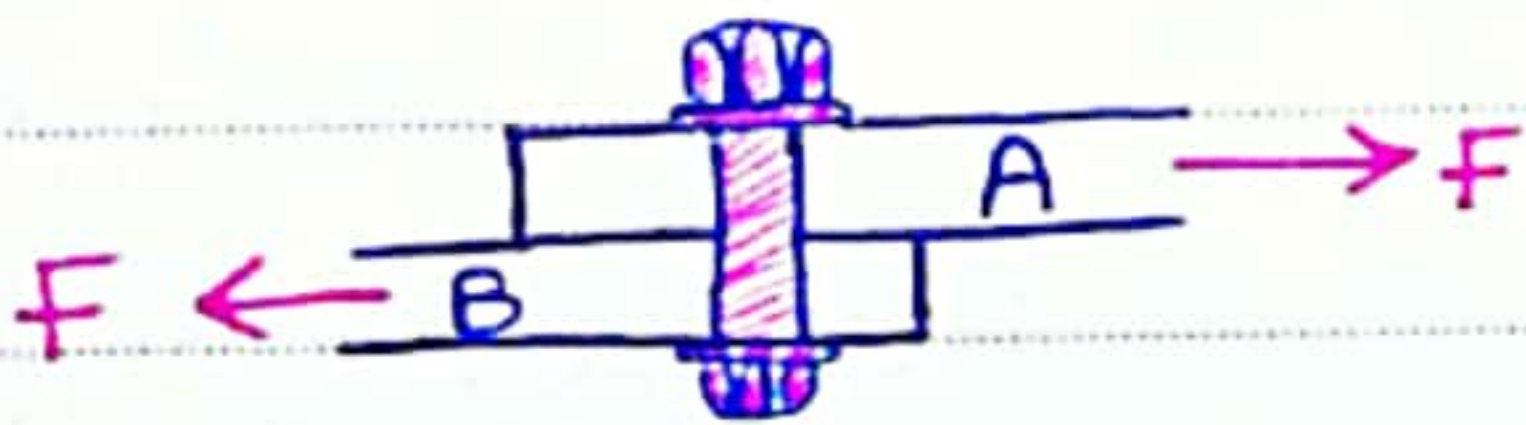


[
 رقبته على ال
 Contact Area
 التي عم تأثر عليها القوة
 والقوة نفسها
]



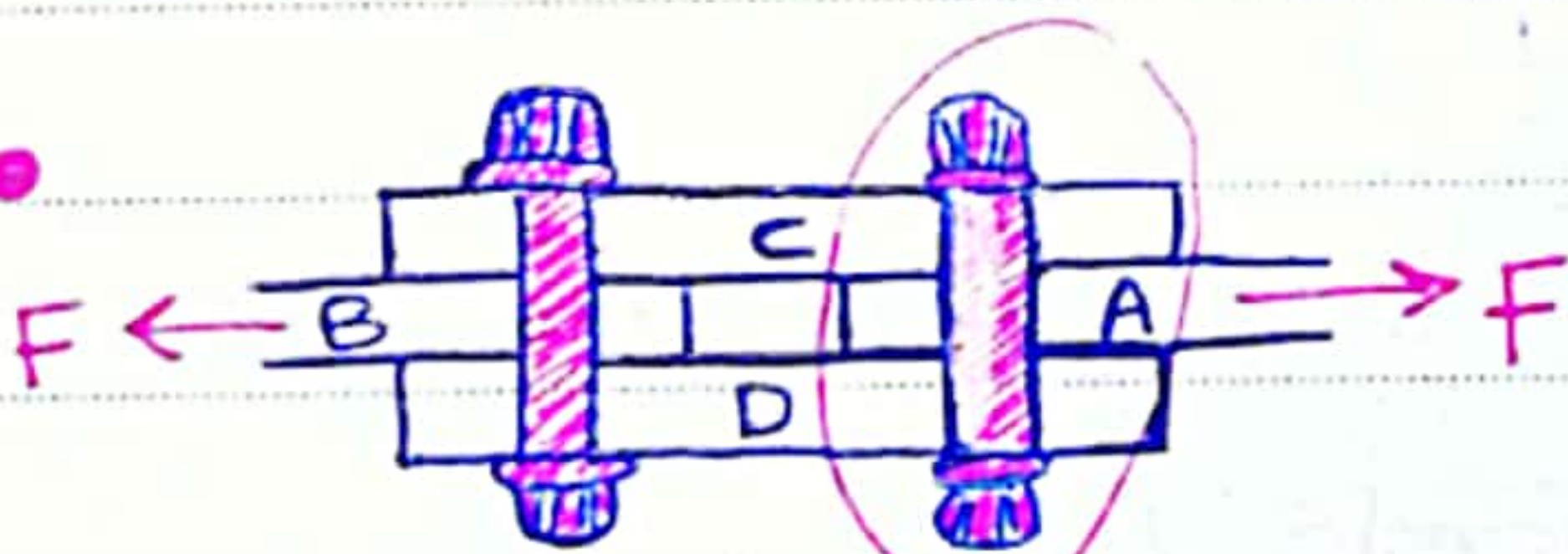
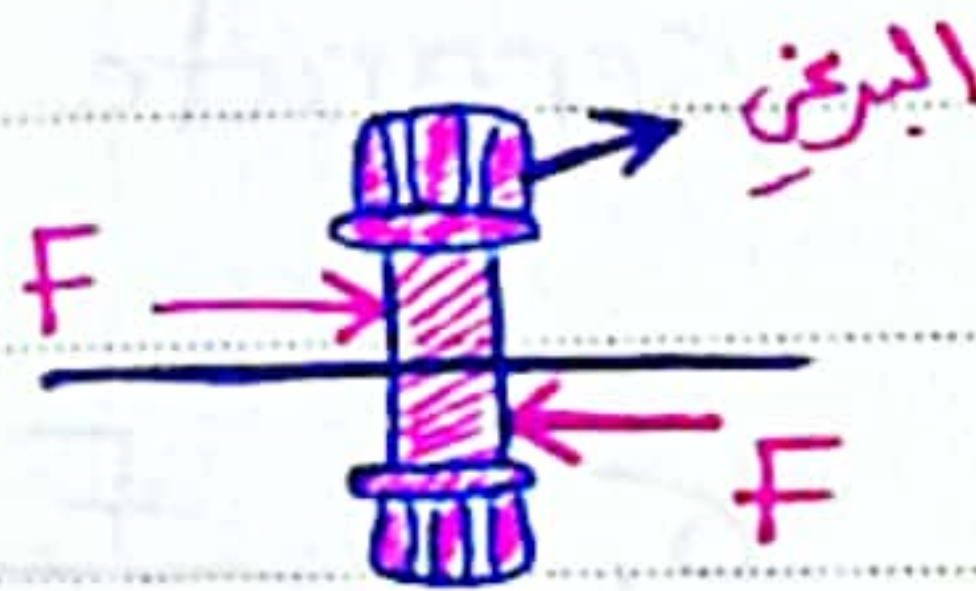
$$\tau = \frac{F}{A}$$

* Shearing stresses are commonly found in bolts, pins, and rivets used to connect various structural members and machine components.



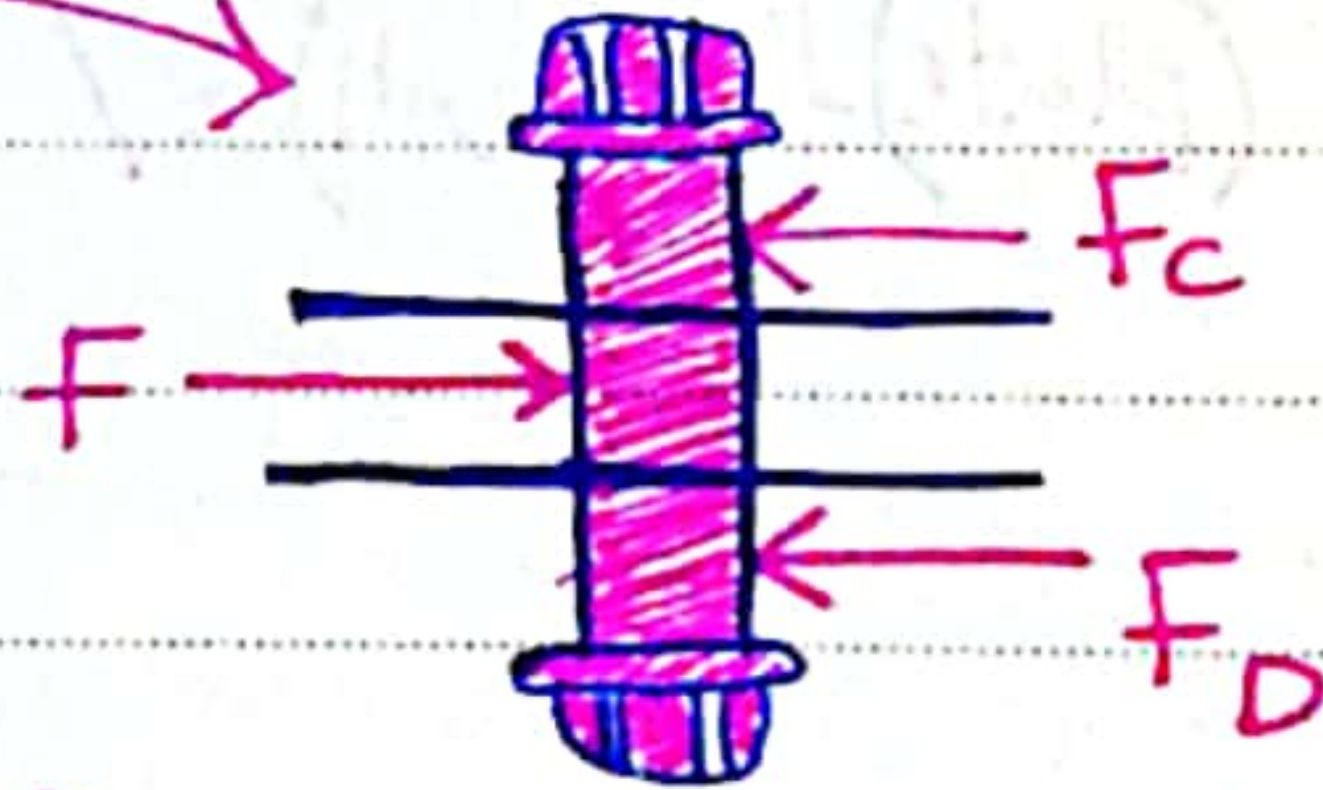
Bolt subject to single shear

$$\tau = \frac{F}{A}$$

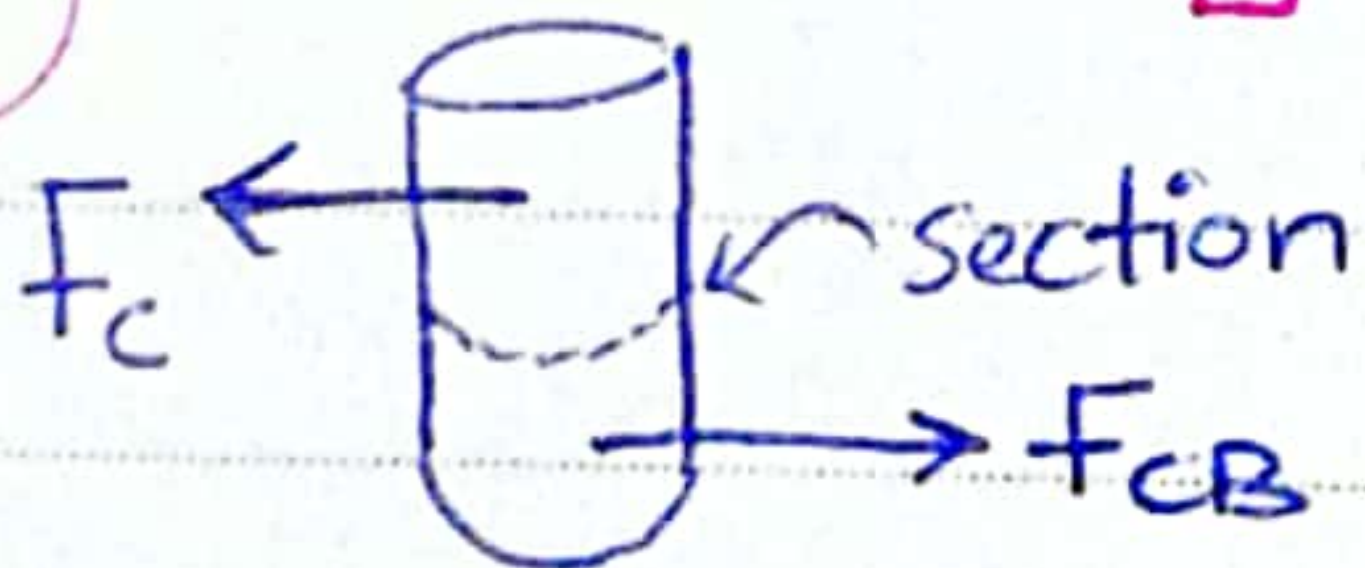


Bolts subject to double shear \Rightarrow [سكتين قسمة]

$$\tau = \frac{F/2}{A} = \frac{F}{2A}$$



* at pin C: $\tau = \frac{F}{A}$ (Shearing surface)
 قسمة السطح (shearing surface)

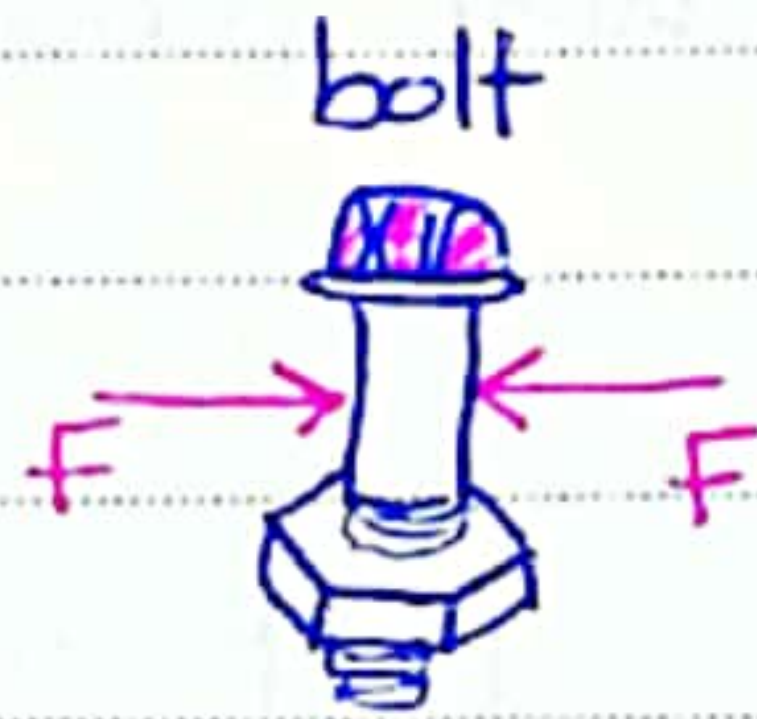
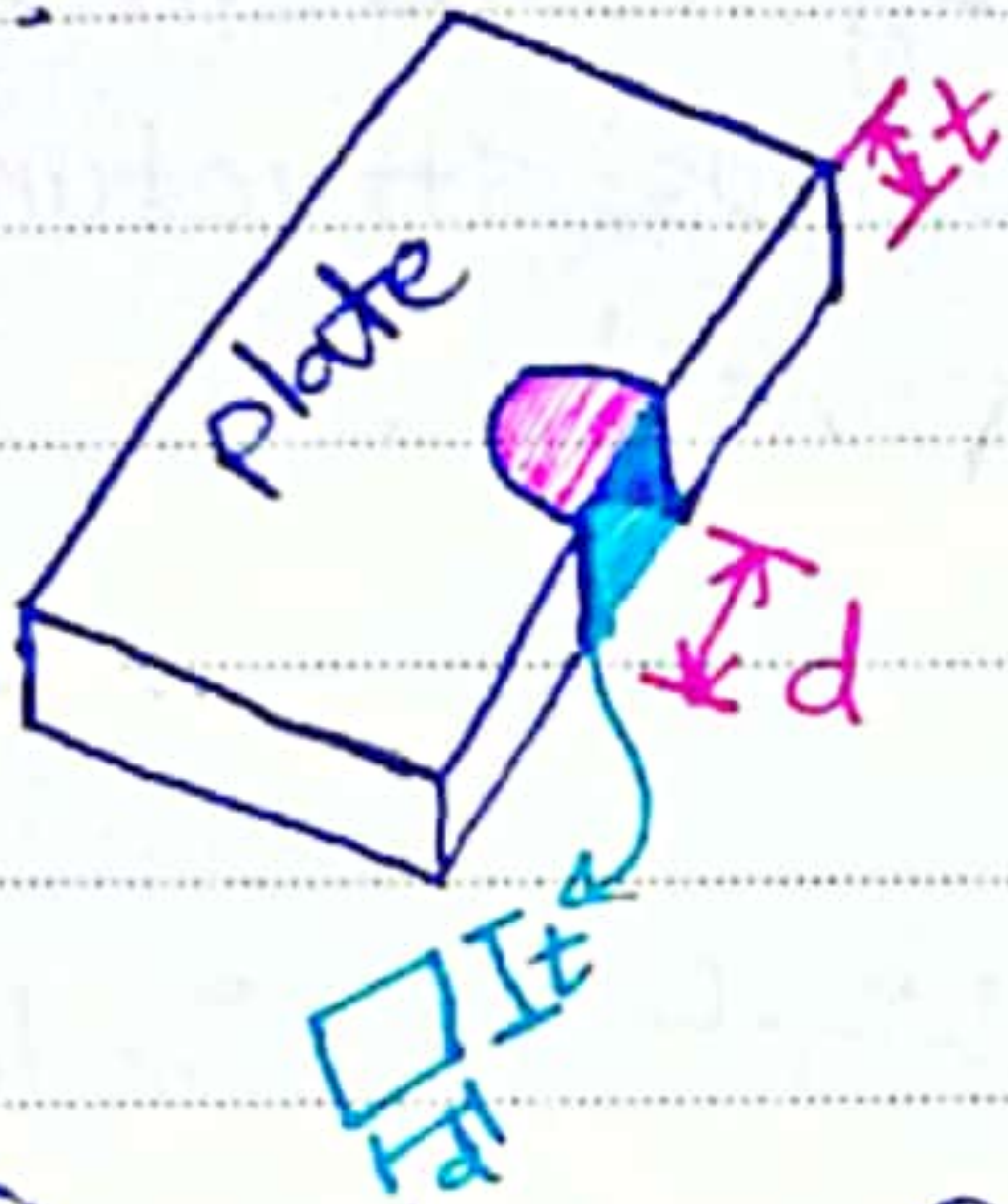


قوى أفقية

بالنصفين

1.7 | Bearing Stress in Connections : "Contact"

Bolts, pins, and rivets create stresses in the members they connect, along the bearing surface, or surface of contact.



t : thickness of plate

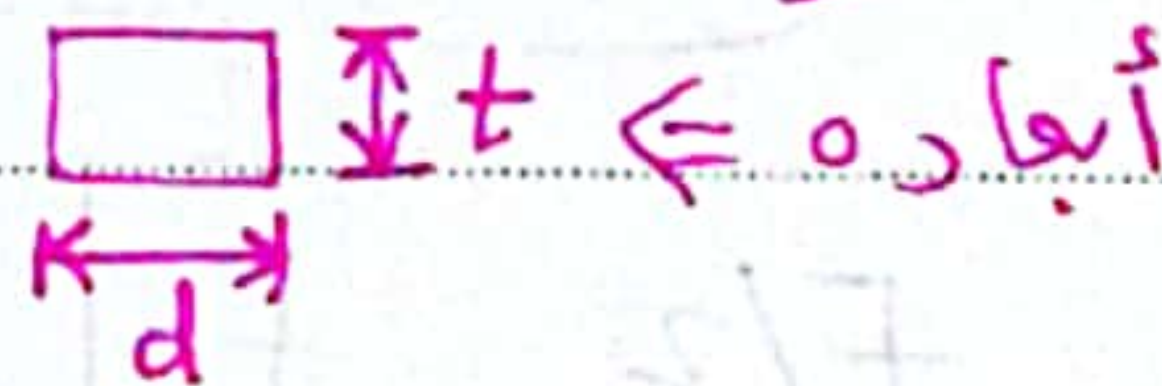
d : bolt diameter

∴ bearing stress → is the contact pressure between the separate bodies.

$$\sigma_b = \frac{F}{A} = \frac{F}{td}$$

contact area
بين (البرغي) وال (plate)

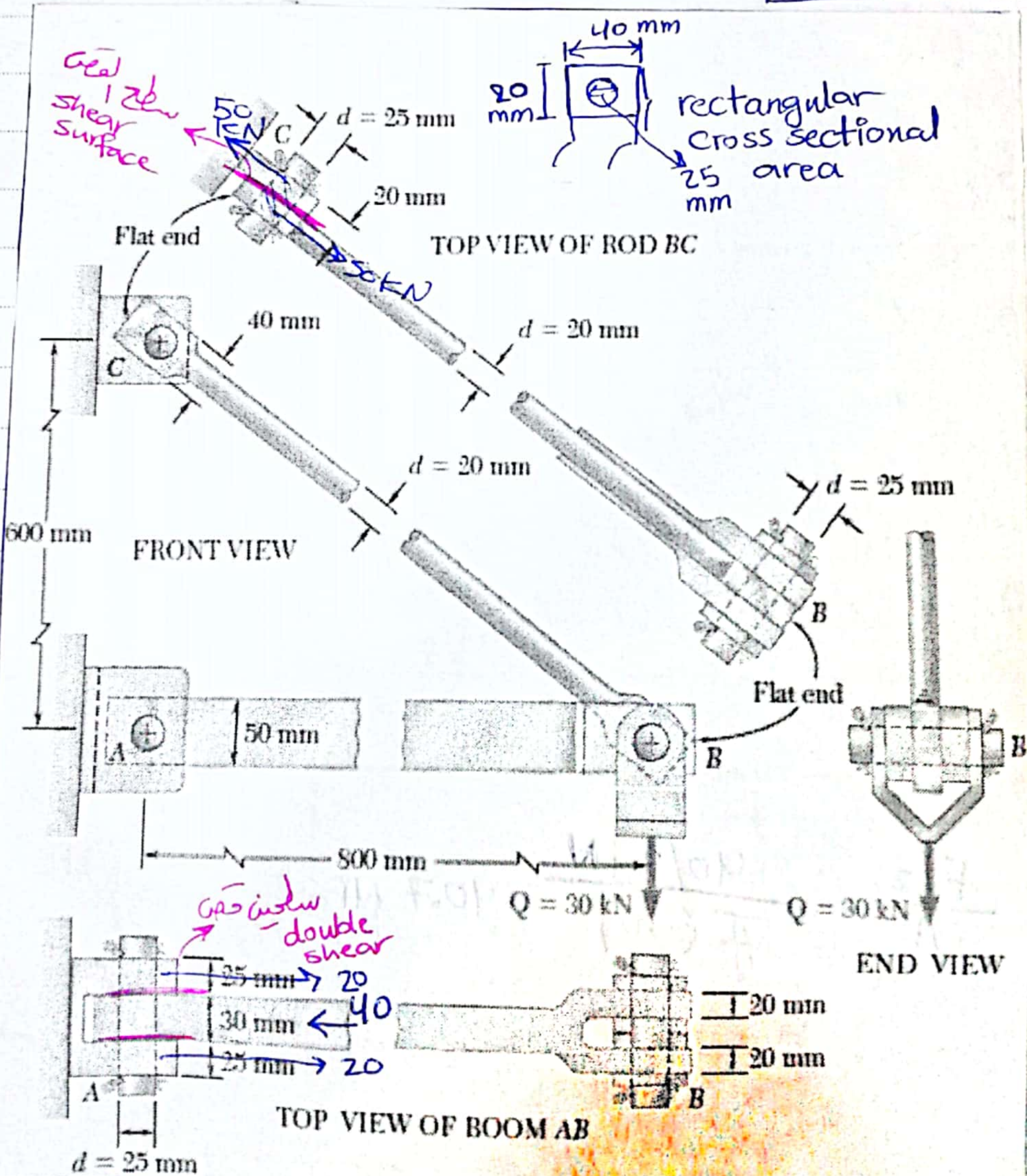
لأنه كما يتطوع من
قوام ملائم الشكل متطوع



1.8] Application to the analysis and design of simple structures :

Ex :

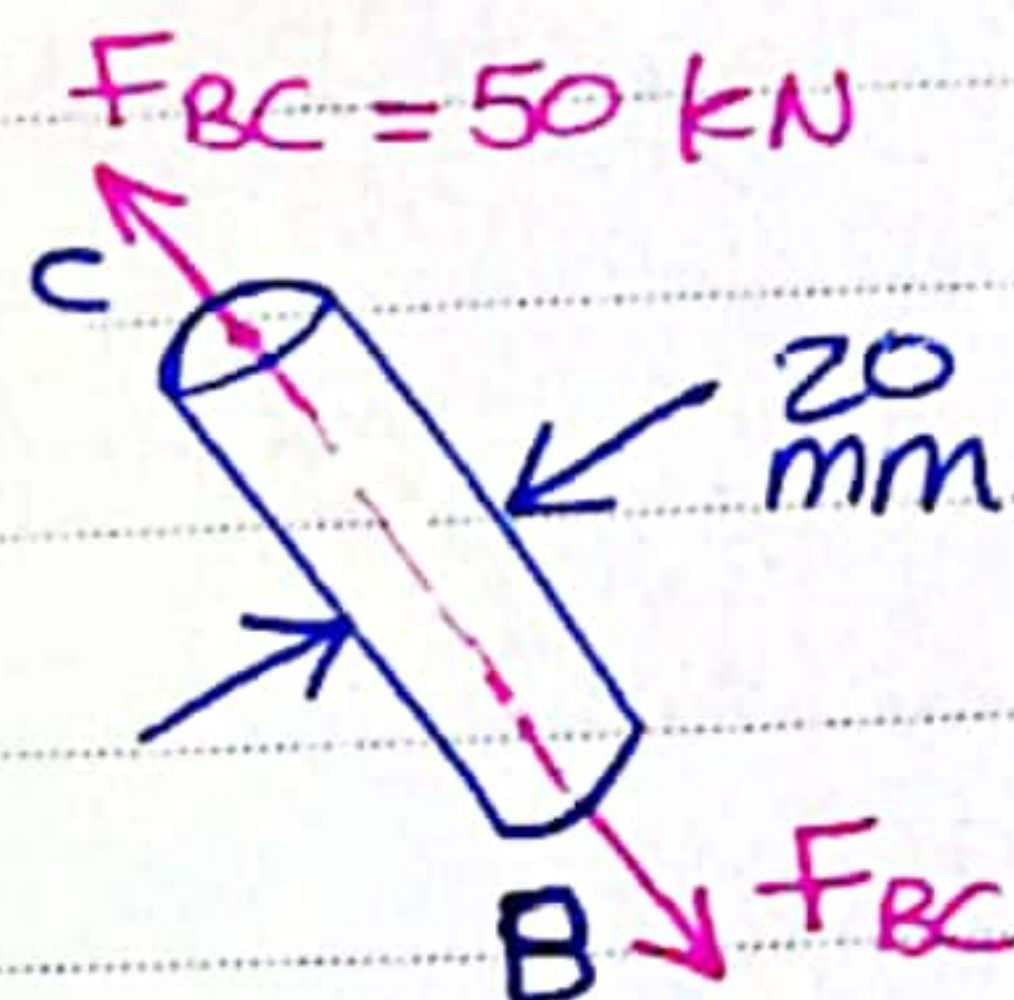
سببش 1.2 اوجها ال
 (Forces)
 ال [Structure] ال
 ال بنا نوجده ال (stresses)



a. Determination of the normal stress in Boom AB and rod BC :

(BC)

$$\sigma_{\text{normal BC}} = \frac{F}{A} = \frac{50 \text{ kN}}{\pi (0.1)^2} = +159 \text{ MPa}$$



also, Flat parts at the end of the Rod BC (at c) are under tension, where a hole is located

$$\text{Area} = \pi r^2$$

للغاشة



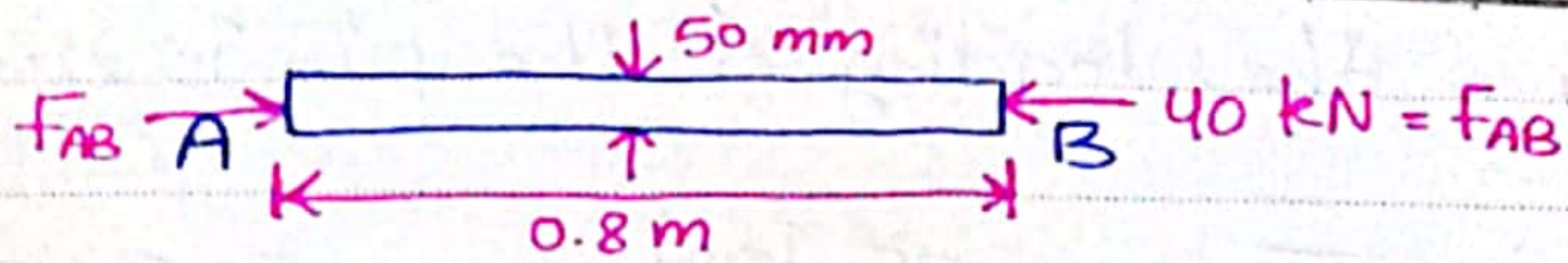
$$\sigma_{\text{normally c}} = \frac{F}{A} = \frac{50,000 \text{ N}}{20 * (40 - 25) * 10^{-6}} = 167 \text{ MPa}$$

* ألقى (stress) يكون عى الأقف مفرد ← the stress will reach a much larger value, close to the hole.

هذا صياح ازدياد اللود ، ال [rod] مفرد على المفردة ، لقريبة من ال holes كويها الأقف

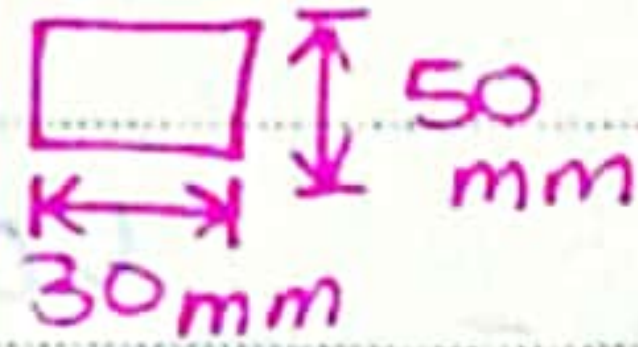
So, in its design therefore, could be improved by increasing the width or the thickness of the flat ends of the rod.

(AB)



$$\sigma_{\text{normally AB}} = \frac{F}{A} = \frac{-40,000 \text{ N}}{30 \times 50 \times 10^{-6}} = -26.7 \text{ MPa} \rightarrow \text{Comp. force pushes on the pins}$$

rectangular cross sectional area

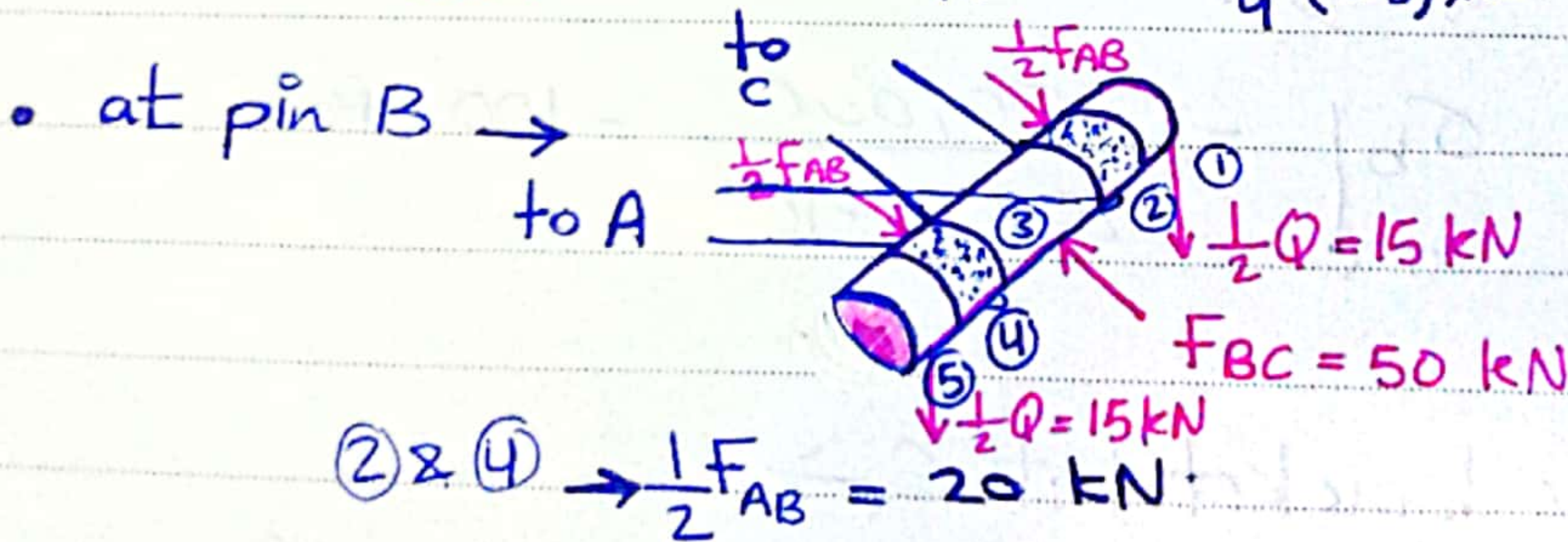


b. Determination of the shearing stress in various Connections :

such as bolts, pins, and rivets

• at pin C → $T_c = \frac{F}{A} = \frac{50,000}{\frac{\pi}{4} (25 \times 10^{-3})^2} = 102 \text{ MPa}$
Single Shear

• at pin A → $T_a = \frac{F/2}{A} = \frac{\frac{40}{2} \text{ kN}}{\frac{\pi}{4} (25)^2 \times 10^{-6}} = 40.7 \text{ MPa}$
double shear



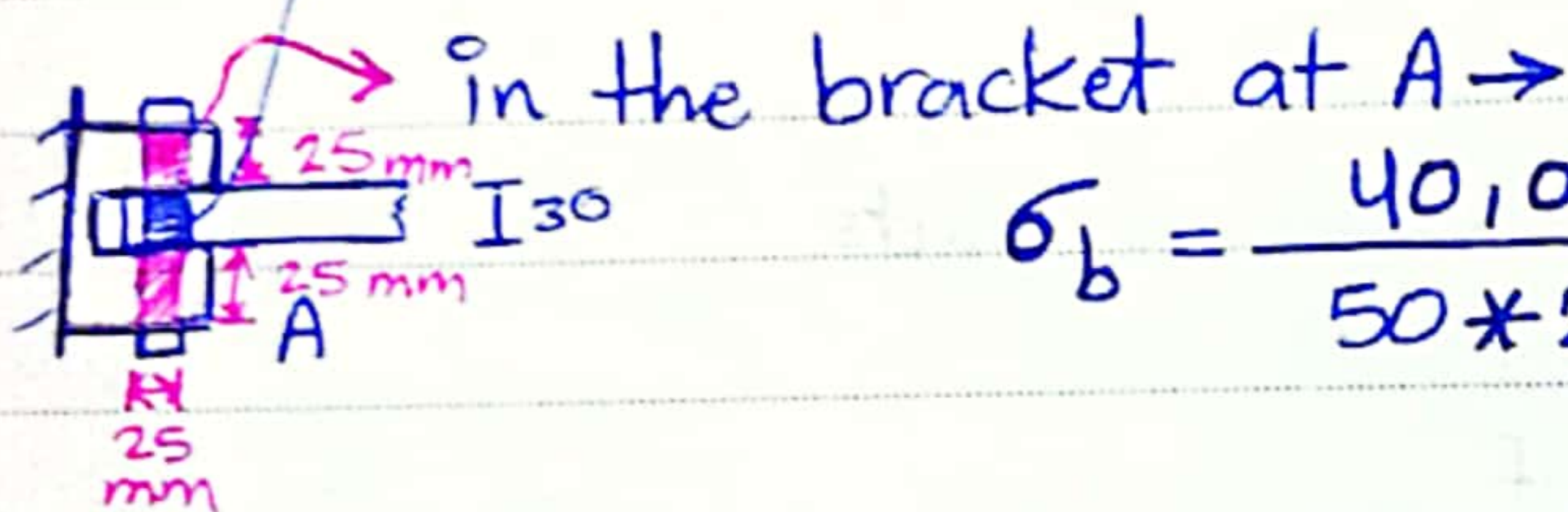
الأكبر [shear stress] في ال (pin B) هو عند المقطع في المنتصف
 $F = 25 \text{ kN}$

Since the loading of the pin is symmetric

$$\tau_B = \frac{25 \text{ kN}}{\frac{\pi}{4} (25 \times 10^{-3})^2} = 50.9 \text{ MPa}$$

C. Determination of the Bearing stresses :

• at A : $\sigma_b \Big|_{\text{at A}} = \frac{P}{td} = \frac{40,000 \text{ N}}{30 \times 25 \times 10^{-6}} = 53.3 \text{ MPa}$
on the rod



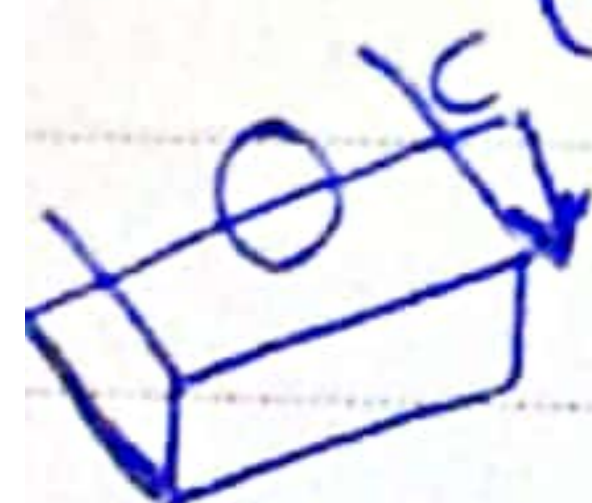
in the bracket at A →

$$\sigma_b = \frac{40,000 \text{ N}}{50 \times 25 \times 10^{-6}} = 32 \text{ MPa}$$

• at B : $\sigma_b \Big|_B = \frac{40,000}{40 \times 25 \times 10^{-6}} = 40 \text{ MPa}$
in the member (AB)

في المرفق
العرض 20 mm

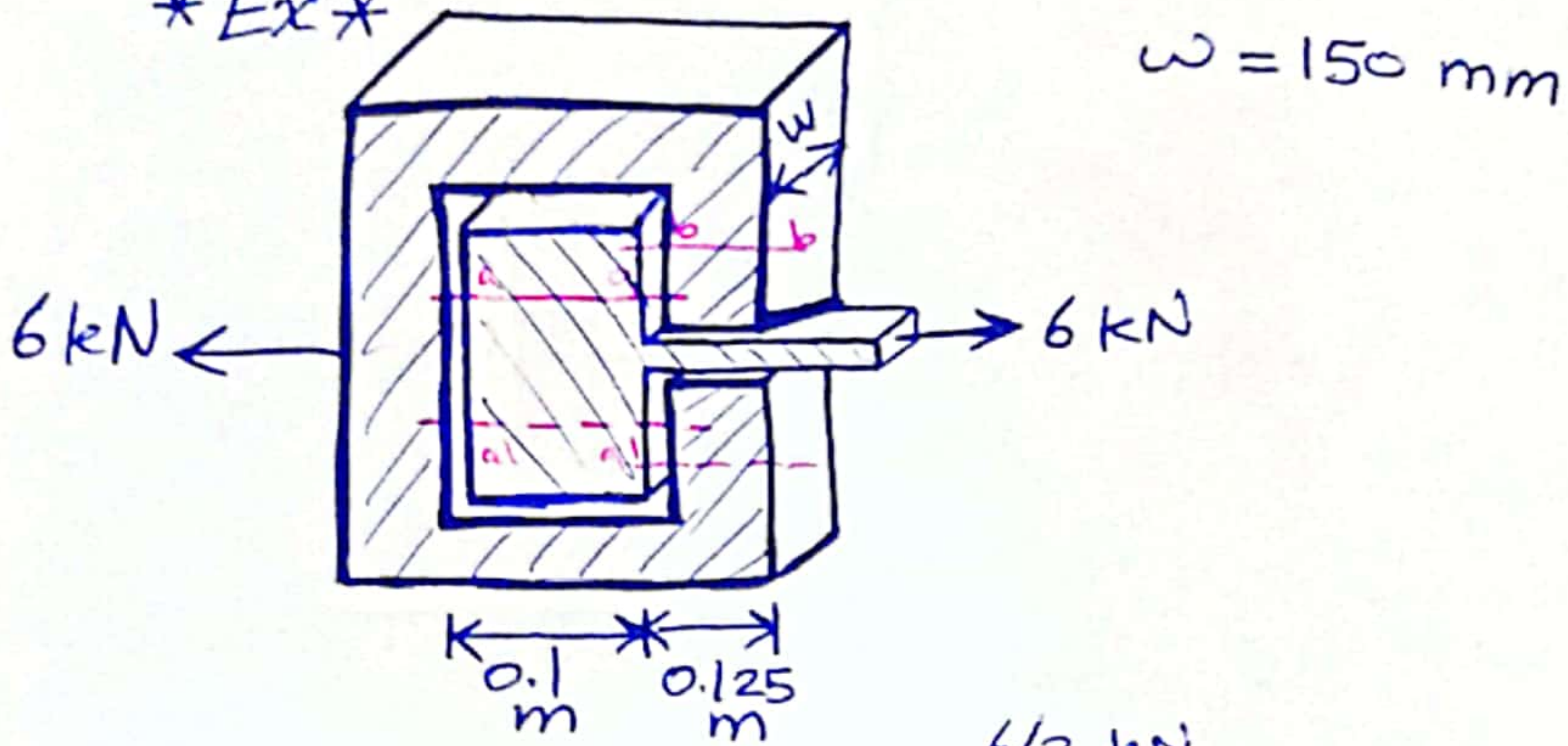
• at C : $\sigma_b \Big|_{\text{at C}} = \frac{50,000}{20 \times 25 \times 10^{-6}} = 100 \text{ MPa}$
in the member (BC)



in the bracket at C →

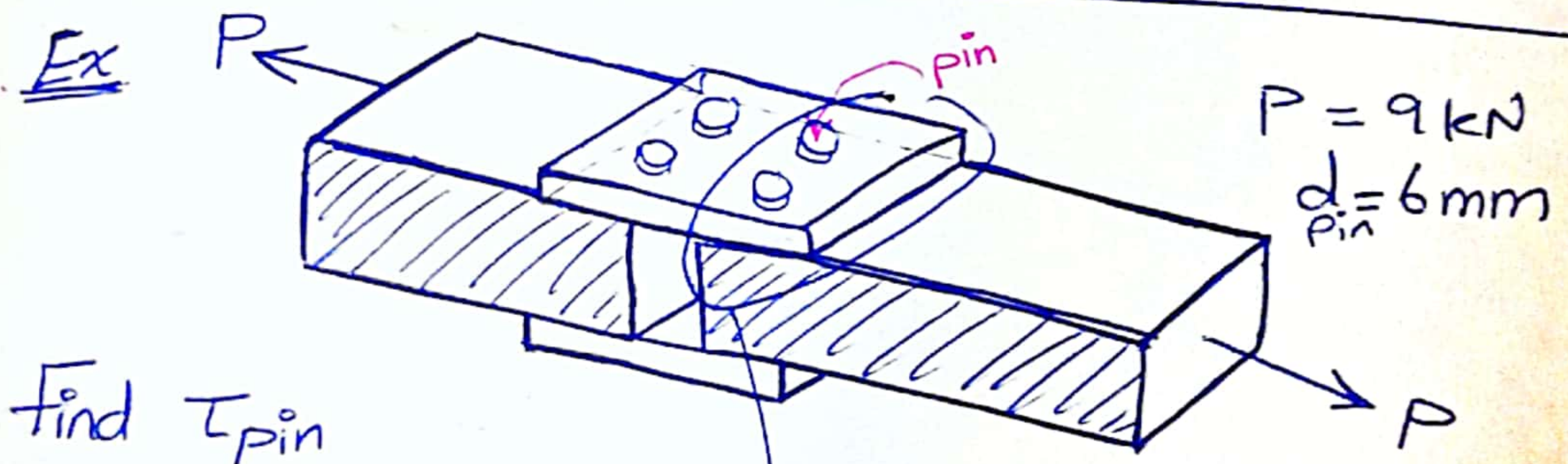
$$\sigma_b = \frac{50,000}{40 \times 25 \times 10^{-6}} = 50 \text{ MPa}$$

* Ex *



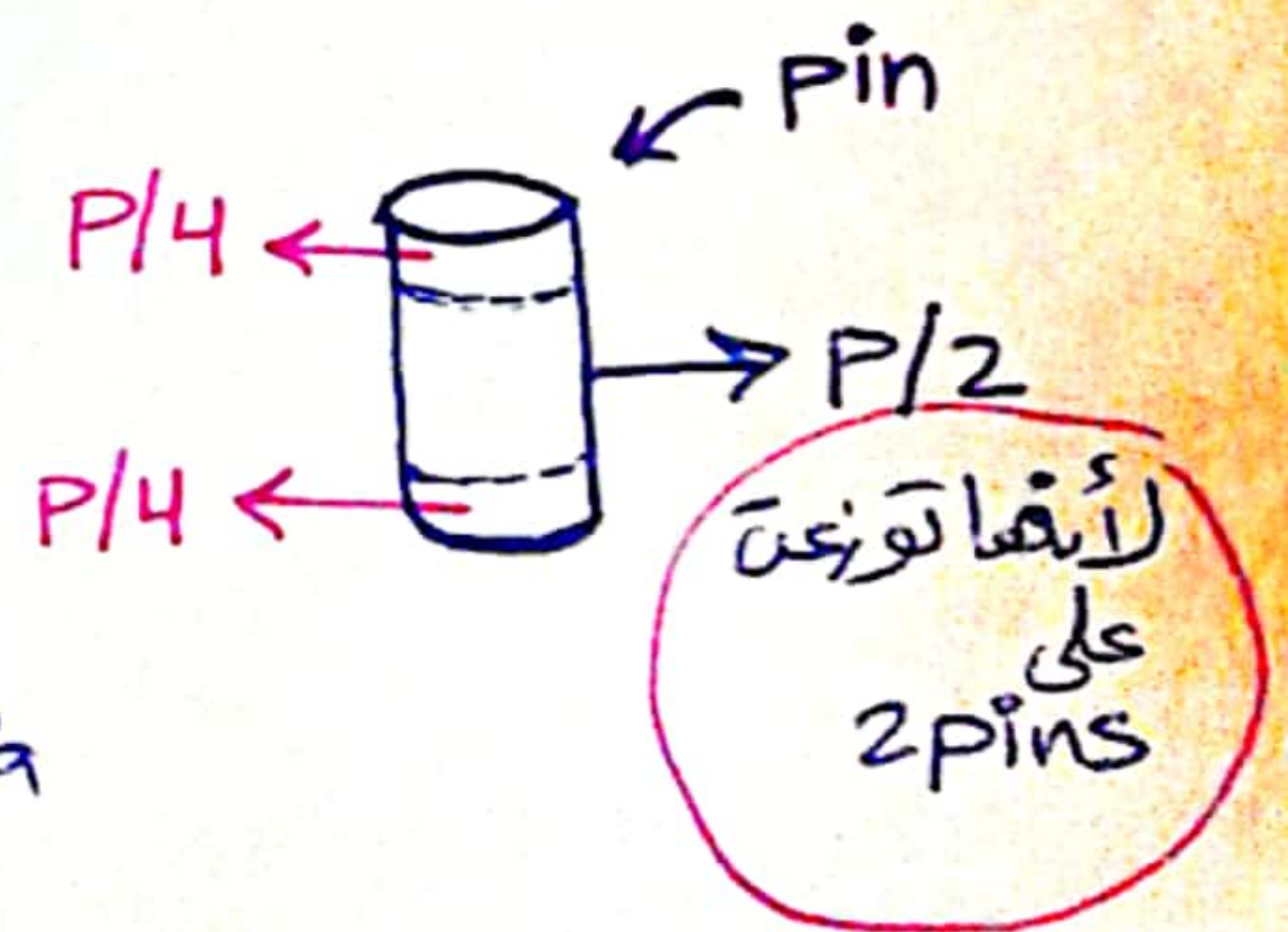
Find: $\tau_{a-a} = \frac{3 \times 10^3}{(0.1)(0.15)} = 200 \text{ kPa}$

$\tau_{b-b} = \frac{6000/2}{(0.125)(0.15)} = 160 \text{ kPa}$



Find τ_{pin}

$$\tau_{pin} = \frac{P/4}{A_0} = \frac{9000/4}{\frac{\pi}{4} (6 \times 10^{-3})^2} = 79.6 \text{ MPa}$$



Double shear

لو كانوا بـ 2 pins ← 3 pins

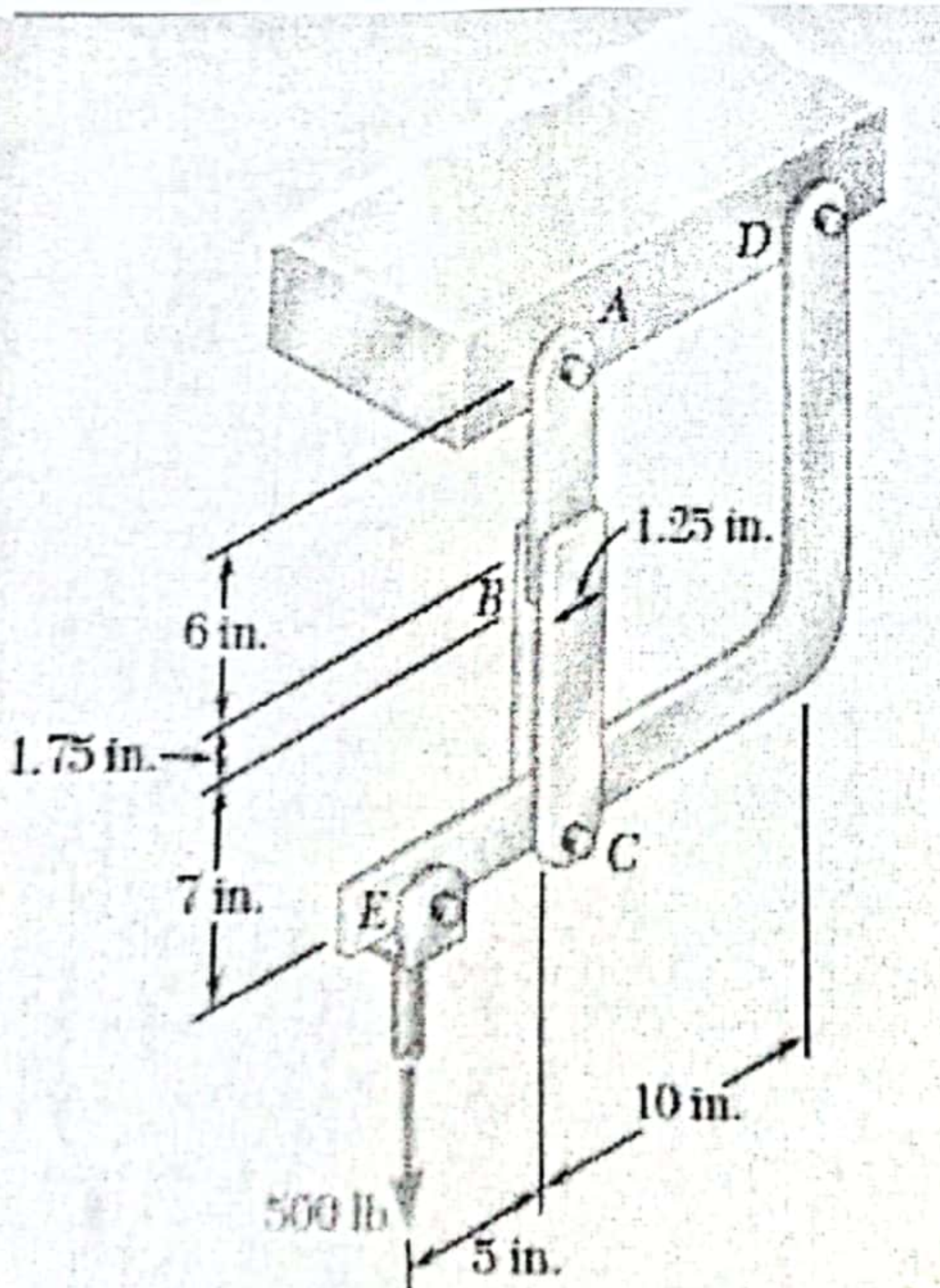
$$\tau_{pin} = \frac{P/6}{A_0}$$

Sample Problem (1.1) ⇒

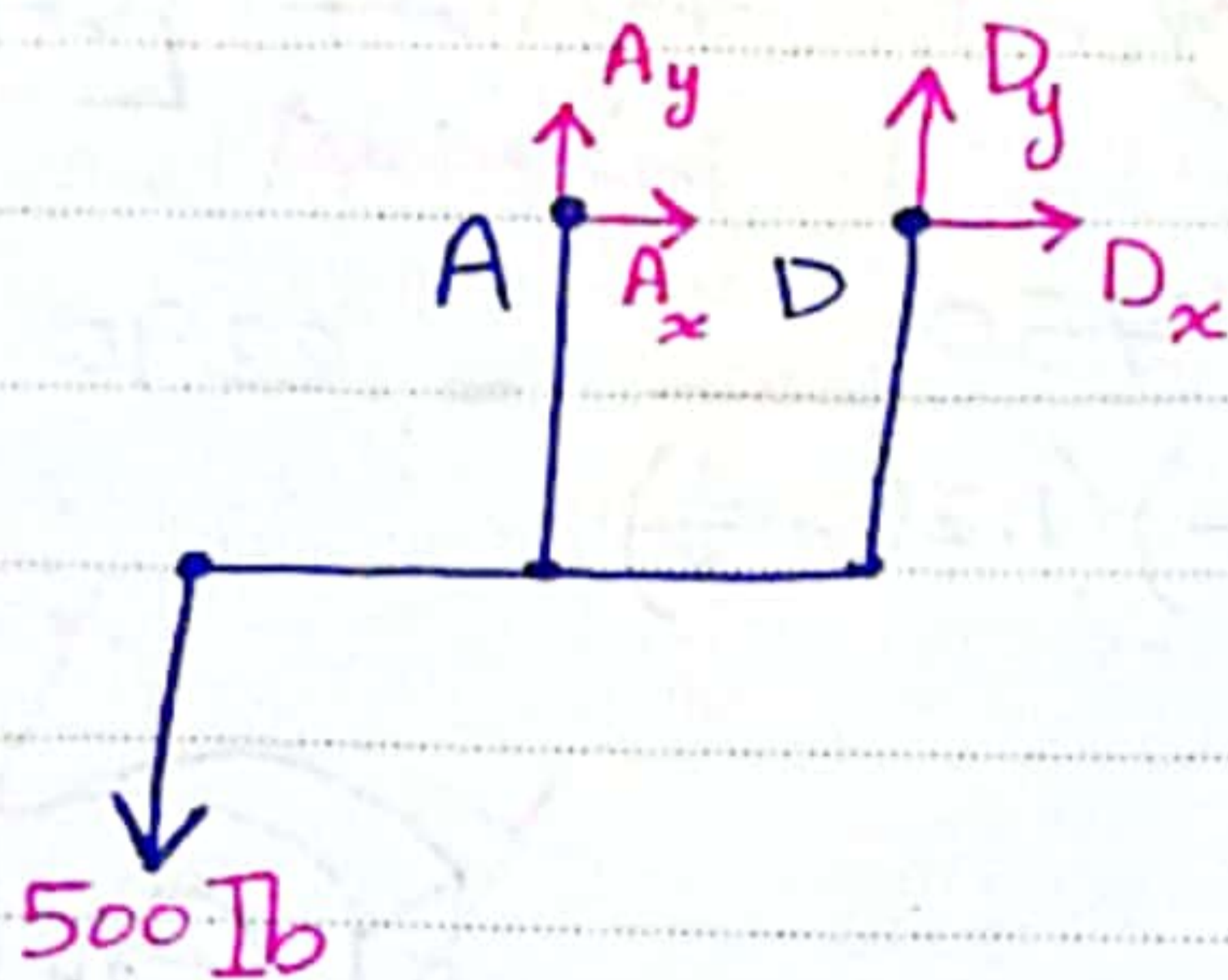
In the hanger shown, the upper portion of link ABC is $\frac{3}{8}$ inch thick and the lower portions are each $\frac{1}{4}$ inch thick.

Epoxy resin is used to bond the upper and lower portions together at B. The pin at A is of $\frac{3}{8}$ inch diameter while a $\frac{1}{4}$ inch diameter pin is used at C. Determine:

- the shearing stress in pin A?
- " " " " = C?
- = largest normal stress in link ABC?
- = average shearing stress on the bonded surfaces at B?
- the bearing stress in the link at C?



→ Free body diagram



(Fixed point) Reaction forces

• $\sum M_D = 0$

$$-A_y(10) + 500(15) = 0 \Rightarrow \boxed{A_y = 750 \text{ lb}}$$

• $\sum F_y = 0$

$$A_y + D_y - 500 = 0 \Rightarrow \boxed{D_y = -250 \text{ lb}}$$

a) $\tau_A = \frac{F_{AC}}{A} = \frac{750 \text{ lb}}{\frac{\pi}{4} \left(\frac{3}{8}\right)^2} = 6790 \text{ psi}$

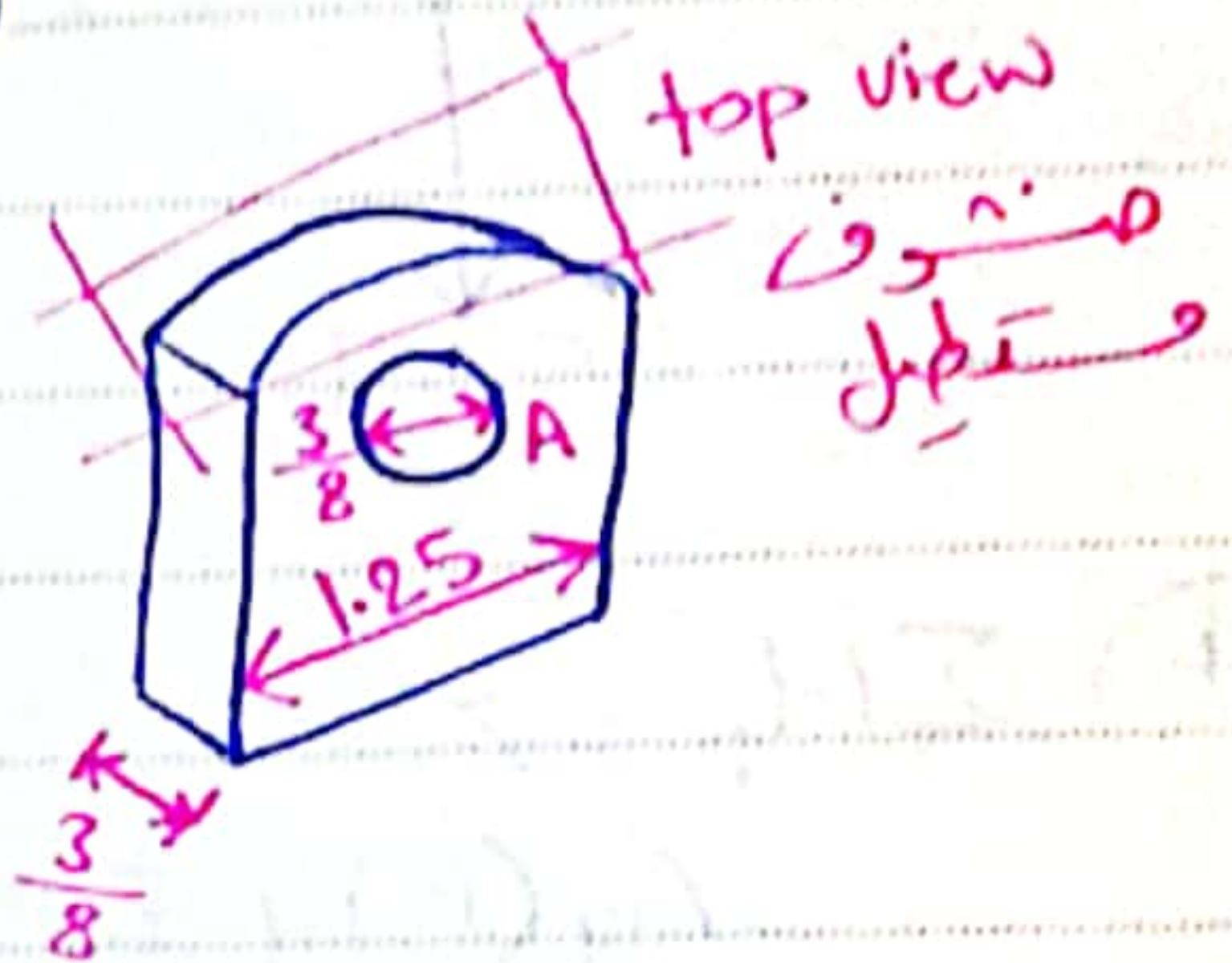
$\left[\frac{3}{8} \text{ inch}\right]$ → diameter pin which is in single shear...

b) $\tau_C = \frac{\frac{1}{2} F_{AC}}{A} = \frac{750/2}{\frac{\pi}{4} \left(\frac{1}{4}\right)^2} = 7640 \text{ psi}$

← diameter pin which is in double shear...

c) largest normal stress in the link ABC occurs where the area is smallest [at A]

$$\sigma_A = \frac{F_{AC}}{A_{net}} = \frac{750}{\left(\frac{3}{8}\right)\left(1.25 - \frac{3}{8}\right)} = 2290 \text{ psi}$$



d) $\tau_B = \frac{F}{A} = \frac{750/2}{1.25 \times 1.75}$

because there are 2 plates

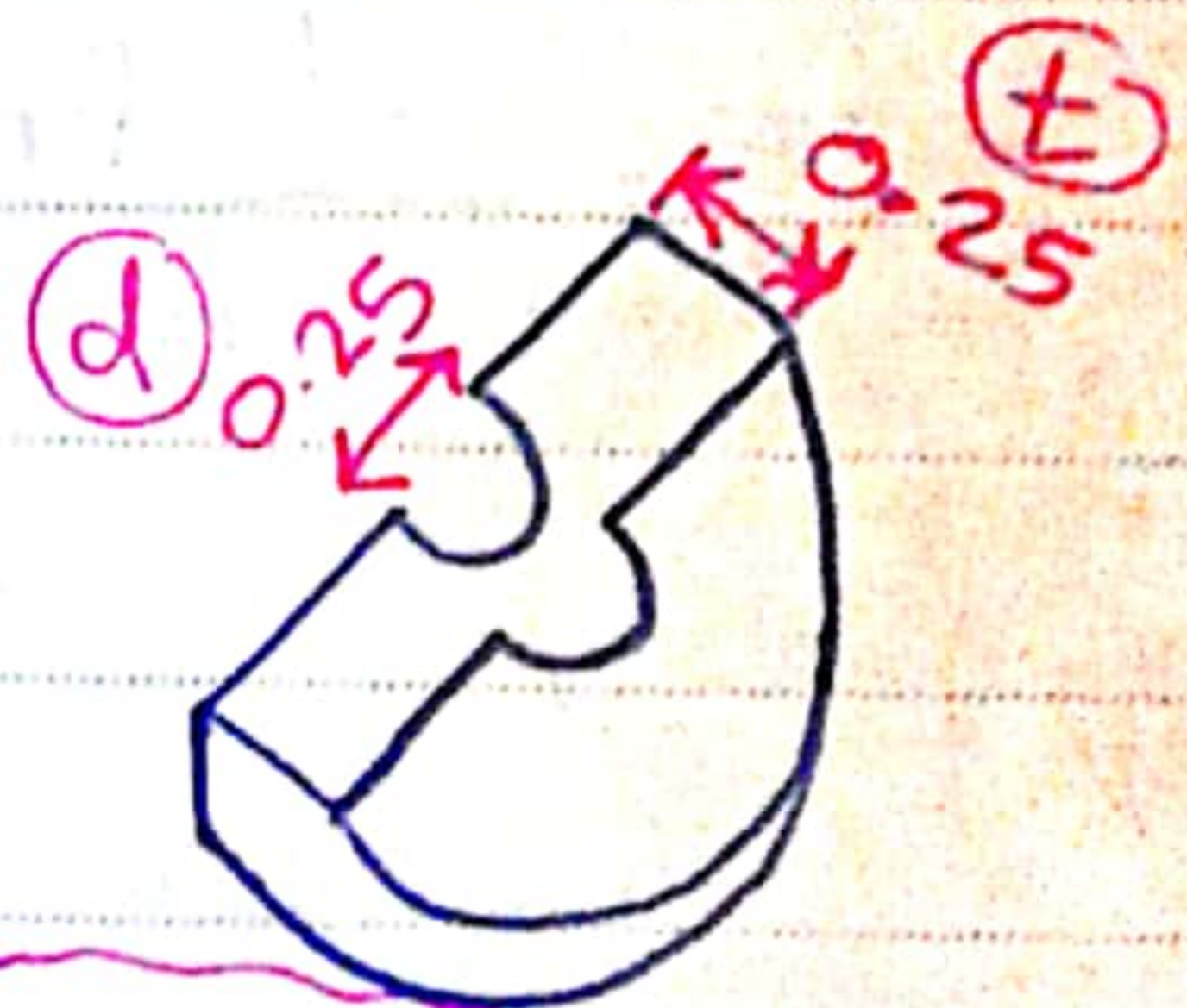
اصطفت ال area هون
كونه ما في برغي فرغ تكون
الاصالة تاسة ال

at B : double shear

Contact Surface

e) $\sigma_b = \frac{F}{A} = \frac{750/2}{0.25 \times 0.25}$

= 6000 psi

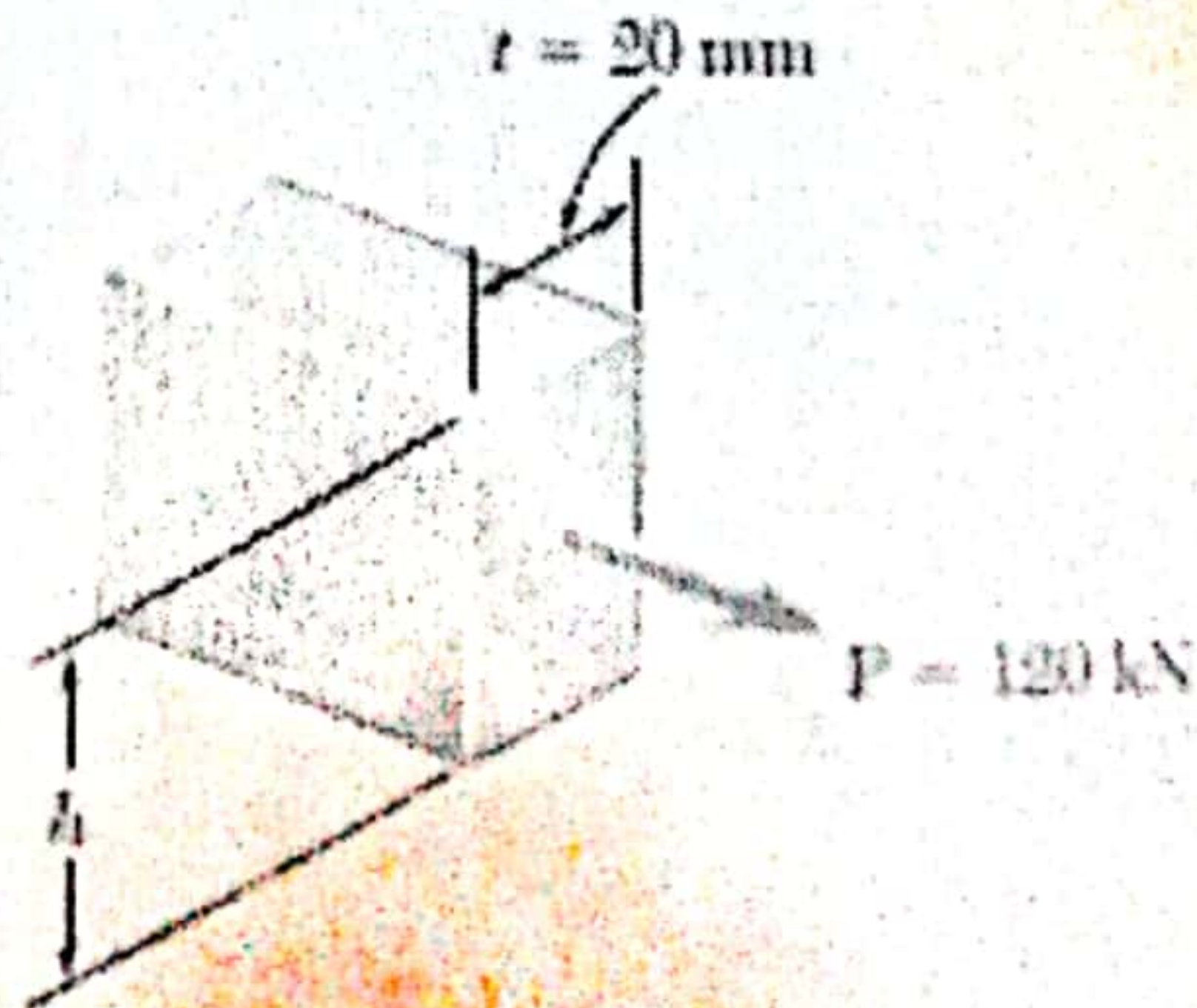
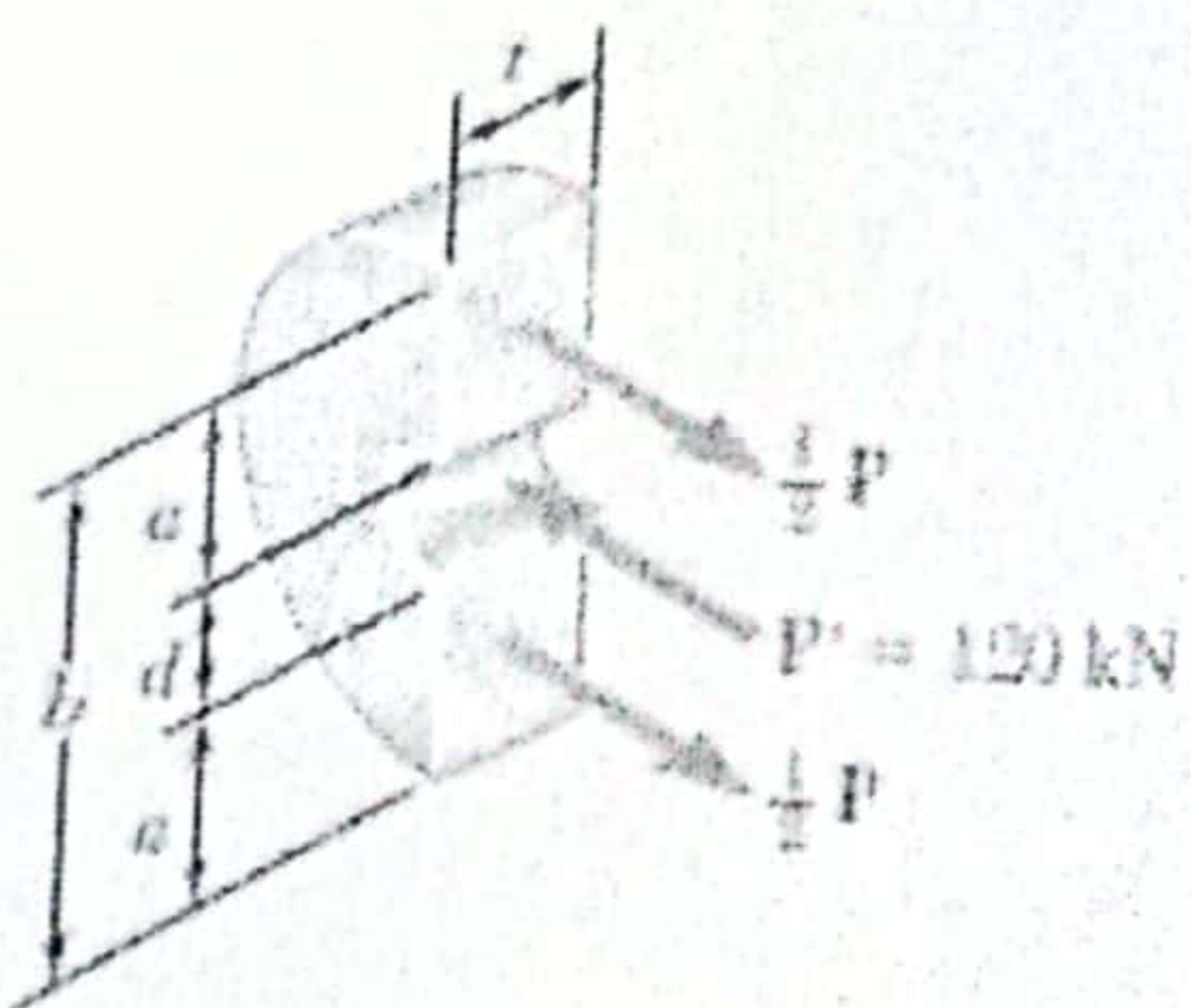
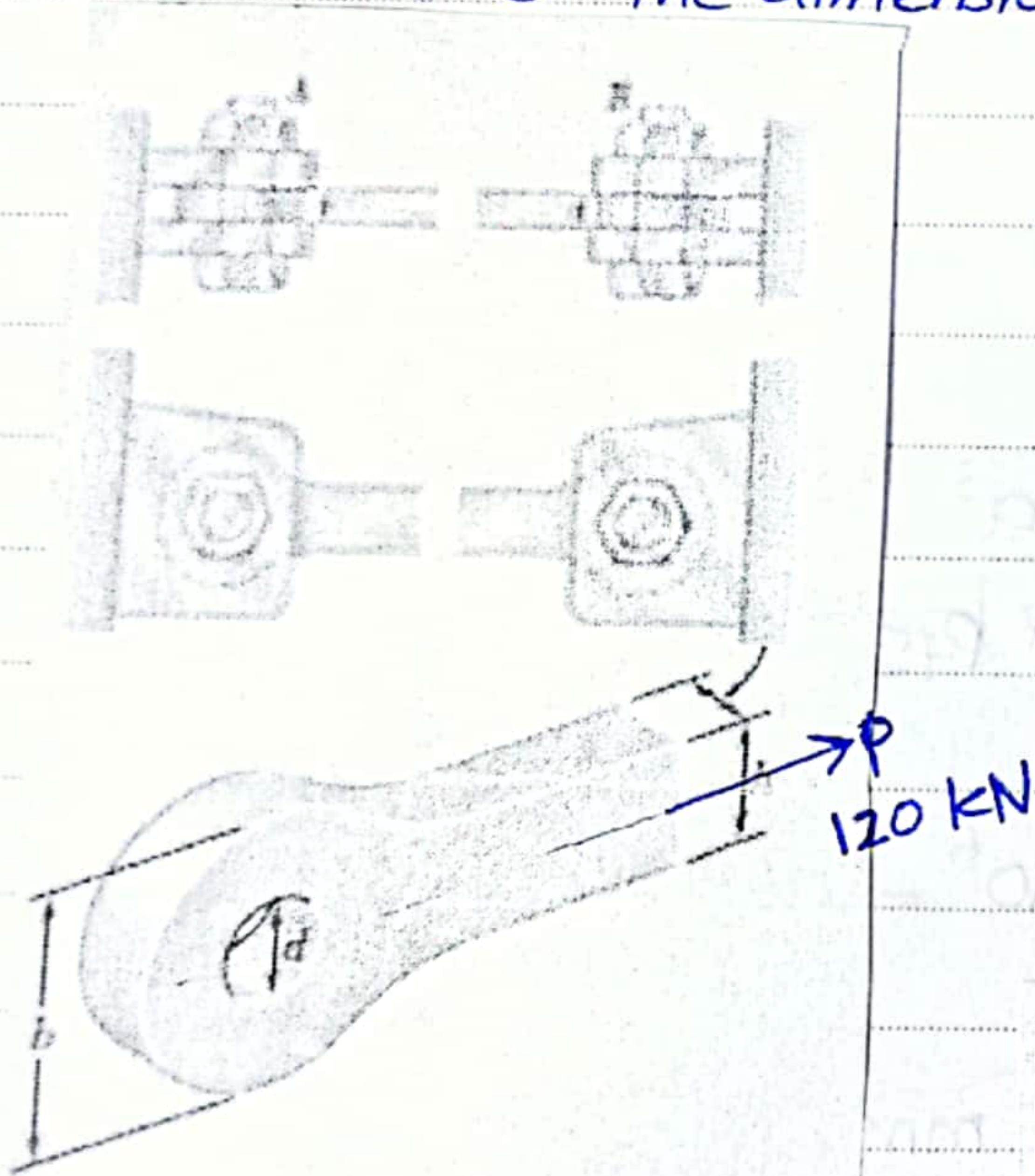


for the two brackets → $F = 750 \text{ lb}$

Sample Problem (1.2) ⇒

The steel tie bar shown is to be designed to carry a tension force of magnitude $P = 120 \text{ kN}$ when bolted between double brackets at A and B. The bar will be fabricated from 20 mm thick plate stock. For the grade of steel to be used, the max. allowable stresses are: $\sigma = 175 \text{ MPa}$, $\tau = 100 \text{ MPa}$, $\sigma_b = 350 \text{ MPa}$.

- Design the tie bar by determining the required values of:
- a- the diameter (d) of the bolt
 - b- the dimension (b) at each end of the bar
 - c- the dimension (h) of the bar



a) the bolt is in double shear

$$F_1 = \frac{1}{2} P = 60 \text{ kN}$$

$$\tau = \frac{F_1}{A} = \frac{60 \text{ kN}}{\frac{\pi}{4} d^2} = 100 \text{ MPa}$$

$$d = 27.6 \text{ mm}$$

مناسبة على 28 mm

فإنه يجب أن يكون σ_b من مطلق قيمة d التي أوجدناها

$$\sigma_b = \frac{120 \times 10^3 \text{ N}}{20 \times 28 \times 10^{-6}} = 214 \text{ MPa} < 350 \text{ MPa}$$

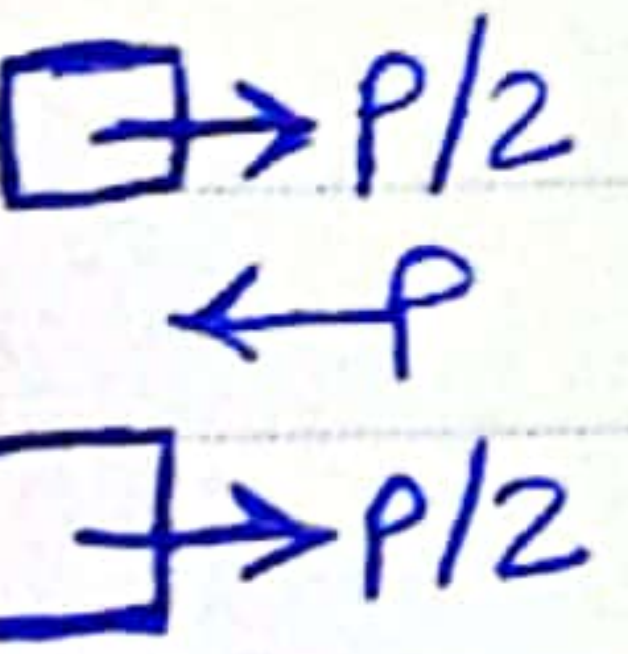
(bearing stress) لأنه ال
حامية على ال
hole
مطروح ما يتفوق
الفورس كامل

البنان صحيح ✓

b) the steel plate thickness = 20 mm

σ_{avg} must not exceed 175 MPa

$$\sigma = \frac{F/2}{A \rightarrow t d} \Rightarrow 175 \times 10^6 = \frac{60 \times 10^3 \text{ N}}{(0.02) a}$$



$$a = 17.14 \text{ mm}$$

$$b = d + 2a = 28 \text{ mm} + 2(17.14) \text{ mm}$$

$$[b = 62.3 \text{ mm}] \#$$

c) Dimension h of the bar

$$\sigma = \frac{P}{th} \Rightarrow 175 \text{ MPa} = \frac{120 \text{ kN}}{202 \times h}$$

$$[h = 34.3 \text{ mm}]$$

ultimate ←

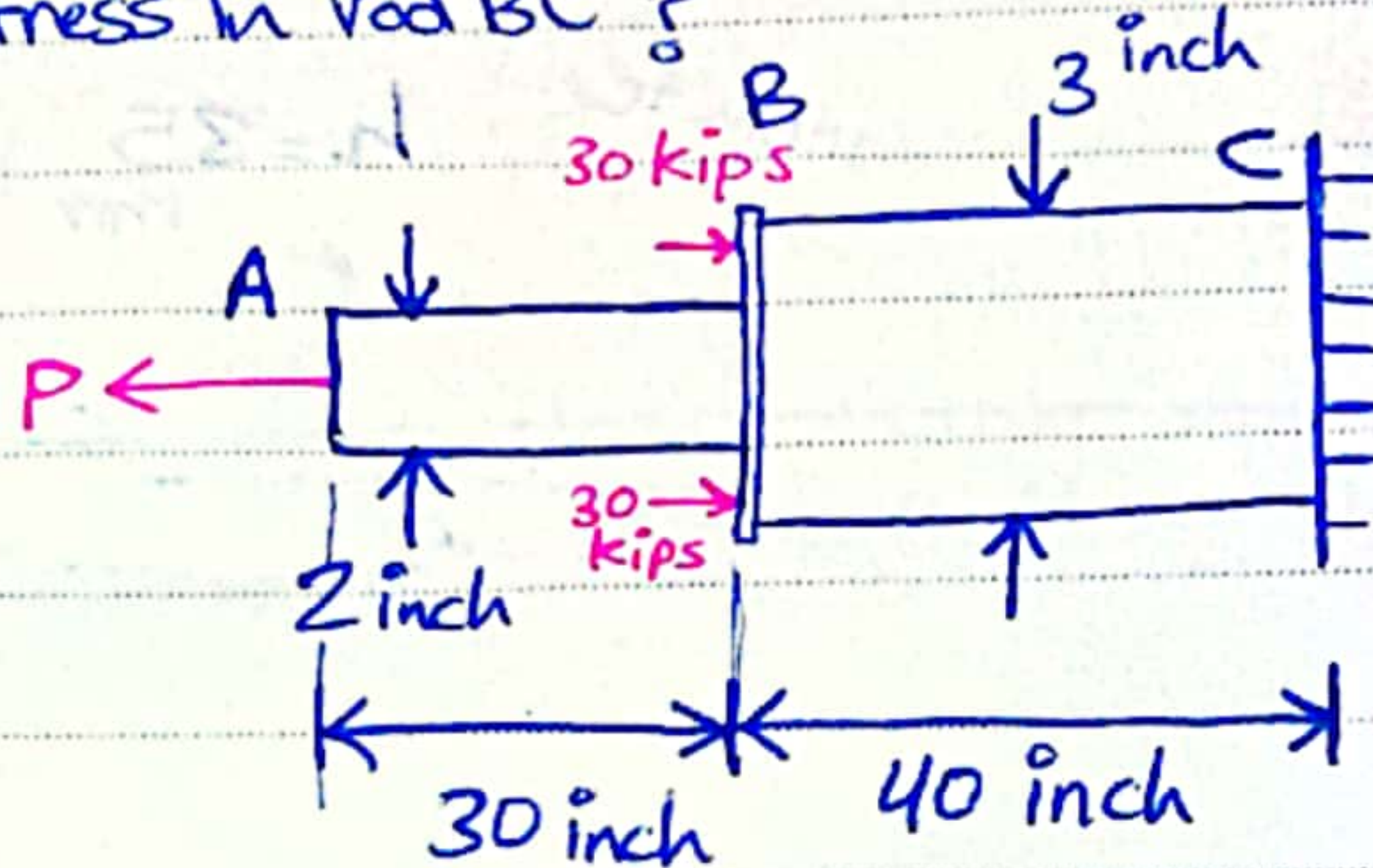
$$h = 35 \text{ mm}$$

$\sigma_{\text{ult}} \leftarrow$

Problems

1.3 Two solid cylindrical rods AB and BC are welded together at B and loaded as shown. Determine the magnitude of the force P for which the tensile stress in rod AB has the same magnitude as the compressive stress in rod BC?

Ans: 18.2 kips



$$A_{AB} = \frac{\pi}{4} (2)^2 = 3.14 \text{ in}^2$$

$$\sigma_{AB} = \frac{P}{3.14} = \underline{0.318 P}$$

$$A_{BC} = \frac{\pi}{4} (3)^2 = 7.068 \text{ in}^2$$

$$\sigma_{BC} = \frac{30(2) - P}{7.068} = \underline{8.488 - 0.141 P}$$

$$\sigma_{AB} = \sigma_{BC}$$

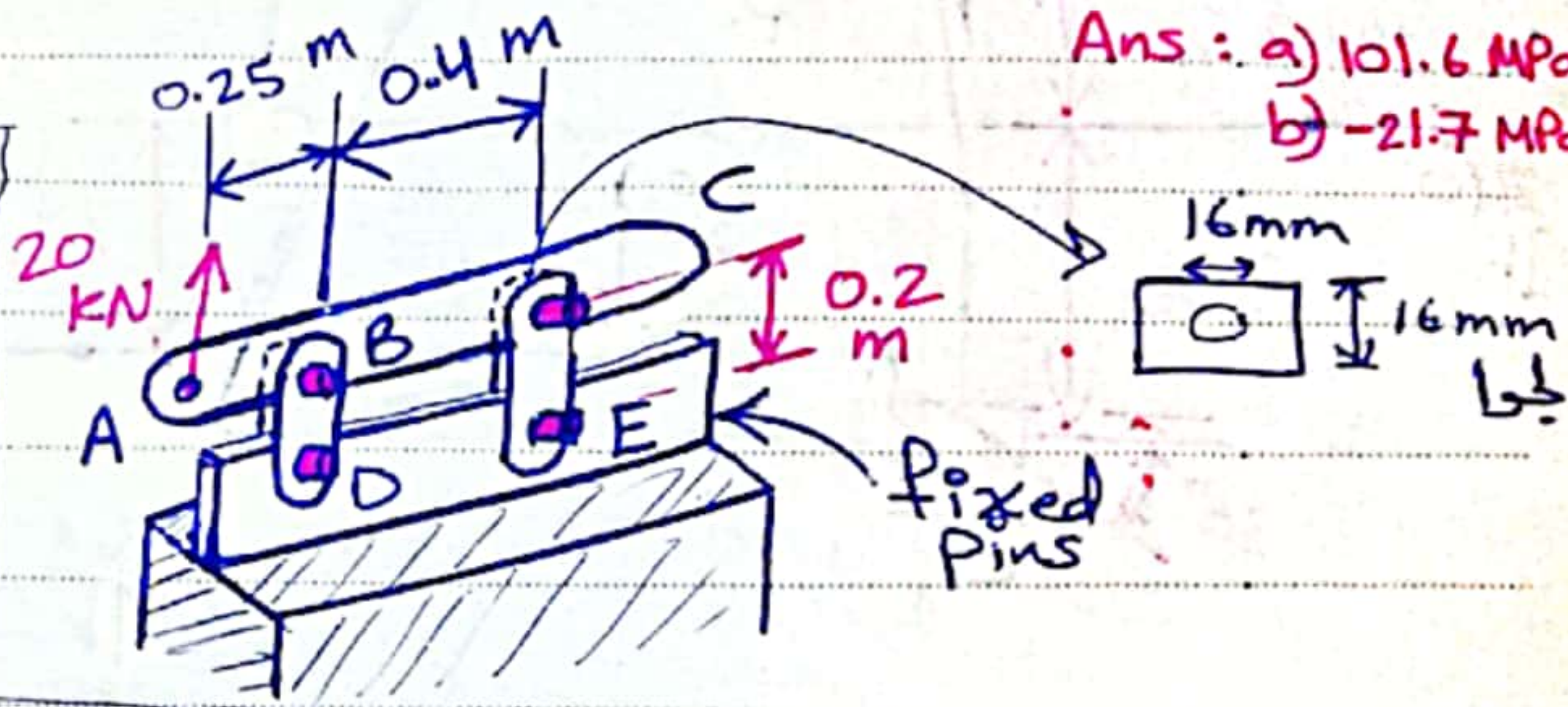
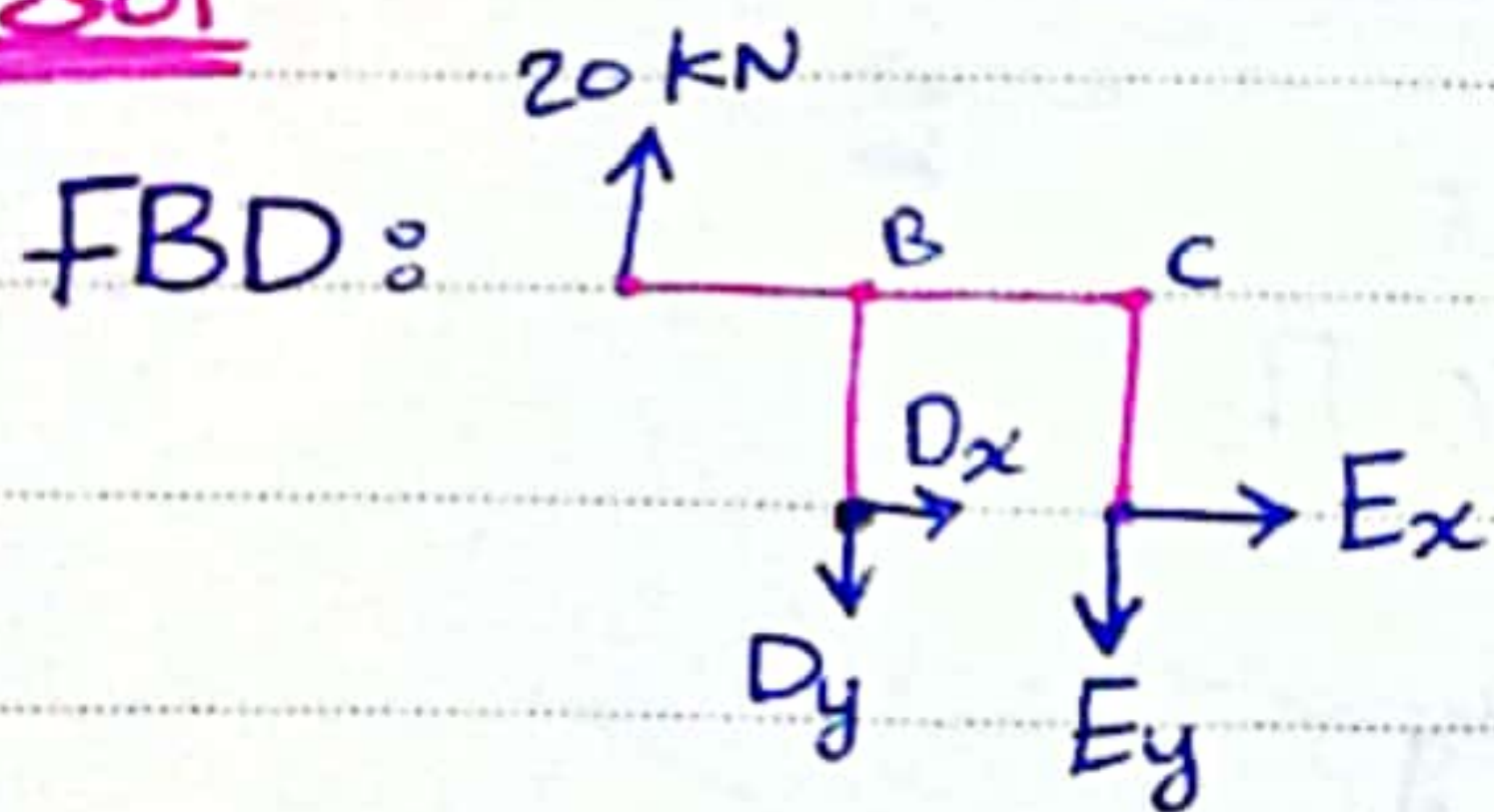
$$0.318 P = 8.488 - 0.141 P$$

$$P = 18.5 \text{ kips}$$

1.7 Each of the four vertical links has an 8×36 mm uniform rectangular cross section and each of the four pins has a 16 mm diameter. Determine the max. value of the average normal stress in the links connecting:

- points B and D
- points C and E

Sol



Ans: a) 101.6 MPa
b) -21.7 MPa

$$\sum M_E = 0 \rightarrow$$

$$-D_y(0.4) + 20,000(0.4 + 0.25) = 0 \quad \boxed{D_y = 32,500 \text{ N}} \text{ tension}$$

$$\sum F_y = 0 \rightarrow 20,000 - D_y - E_y = 0 \quad \boxed{E_y = -12,500 \text{ N}} \text{ compression}$$

$$\sum F_x = 0 \rightarrow E_x + D_x = 0$$

* Net area of one link for tension = $0.008(0.036 - 0.016)$

$$a) \quad \sigma_{BD} = \frac{F}{A} = \frac{32,500 \text{ N}}{160 \times 10^{-6} \text{ m}^2}$$

$$= \frac{32,500 \text{ N}}{(0.008)(0.036 - 0.016)2}$$

$$= 101.6 \text{ MPa}$$

For 2 parallel links so

$$\boxed{A_{\text{net}} = 320 \times 10^{-6} \text{ m}^2}$$

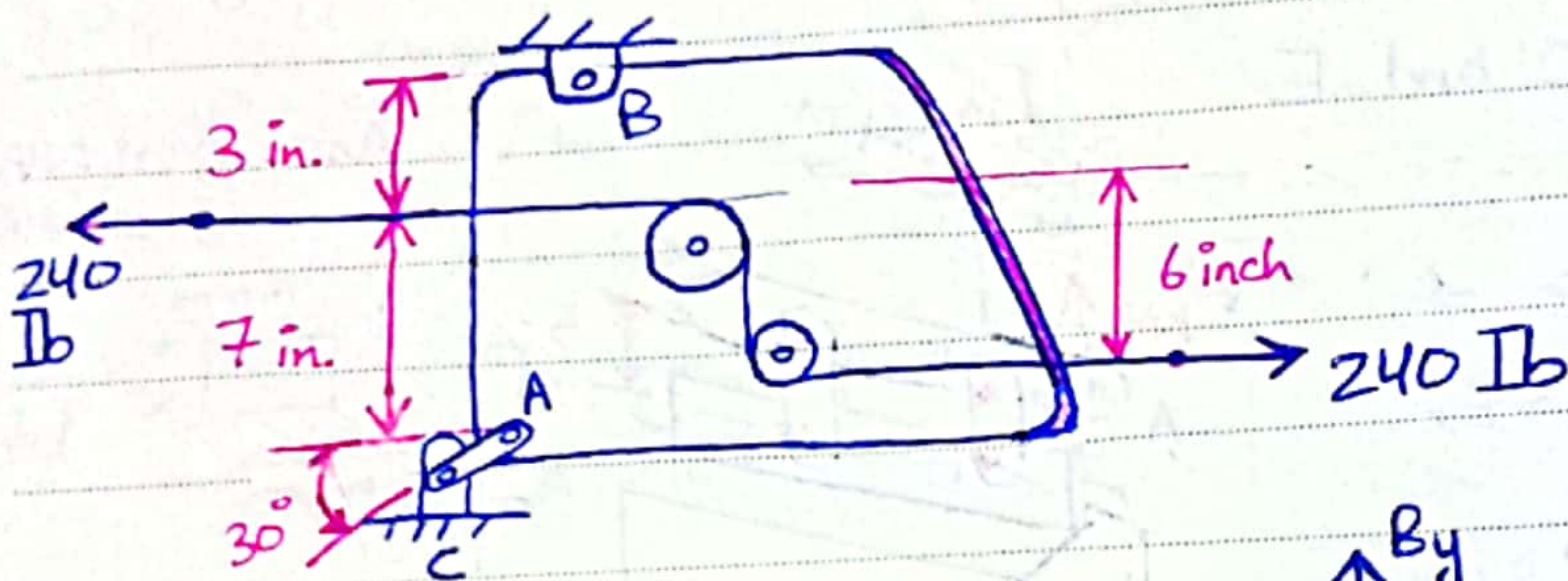
$$b) \text{ Area for one link in compression} = (0.008)(0.036) = 288 \times 10^{-6} \text{ m}^2$$

For 2 parallel links, $A = 576 \times 10^{-6} \text{ m}^2$

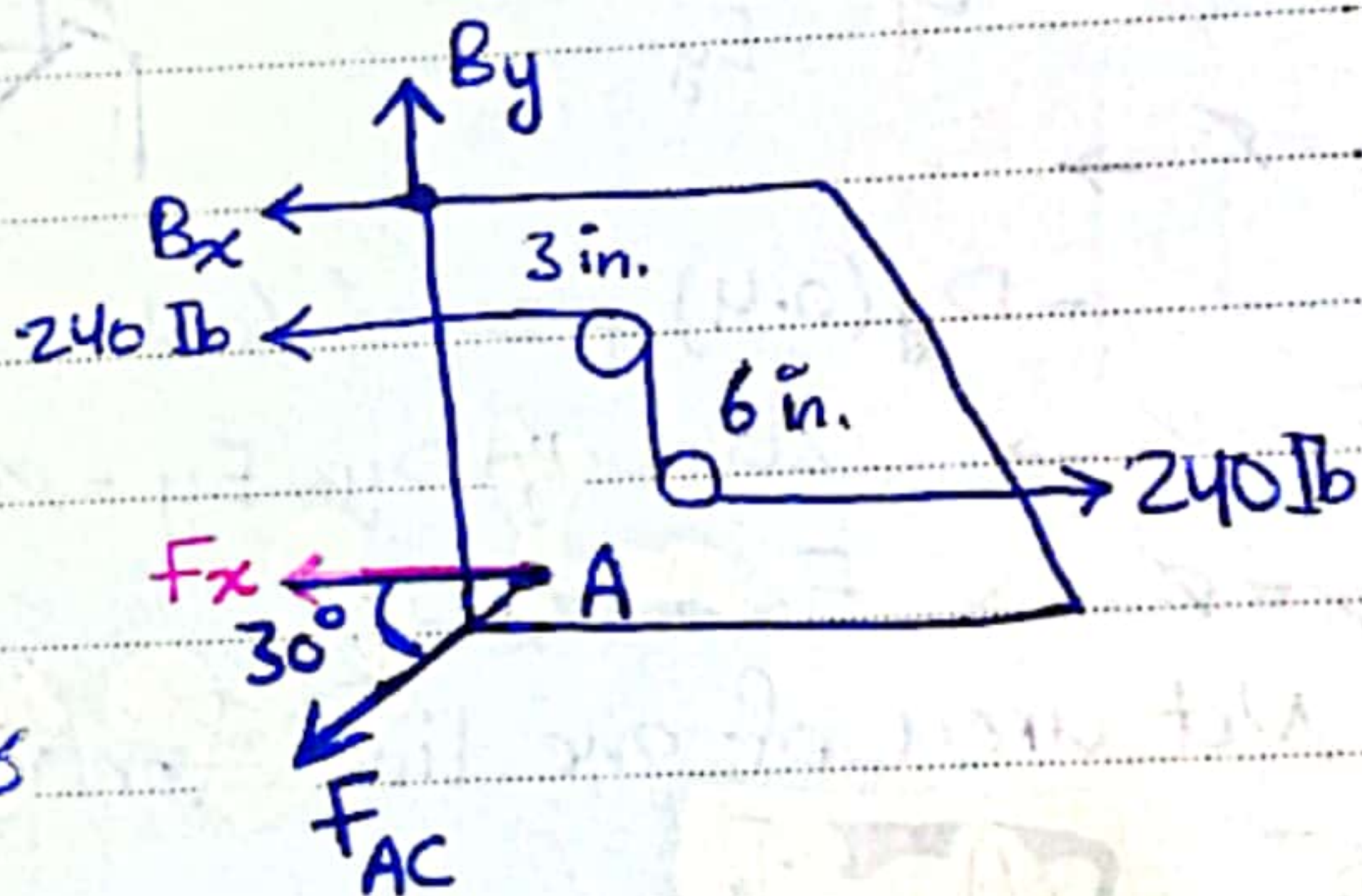
$$\sigma_{CE} = \frac{-12,500}{576 \times 10^{-6}} = -21.7 \text{ MPa}$$

1.9 Link AC has a uniform rectangular cross section $\frac{1}{16}$ in. thick and $\frac{1}{4}$ in. wide. Determine the normal stress in the central portion of the link?

Ans: 10.64 ksi



$$\sigma_{AC} = \frac{F_{AC}}{A}$$



• $\sum M_B = 0 \quad \curvearrowright +$

$$-240(3) - F_x(10) + 240(9) = 0$$

$$F_x = 144 \text{ lb}$$

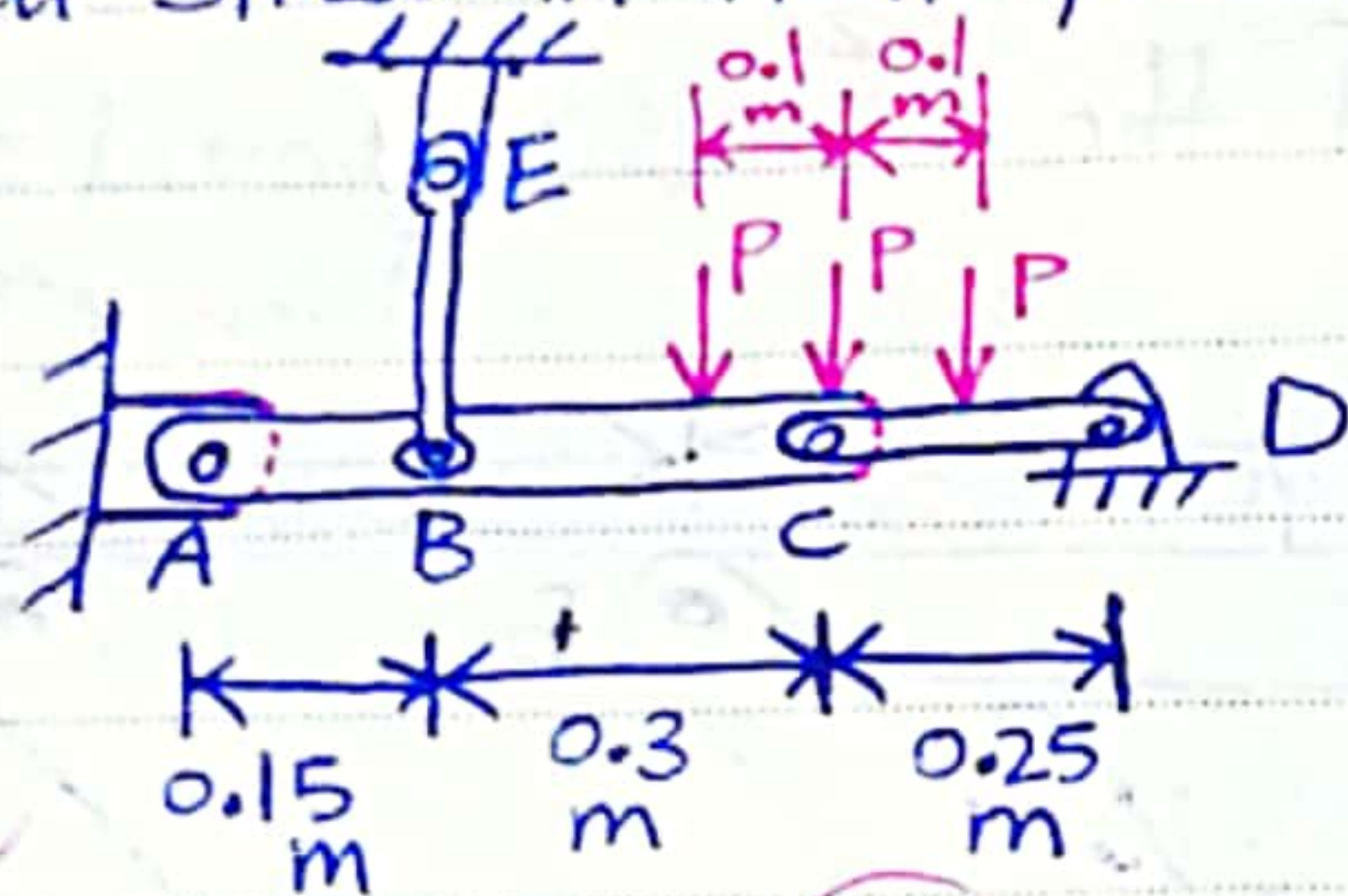
where, $F_x = F_{AC} \cos(30)$

$$F_{AC} = \frac{144}{\cos(30)} = 166.28 \text{ lb}$$

$$\sigma_{AC} = \frac{166.28 \text{ lb}}{\frac{1}{16} \times \frac{1}{4}} = 10.64 \text{ ksi}$$

#

1.10 Three forces, each of magnitude $P = 4 \text{ kN}$, are applied to the mechanism shown. Determine the cross sectional area of the uniform portion of rod BE for which the normal stress in that portion is $+100 \text{ MPa}$?



Ans: 285 mm^2

→ FBD for CD Link:

• $\sum M_D = 0 \curvearrowright$

$$C_y(0.1 + 0.15) - 4000(0.15) = 0$$

$$C_y = 2400 \text{ N}$$

→ FBD for Link ABC:

• $\sum M_A = 0 \curvearrowright$

$$B_y(0.15) - 4000(0.35) - 4000(0.45)$$

$$- 2400(0.45) = 0$$

$$B_y = 28.53 \text{ kN}$$

$$100 \times 10^6 = \frac{28.53 \times 10^3}{A \text{ [m}^2\text{]}}$$

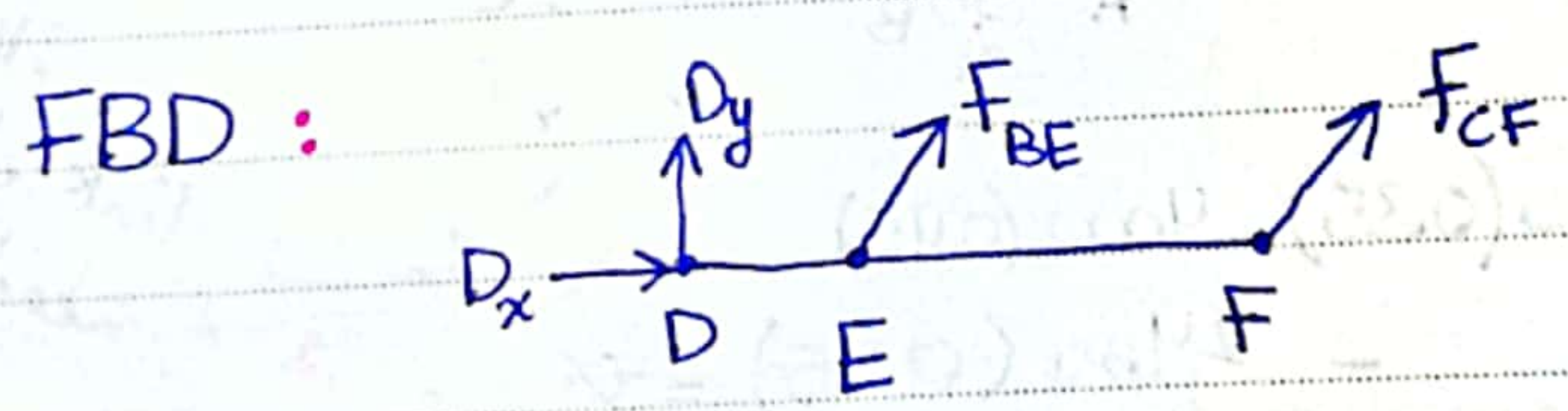
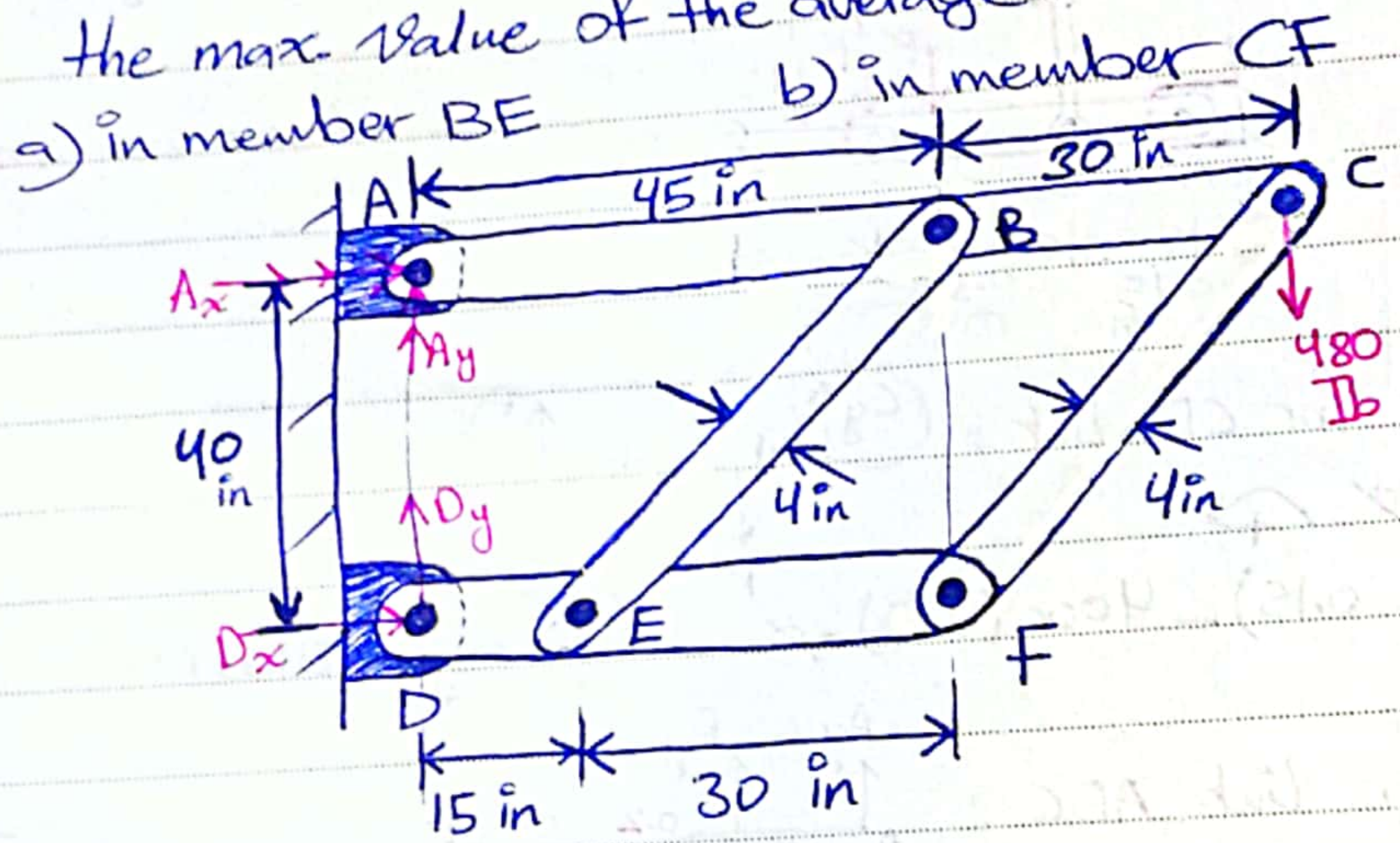
$$A = 2.85 \times 10^{-4} \text{ m}^2 \times \frac{1 \text{ cm}^2}{10^{-6} \text{ m}^2}$$

$$= 285 \text{ mm}^2$$

⇒ Remember

أي اتجاه يفرضه (link) معين، لأنهم أعمل عكسه على الـ (link) الآخر التابع له.

1.11 The frame shown consists of four wooden members, ABC, DEF, BE and CF. Knowing that each member has a 2x4 in. rectangular cross section and that each pin has a $\frac{1}{2}$ in. diameter, determine the max. value of the average normal stress



$$\sum M_A = 0 \Rightarrow 40 D_x - (30 + 45) 480 = 0$$

* $D_x = 900 \text{ lb}$

using DEF member, the reaction at D must be parallel to F_{BE} and F_{CF}

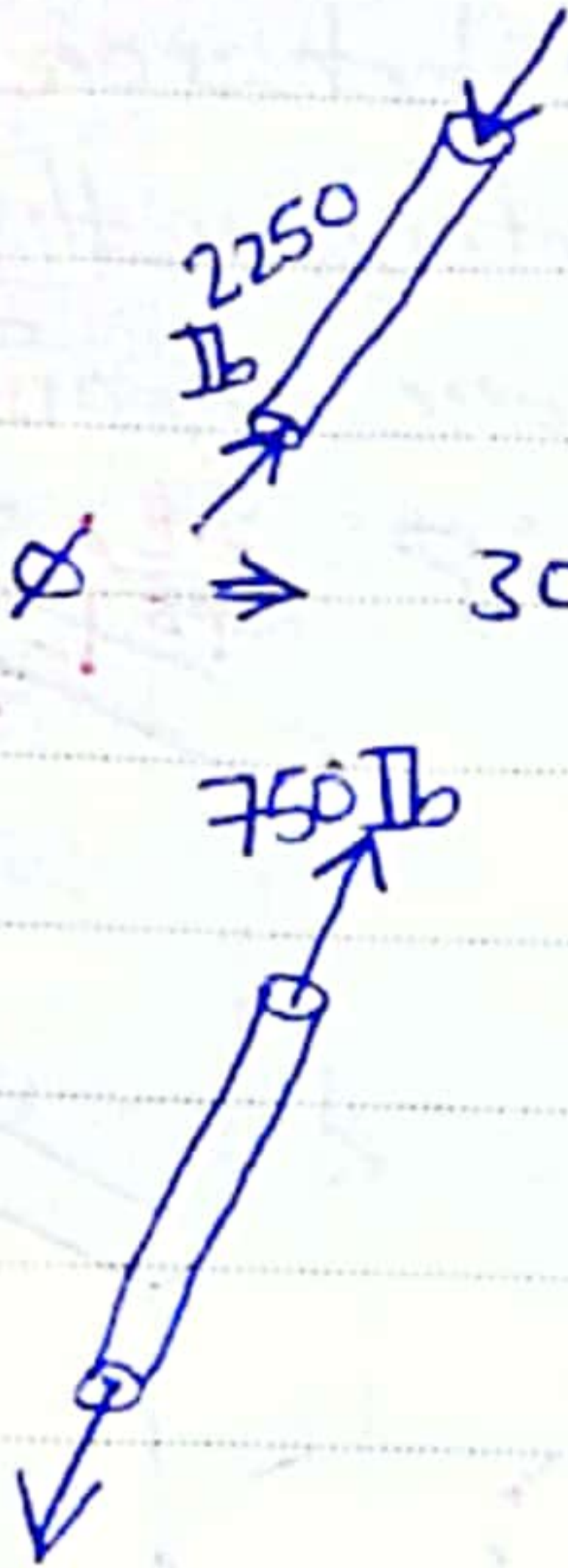
$$* D_y = \frac{4}{3} D_x = 1200 \text{ lb}$$

$$\sum M_F = 0 \Rightarrow -30 \left(\frac{4}{5} F_{BE} \right) - (30+15) D_y = 0$$

$$F_{BE} = -2250 \text{ lb} \quad \text{compression force}$$

$$\sum M_E = 0 \Rightarrow 30 \left(\frac{4}{5} F_{CF} \right) - 15 (D_y) = 0$$

$$F_{CF} = 750 \text{ lb} \quad \text{tension force}$$



$$a) \sigma_{BE} = \frac{-2250 \text{ [lb]}}{2 \times 4 \text{ [in}^2\text{]}} = -281 \text{ psi}$$

min. section area occurs at pin

$$A_{\min} = 2(4 - 0.5) = 7 \text{ in}^2$$

$$b) \sigma_{CF} = \frac{750 \text{ [lb]}}{7 \text{ [in}^2\text{]}} = 107.1 \text{ psi}$$

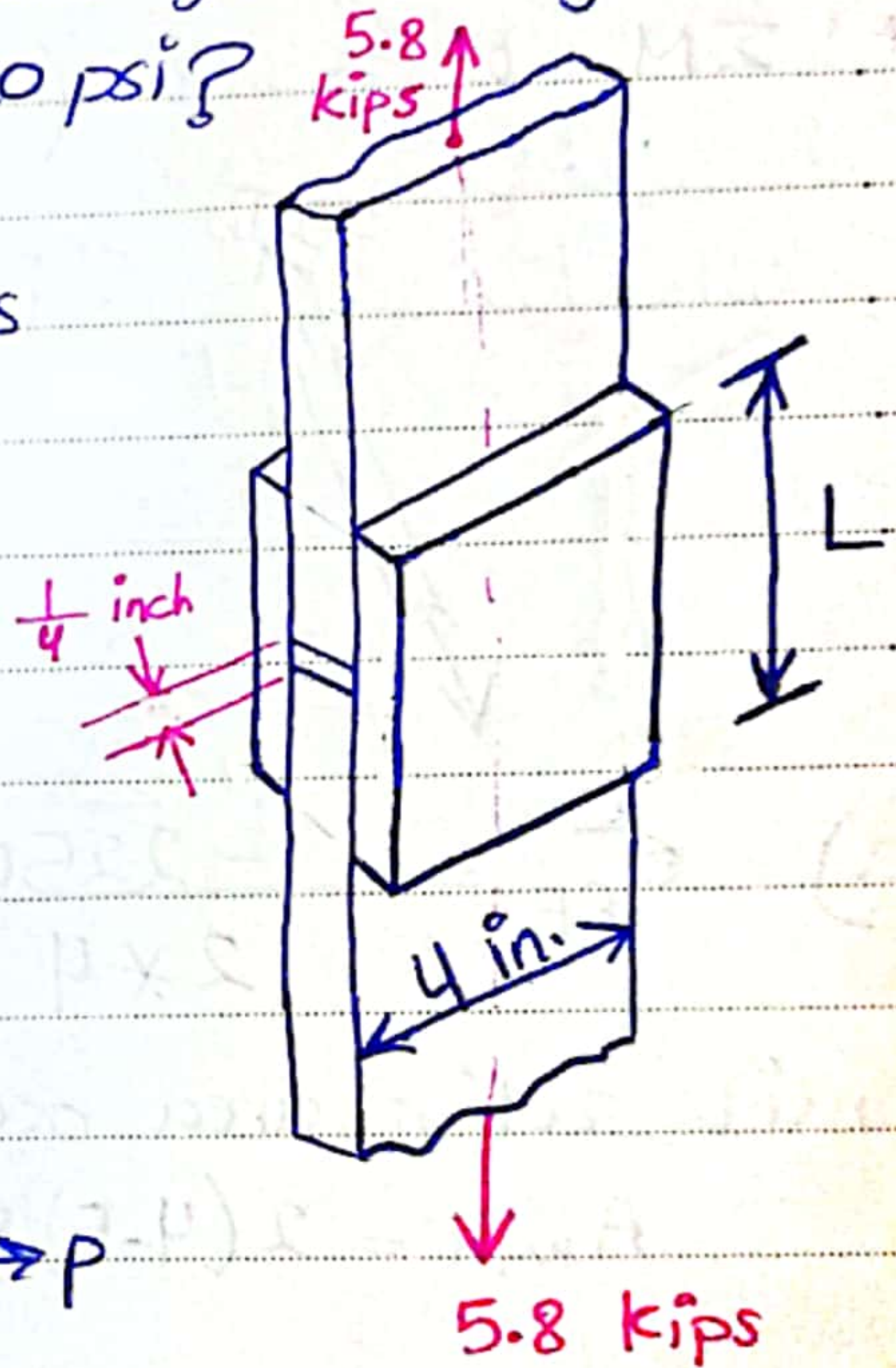
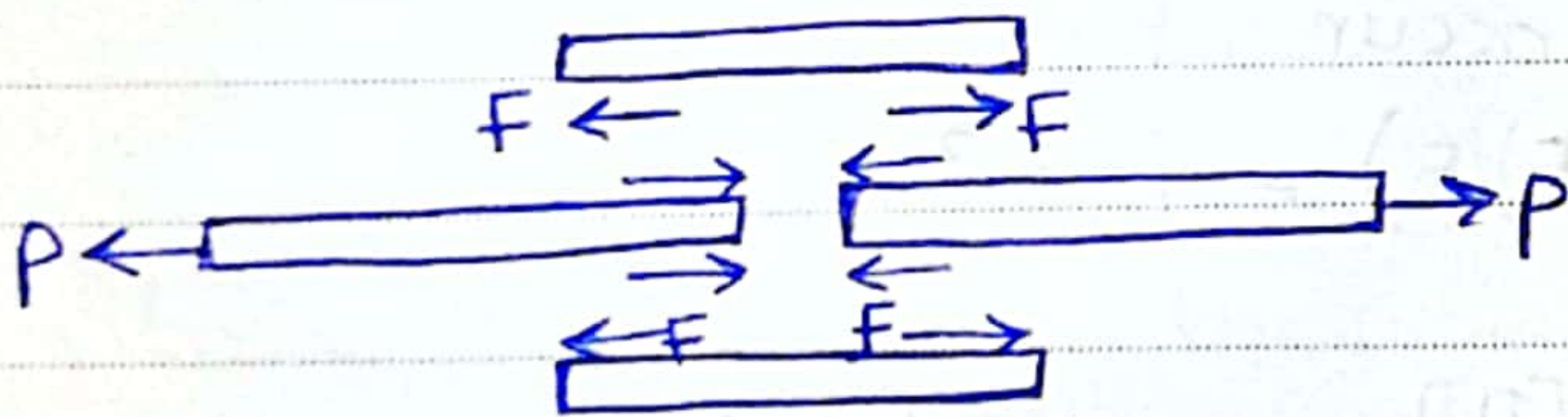
1.16 The wooden members A and B are to be joined by plywood splice plates that will be fully glued on the surfaces in contact. As part of the design of the joint, and knowing that the clearance between the ends of the members is to be $\frac{1}{4}$ inch, determine the smallest allowable length (L) if the average shearing stress in the glue isn't to exceed 120 psi?

Ans: 12.33 inch

→ There are 4 separate areas that are glued.

Each of these areas transmits one half the 5.8 kips load. Thus

$$F = \frac{1}{2} * 5.8 = 2.9 \text{ kips}$$



let L = length of one glued area and $w = 4$ inch be its width.

For each glued area, $A = LW$

/ /

Average shearing stress : $\tau = \frac{F}{A} = \frac{F}{LW}$

$$\left[\tau_{\text{allow}} = 120 \text{ psi} \right]$$

$$L = \frac{F}{\tau W} = \frac{2900}{120 * 4 \text{ in.}} = 6.042 \text{ in.}$$

● Total length = $6.042 + \frac{1}{4} + 6.042$
(L_{tot})

gap ↙

$$= 12.33 \text{ in.}$$

#

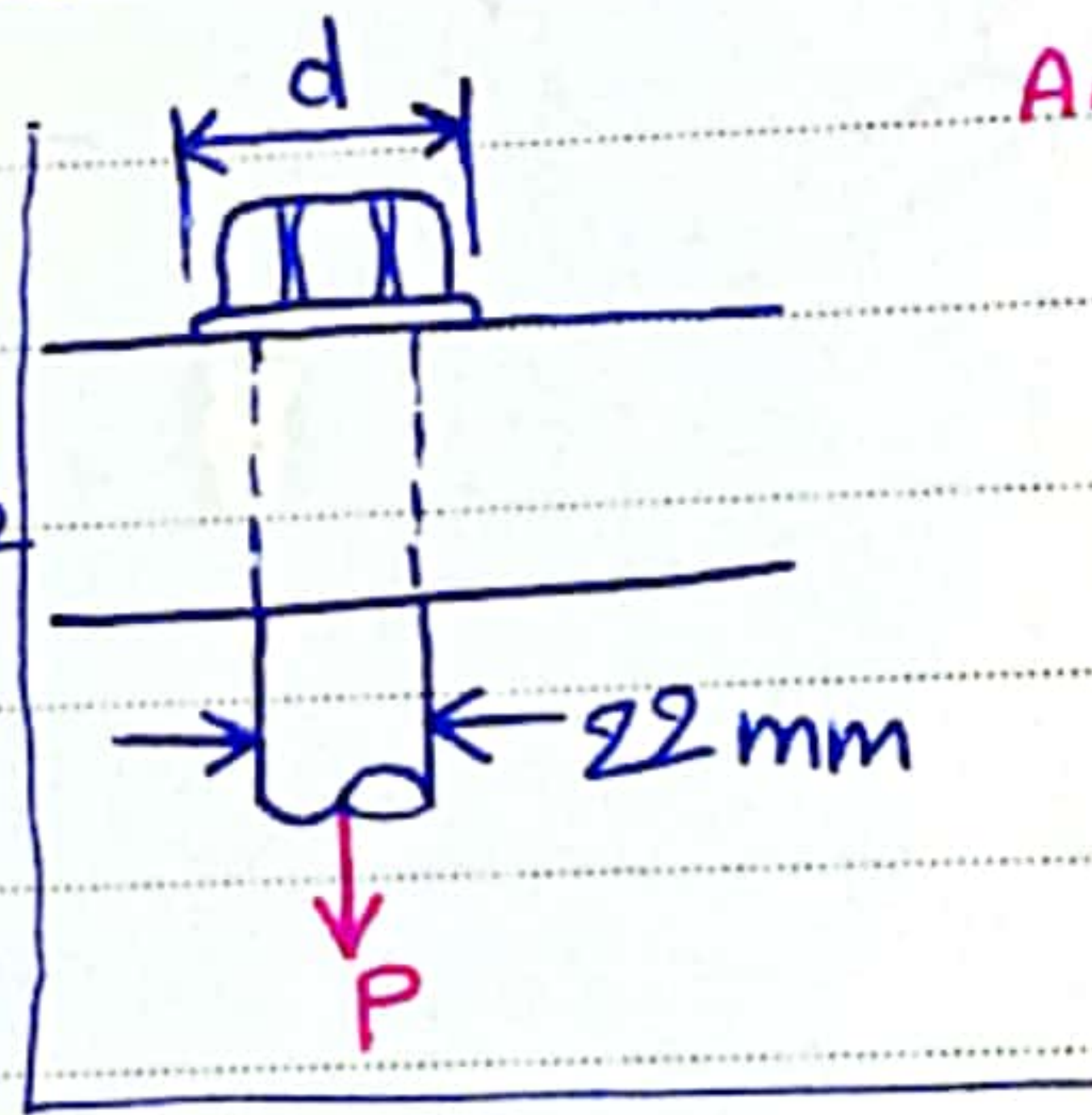
1.19 The load P applied to a steel rod is distributed to a timber support by an annular washer. The diameter of the rod is 22 mm and the inner diameter of the washer is 25 mm, which is slightly larger than the diameter of the hole. Determine the smallest allowable outer diameter d of the washer knowing that the axial normal stress in the steel rod is 35 MPa and that the average bearing stress between the washer & the timber must not exceed 5 MPa?

Ans: 63.3 mm

→ steel rod: $A = \frac{\pi}{4} (0.022)^2$
 $= 380.13 \times 10^{-6} \text{ m}^2$

$\sigma = 35 \times 10^6 \text{ Pa}$ given

$P = (35 \times 10^6) (380.13 \times 10^{-6})$
 $= 13.305 \times 10^3 \text{ N}$



Washer: $\sigma_b = 5 \times 10^6 \text{ Pa}$ given

required bearing area: $A_b = \frac{P}{\sigma_b} = \frac{13.305 \times 10^3 \text{ N}}{5 \times 10^6 \text{ Pa}}$
 $= 2.6609 \times 10^{-3} \text{ m}^2$

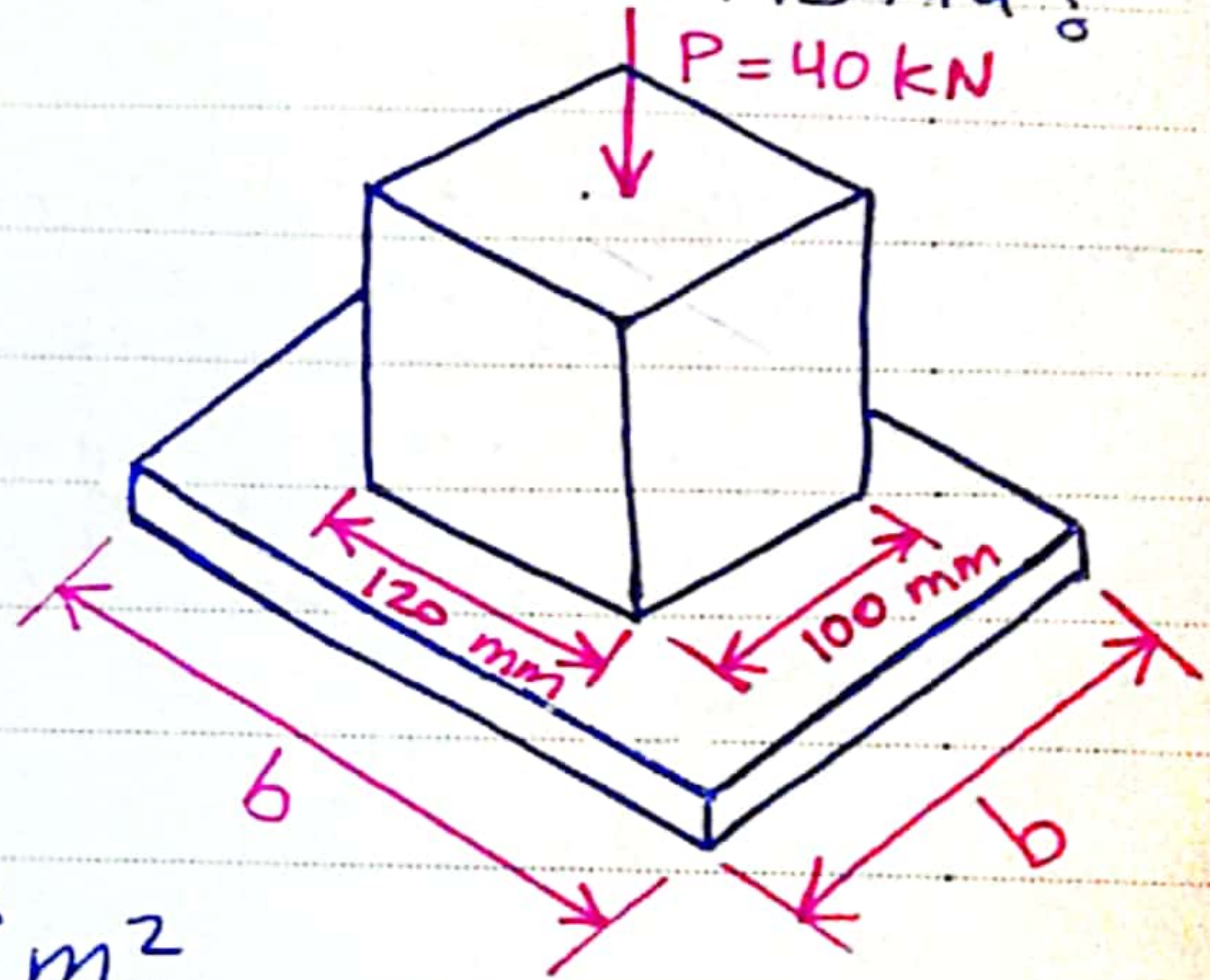
But, $A_b = \frac{\pi}{4} (d^2 - d_i^2)$

$\sqrt{d^2} = d_i + \frac{4A_b}{\pi} = \sqrt{(0.025)^2 + \frac{4(2.6609 \times 10^{-3})}{\pi}}$

$63.3 \times 10^{-3} \text{ m} \rightarrow d = 63.3 \text{ mm}$

1.22 A 40 kN axial load is applied to a short wooden post that is supported by a concrete footing resting on undistributed soil. Determine: a) the max. bearing stress on the concrete footing b) the size of the footing for which the average bearing stress in the soil is 145 kPa?

Ans: a) 3.33 MPa
b) 525 mm



a- Bearing stress on concrete footing

$$P = 40 \text{ kN}$$

$$A = 100 \times 120 = 12 \times 10^{-3} \text{ m}^2$$

$$\sigma = \frac{40 \times 10^3 \text{ N}}{12 \times 10^{-3} \text{ m}^2} = 3.33 \text{ MPa} \quad \#$$

b- footing area

$$A = \frac{P}{\sigma} = \frac{40 \times 10^3 \text{ N}}{145 \times 10^3 \text{ Pa}} = 0.27586 \text{ m}^2$$

Since the area is square, $A = b^2$ قانون مساحة المربع

$$b = \sqrt{A} = 0.525 \text{ m} = 525 \text{ mm} \quad \#$$

Ch.2 Stress & Strain - Axial Loading

↪ أشكال الإجهاد
bending / shear stresses

2.1 Introduction

In Chap 1, we learned how to determine the stresses created in members and connections by the loads applied to a structure or machine.

But indeed, it's not always possible to determine the forces in the members of a structure by applying only the principles of statics.

Hence, in this chapter we will learn the analysis of deformation that will help us in the determination of stresses.

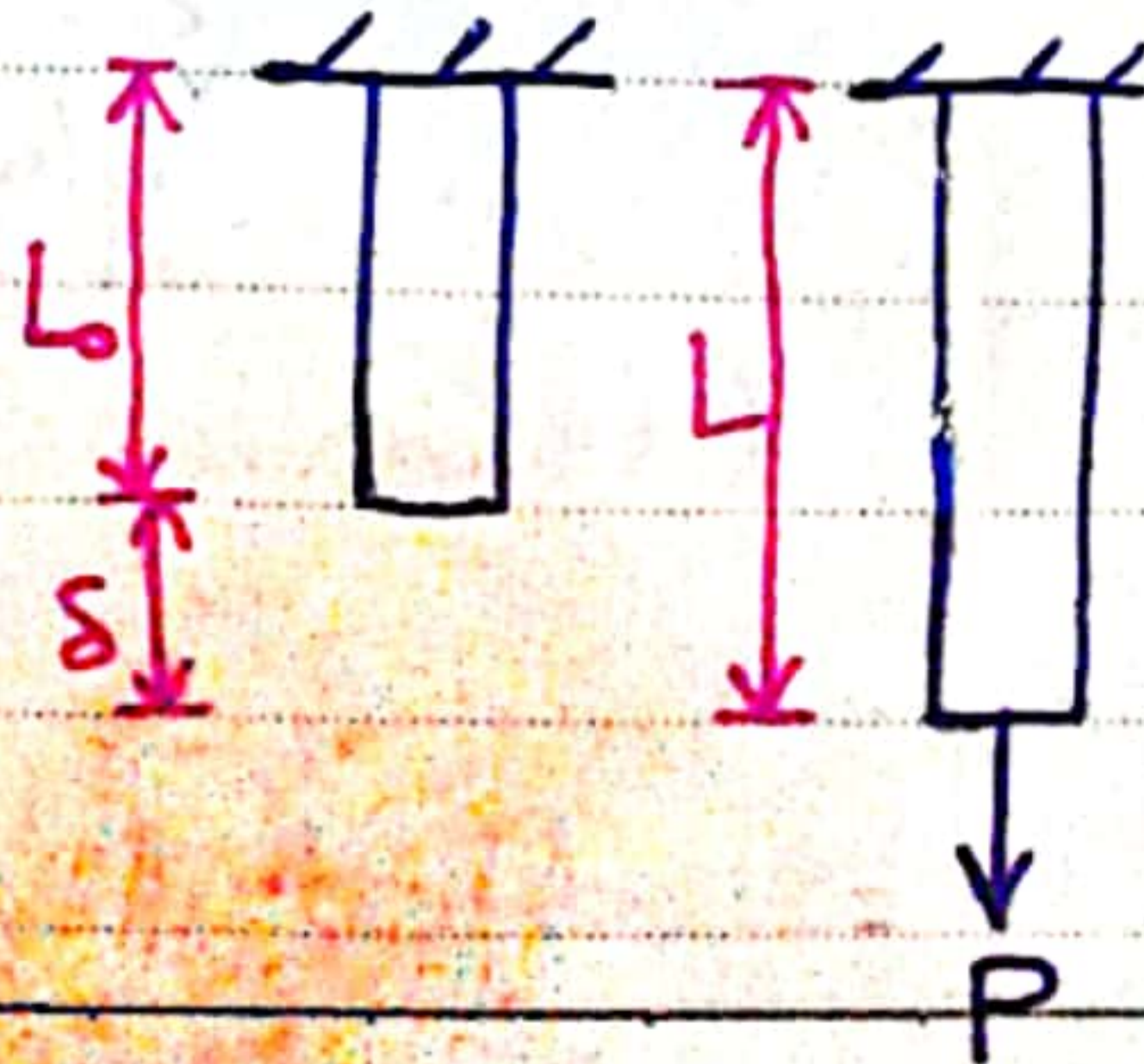
2.2 Normal Strain Under Axial Loading

the normal strain in a rod under axial loading (ϵ) \equiv the deformation per unit length of that rod

$$\delta (\text{deformation}) = L - L_0$$

$$\epsilon = \frac{\delta}{L_0}$$

↪ initial length

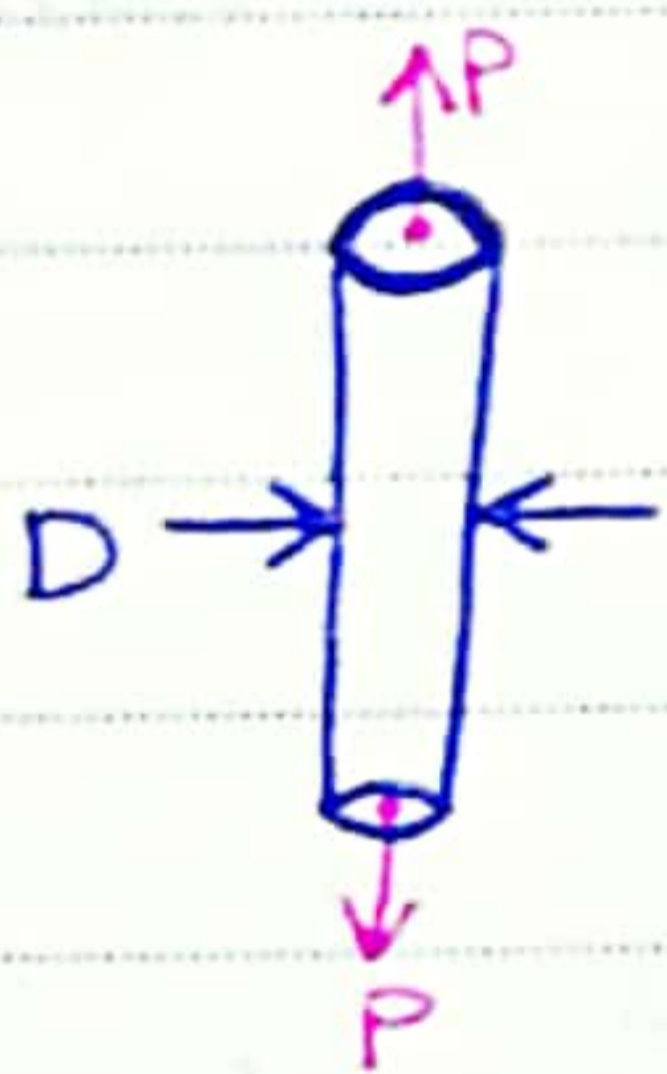


Consider, for instance, a bar of length $L = 0.6 \text{ m}$ and uniform cross section, which undergoes a deformation $\delta = 150 \times 10^{-6} \text{ m}$. The corresponding strain is:

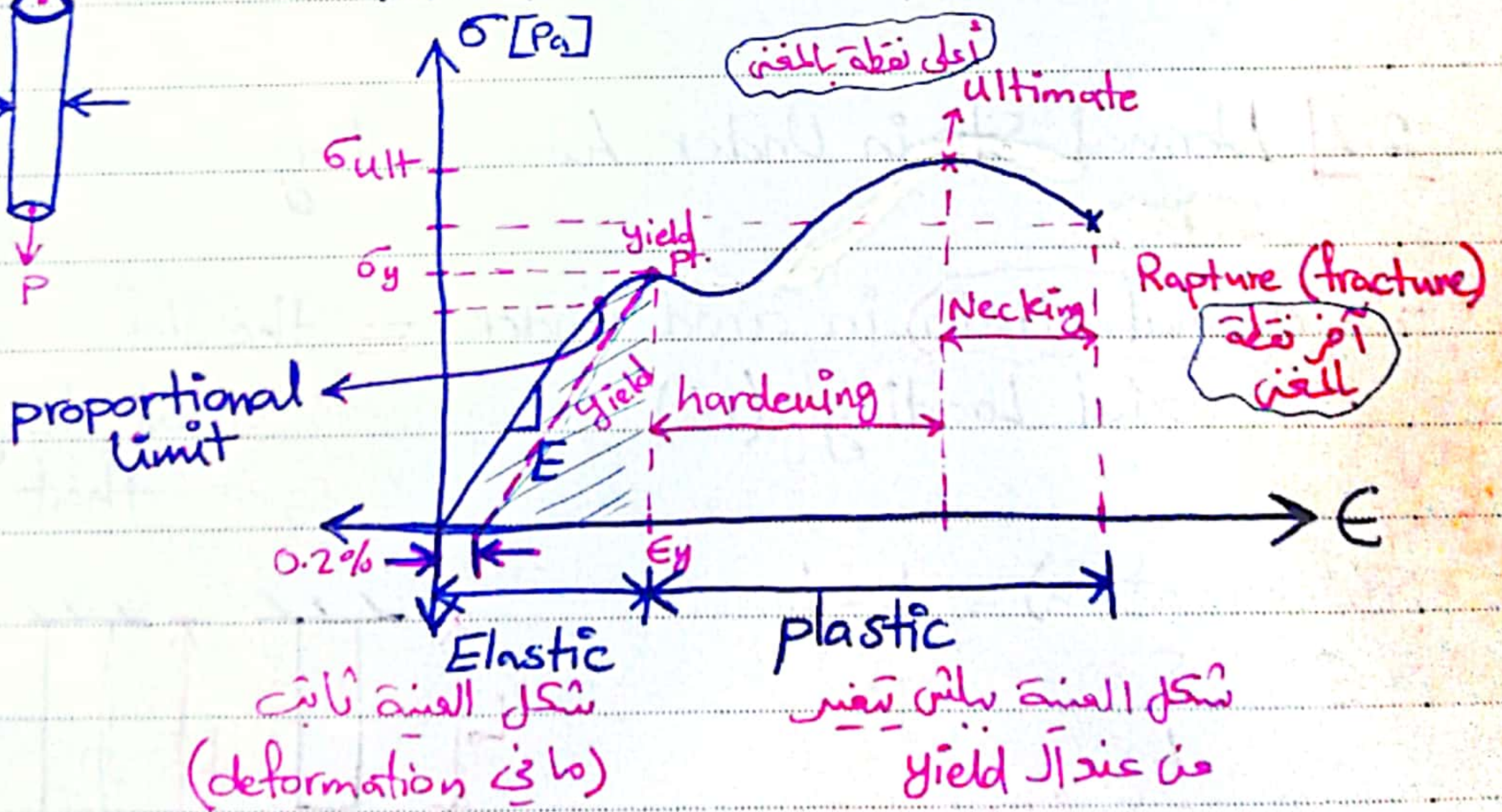
$$\epsilon = \frac{\delta}{L_0} = \frac{150 \times 10^{-6} \text{ [m]}}{0.6 \text{ [m]}} = 250 \times 10^{-6}$$

2.3 | Stress-Strain Diagram

It's used to determine some important characteristics for the materials like
 [E , material type, brittle or ductile, elastic or plastic deformation, ...]



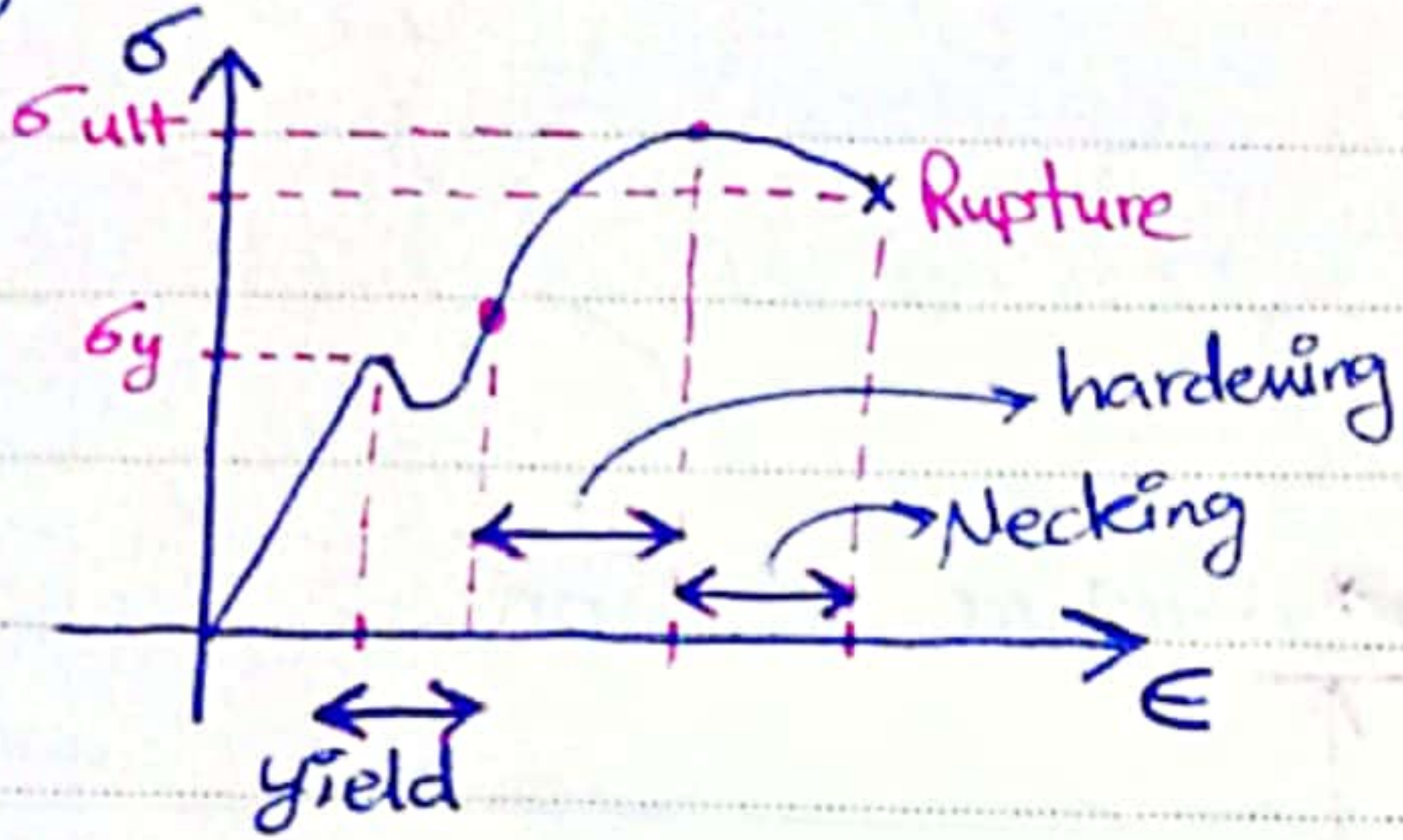
• For a tension force, ductile materials also



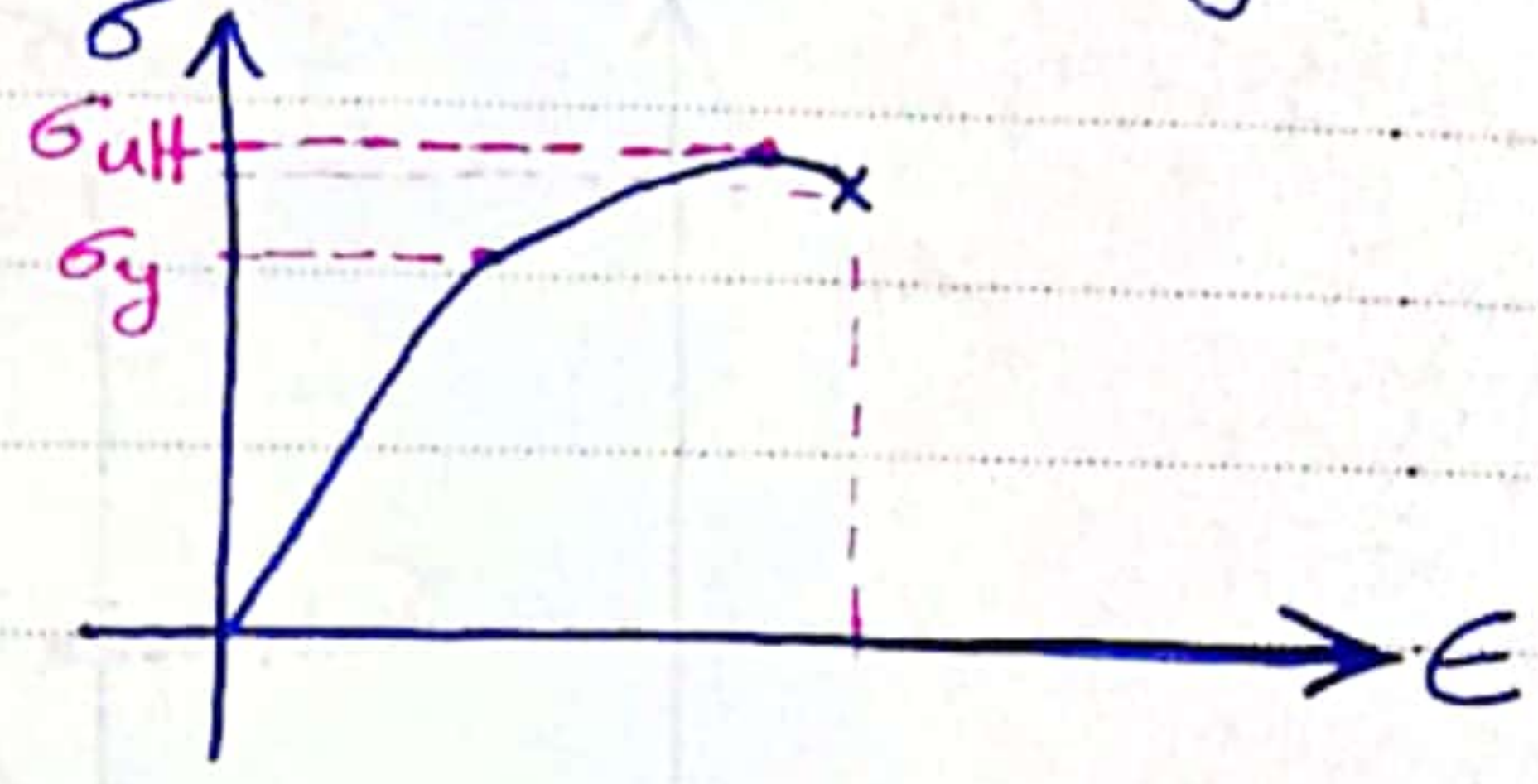
0.2% \equiv an offset to determine the yield strength

● Ductile Materials →

a) Low Carbon Steel

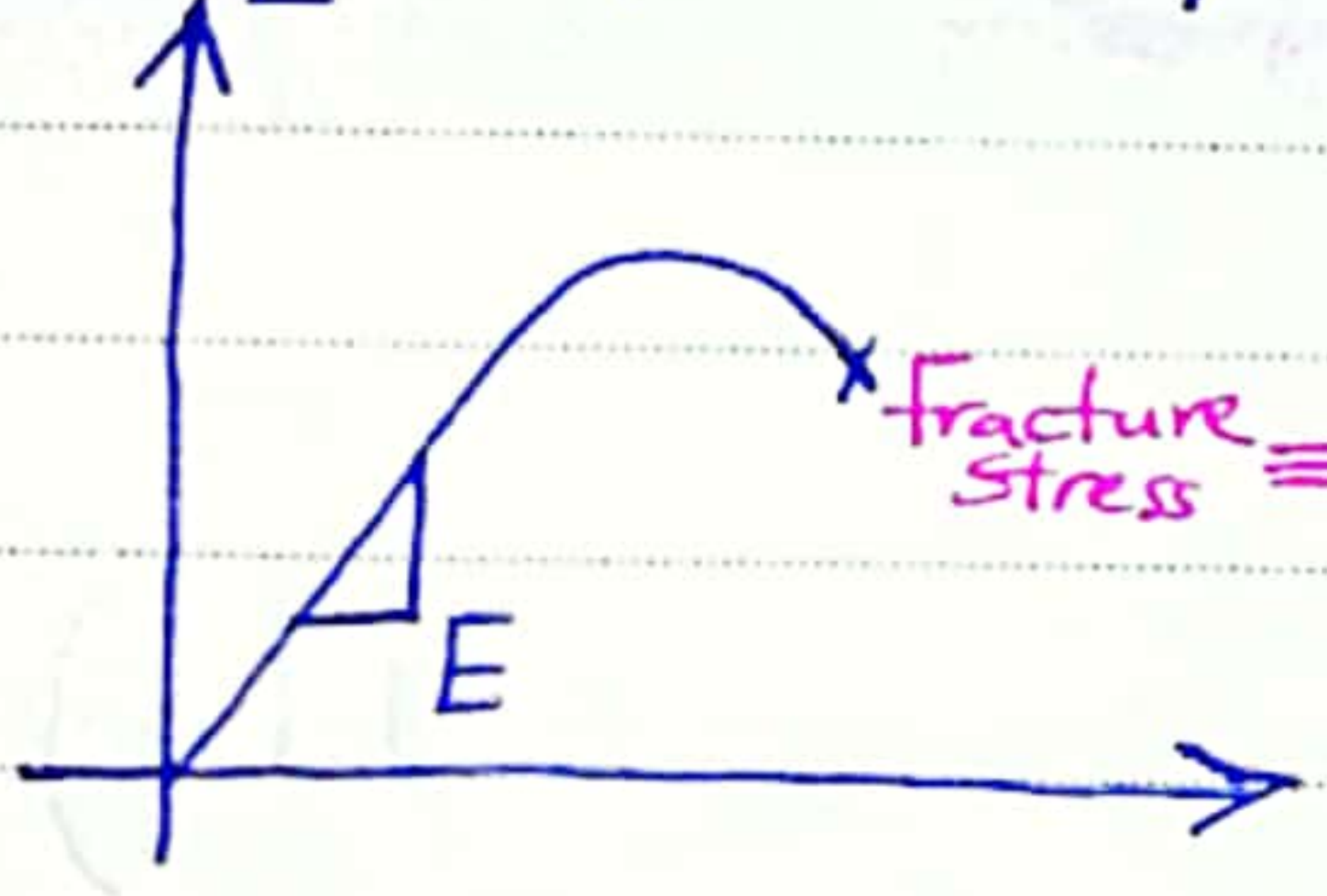


b) Aluminum Alloy



● Brittle Materials →

Like [Brass / Concrete / ...]



اللي كالتالي

Fracture stress \equiv Yield \equiv Ultimate

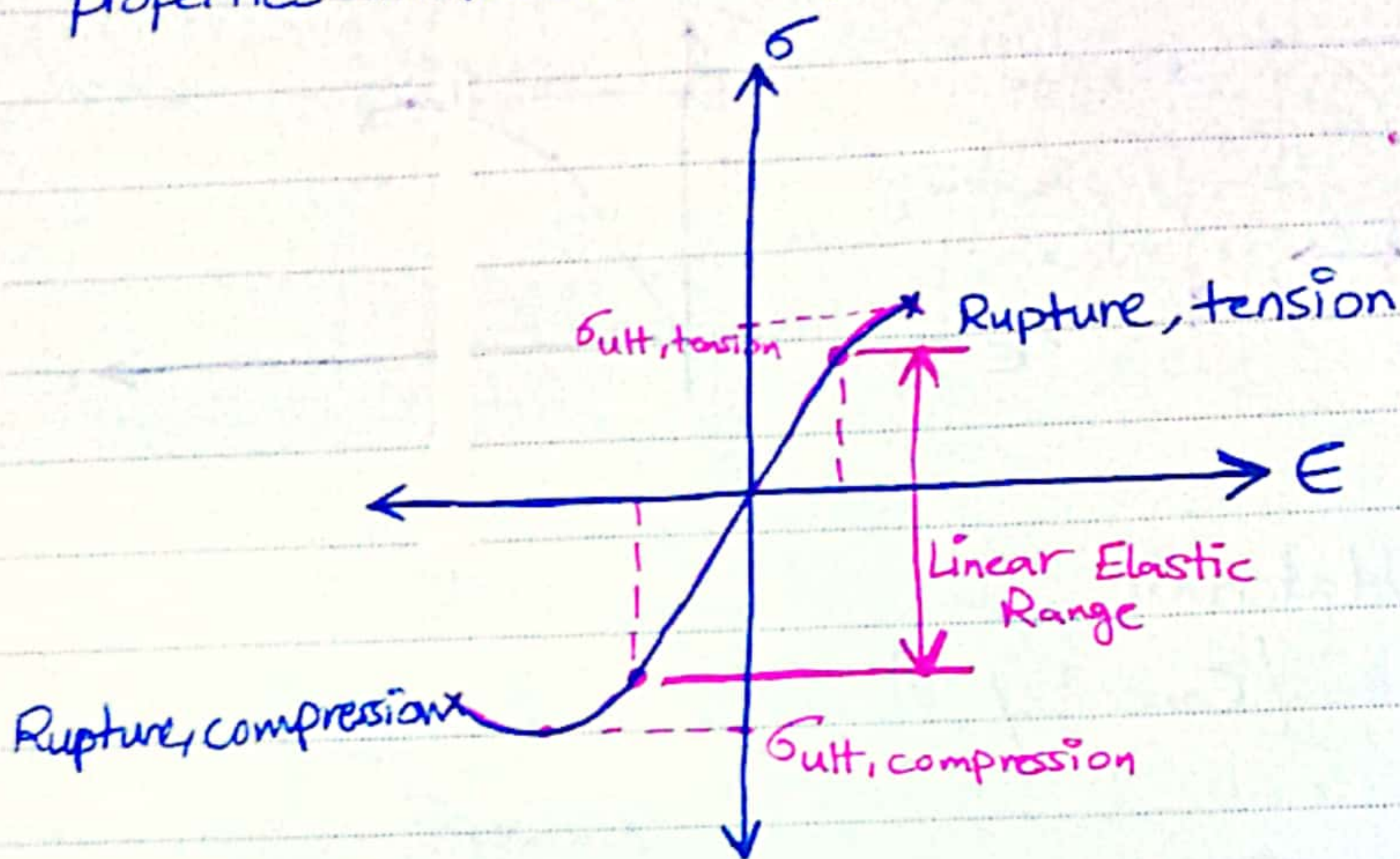
⇒ $\sigma_{ult} / \text{Compression} > \sigma_{ult} / \text{tension}$

• due to the presence of flaws such as microscopic cracks or cavities, which tend to weaken the material in tension ...

Fracture shape in the tension → ductile (Necking, "Cup & Cone", 45°)
 → brittle (Flattening 90°)

due to ↓ the normal stress

an example of brittle material with different properties in tension & compression is provided by "concrete"



2.5] Hooke's Law

- E (Modulus of Elasticity) = (Young's Modulus) \Rightarrow

$$\sigma = E \epsilon \quad \text{Hooke's Law}$$

(E) مَبْدَلُ الْمَبْدَلِ [slope] عِنْدَ $(\sigma - \epsilon)$ فِي مَنطِقَةِ الْعِلَاقَةِ (elastic) مَوْجِدَةٌ
 Linear Region

2.7 Repeated Loadings ; Fatigue

Fatigue \equiv loading are repeated thousands or millions of times on a specimen

In such cases, rupture will occur at a stress much lower than the static breaking strength.

2.8 Deformations of Members Under Axial Loading

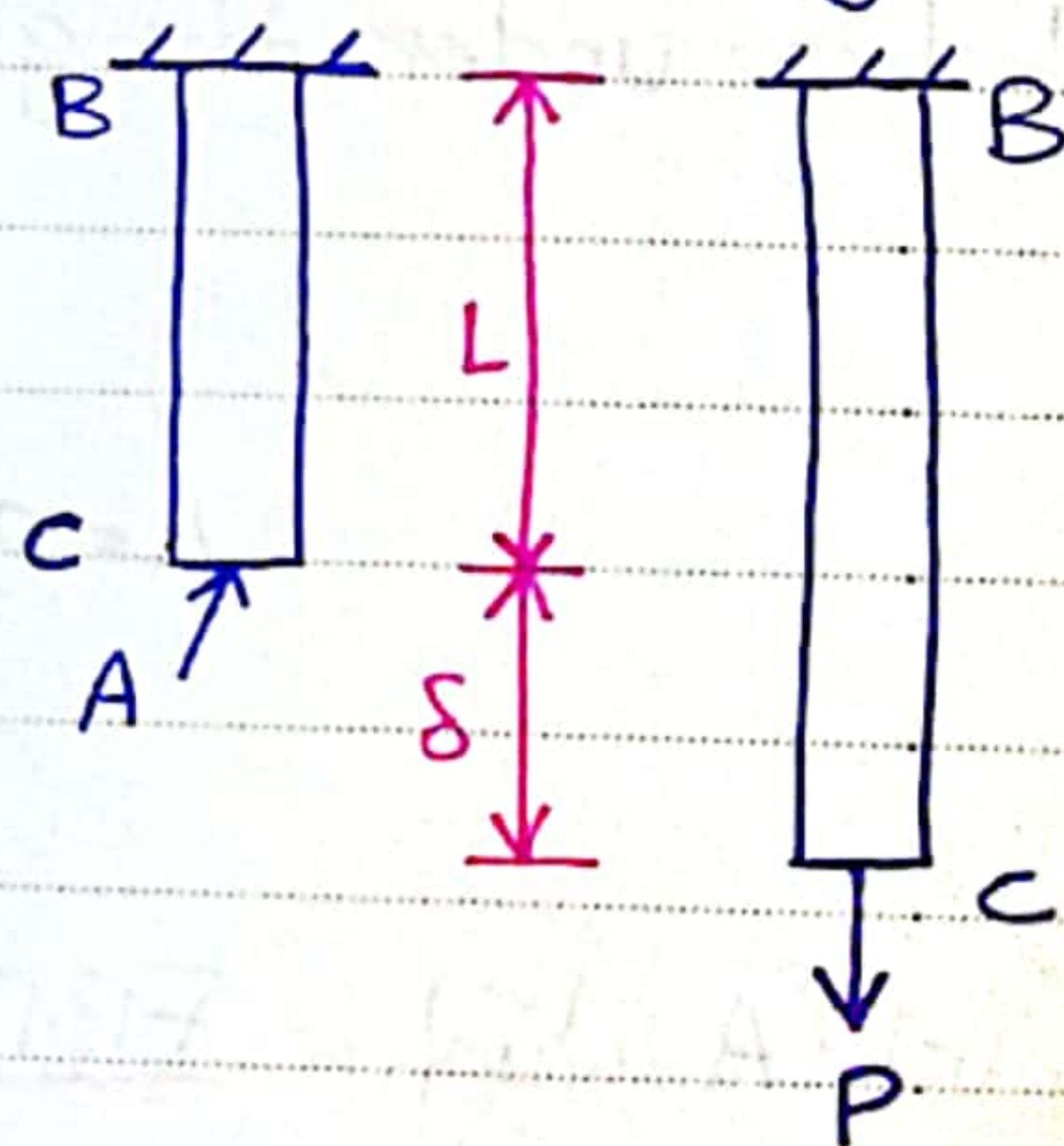
If the resulting axial stress

$$\sigma = \frac{P}{A}$$

doesn't exceed the proportional limit [elastic] [Linear]

$$\sigma = E \epsilon$$

$$\left[\epsilon = \frac{\sigma}{E} = \frac{P}{AE} \right]$$



$$E = \frac{\delta}{L}$$

∴

$$\delta = \frac{PL}{AE}$$

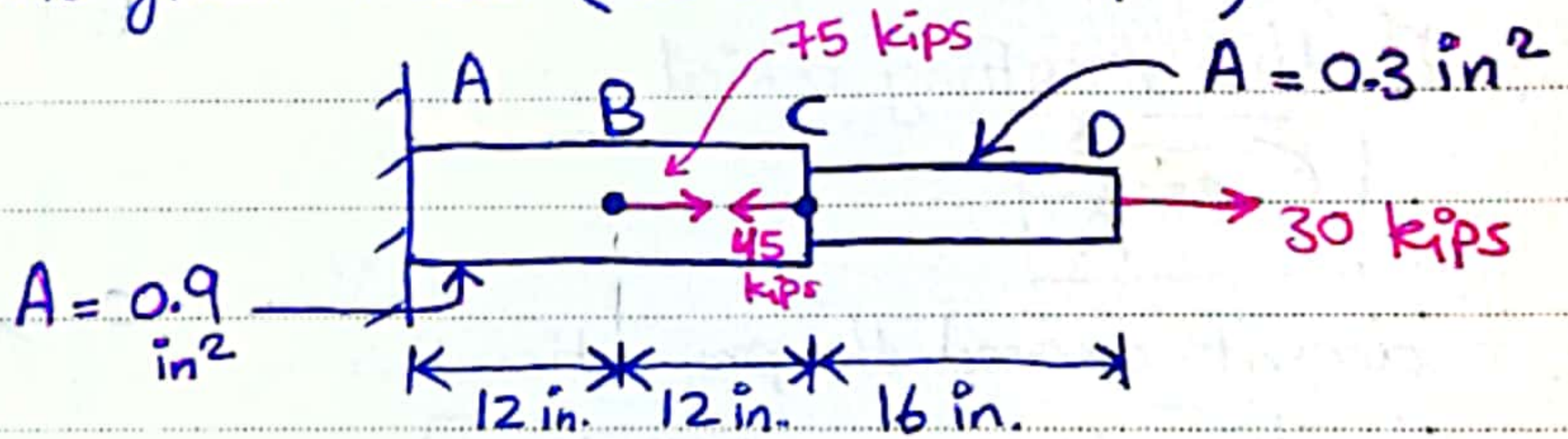
⇒ used only if the rod has a uniform cross section of area A and is loaded at its end.

If the rod is loaded at other points, possible of different materials or it consists of various cross sections, we'll use \Rightarrow

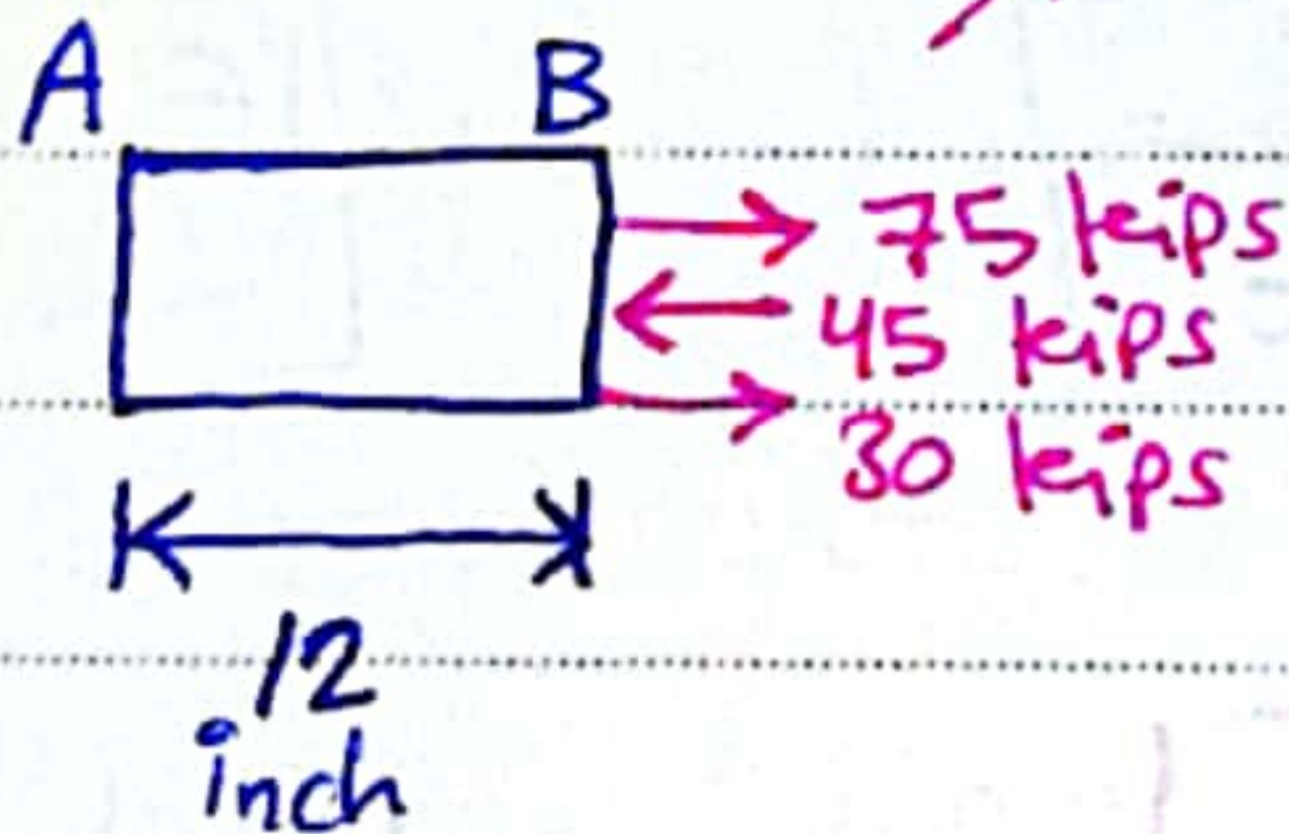
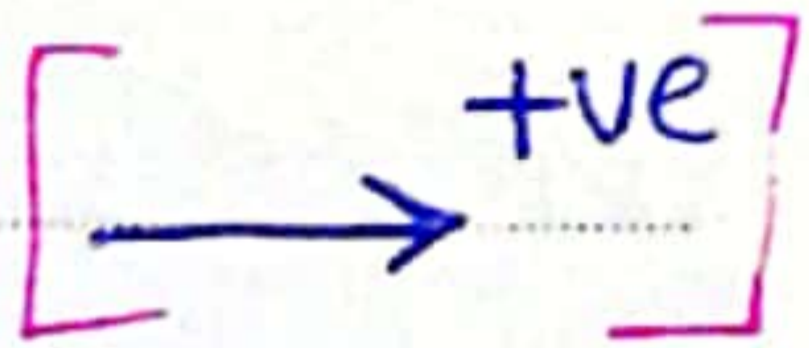
$$\delta = \sum_i \frac{P_i L_i}{A_i E_i}$$

Ex (2.01)

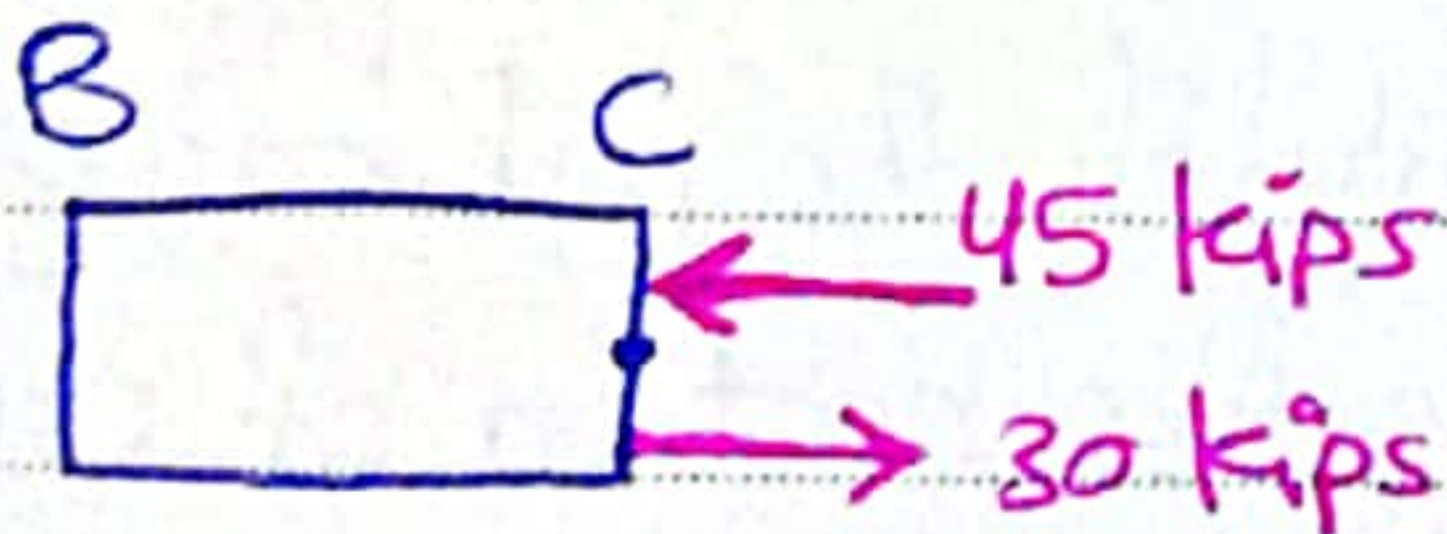
Determine the deformation of the steel rod shown below under the given loads ($E = 29 \times 10^6$ psi)



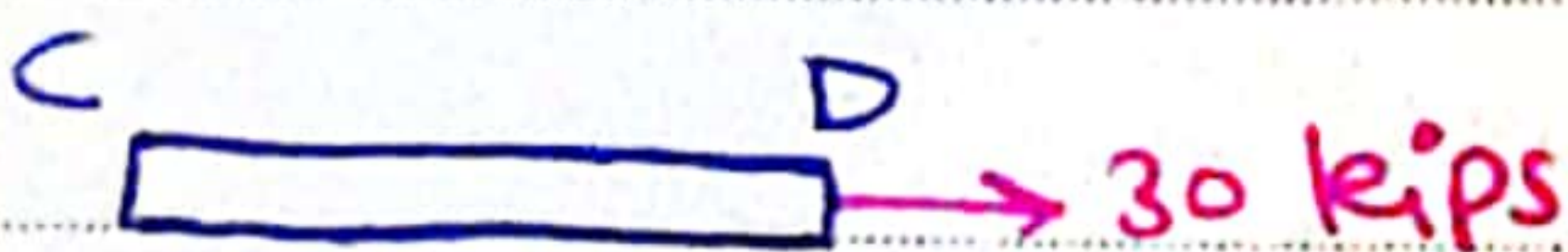
أولاً، نحلل (A) كجزء من FBD أو كل [Component Part] من أجل *
 ولأنه ليس في موضع معين



$$P_{AB} = 75 - 45 + 30 = 60 \text{ kips}$$



$$P_{BC} = 30 - 45 = -15 \text{ kips}$$



$$P_{CD} = 30 \text{ kips}$$

$$\therefore \delta = \frac{P_1 L_1}{A_1 E} + \frac{P_2 L_2}{A_2 E} + \frac{P_3 L_3}{A_3 E}$$

$$A_1 = A_2 = 0.9 \text{ in}^2$$

$$L_1 = L_2 = 12 \text{ in.}$$

$$L_3 = 16 \text{ in.}$$

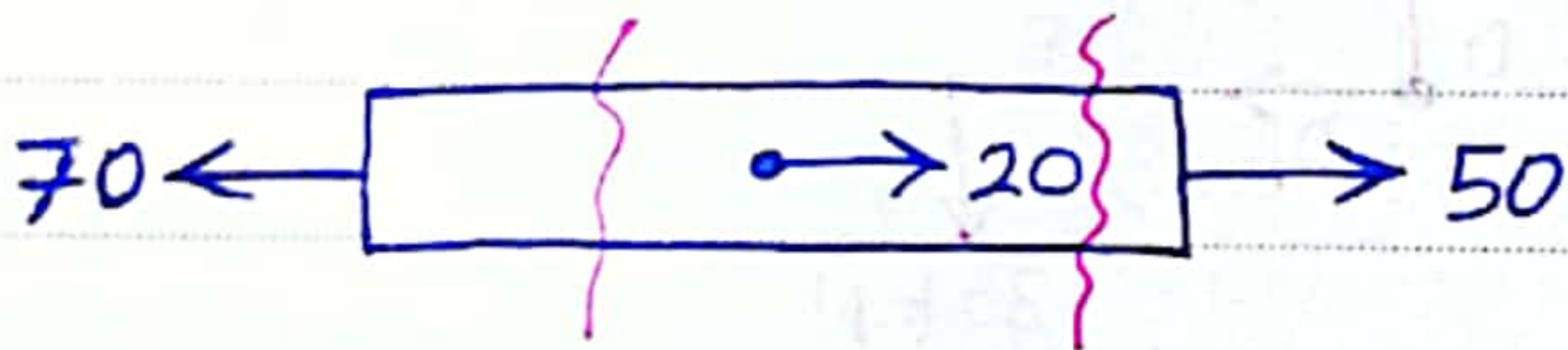
$$A_3 = 0.3 \text{ in}^2$$

$$\Rightarrow \delta = \frac{60 \times 10^3 \times 12}{0.9 \times 29 \times 10^6} + \frac{-15 \times 10^3 \times 12}{0.9 \times 29 \times 10^6} + \frac{30 \times 10^3 \times 16}{0.3 \times 29 \times 10^6}$$

$$\delta = 75.9 \times 10^{-3} \text{ in.}$$

#

ex →



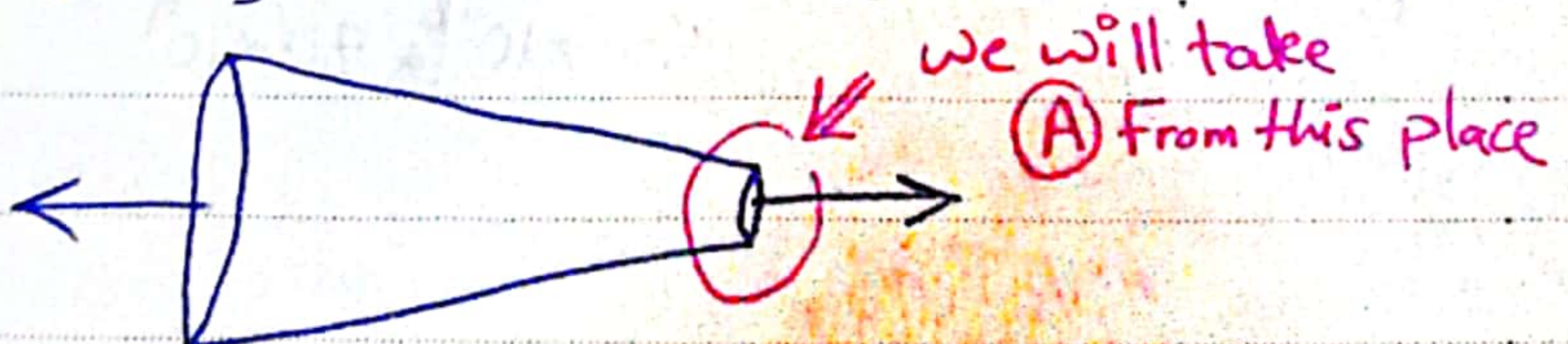
لو طبق في أي قسم (σ_{normal})

take a section at each pt. where the internal force or the cross section area has changed.

* كلما زادت A إلى حد ما stress أقل ولأنه يتم حساب stress

في حالة max. في مكان آخر أنه يتغير

وإننا نأخذ A عند أقل مكان/ قسم لها ، هكذا

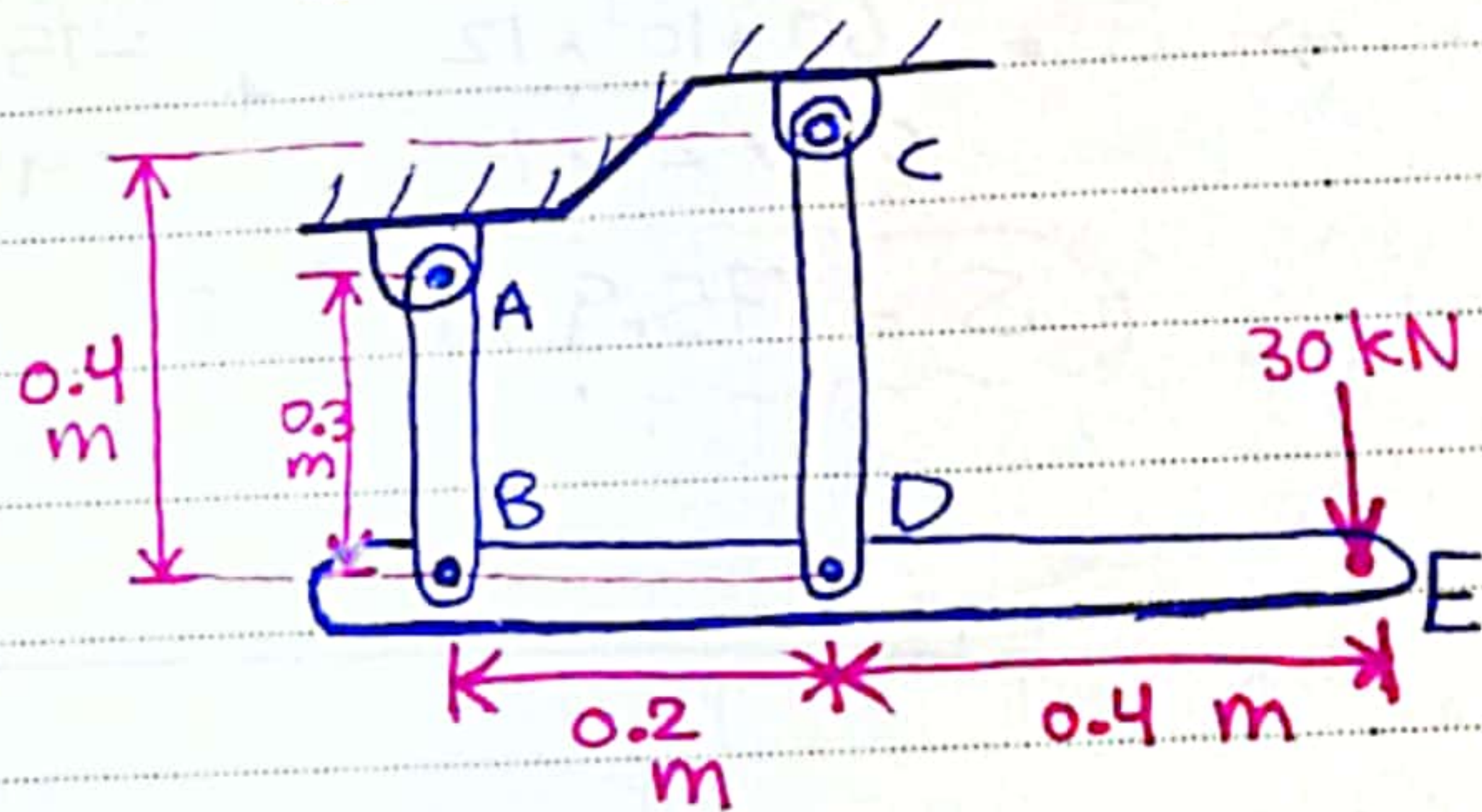


Link CD is made of steel ($E=200\text{GPa}$) and has across sectional area of 600mm^2 .

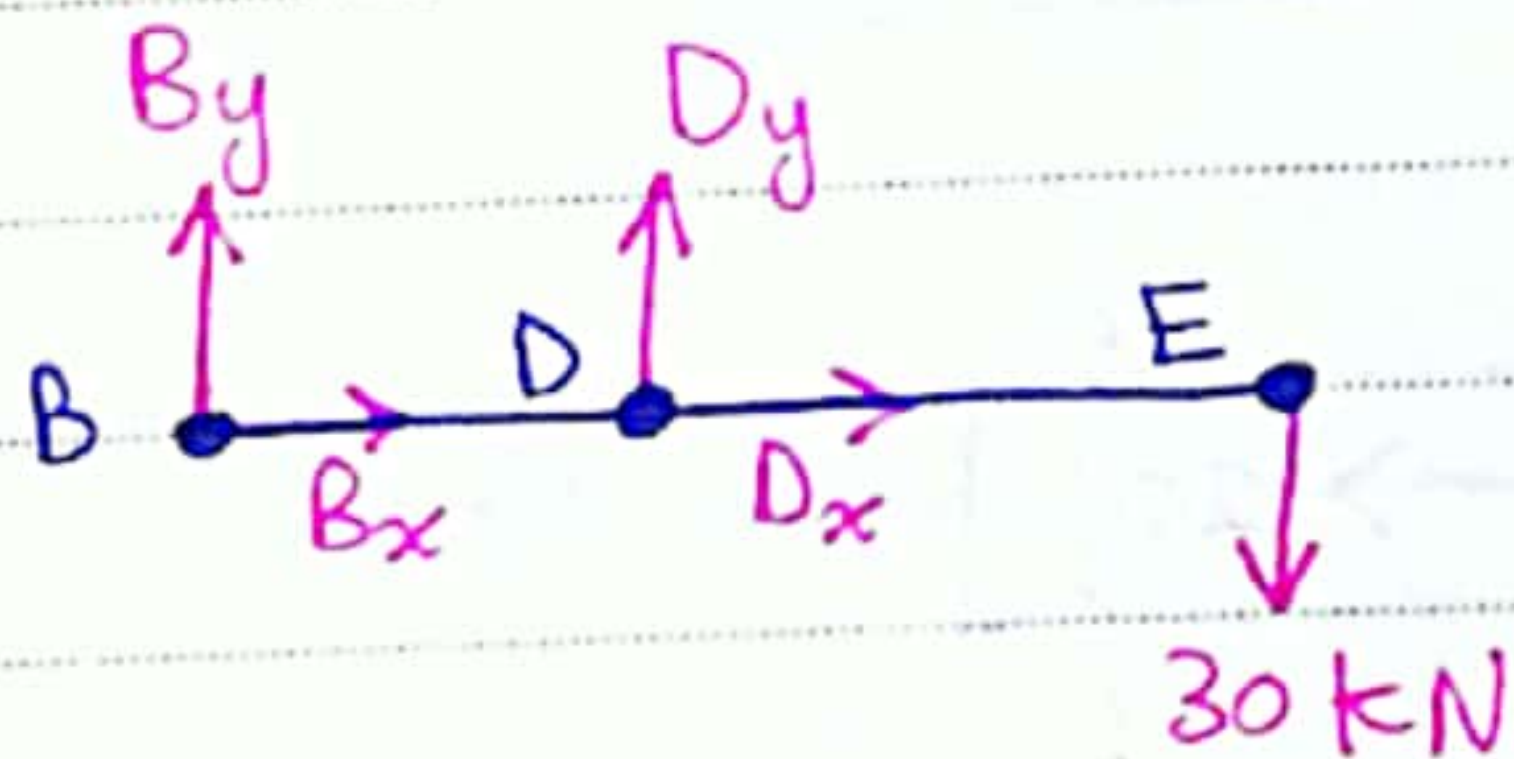
Sample Problem (2.1) \Rightarrow

The rigid bar BDE is supported by two links AB and CD. Link AB is made of aluminum ($E=70\text{GPa}$) and has across sectional area of 500mm^2 ; For the 30kN force shown, determine the deflection of ∞

- a) B b) D c) E



Sol



$$\curvearrowright \sum M_D = 0, \quad 30,000(0.4) + B_y(0.2) = 0$$

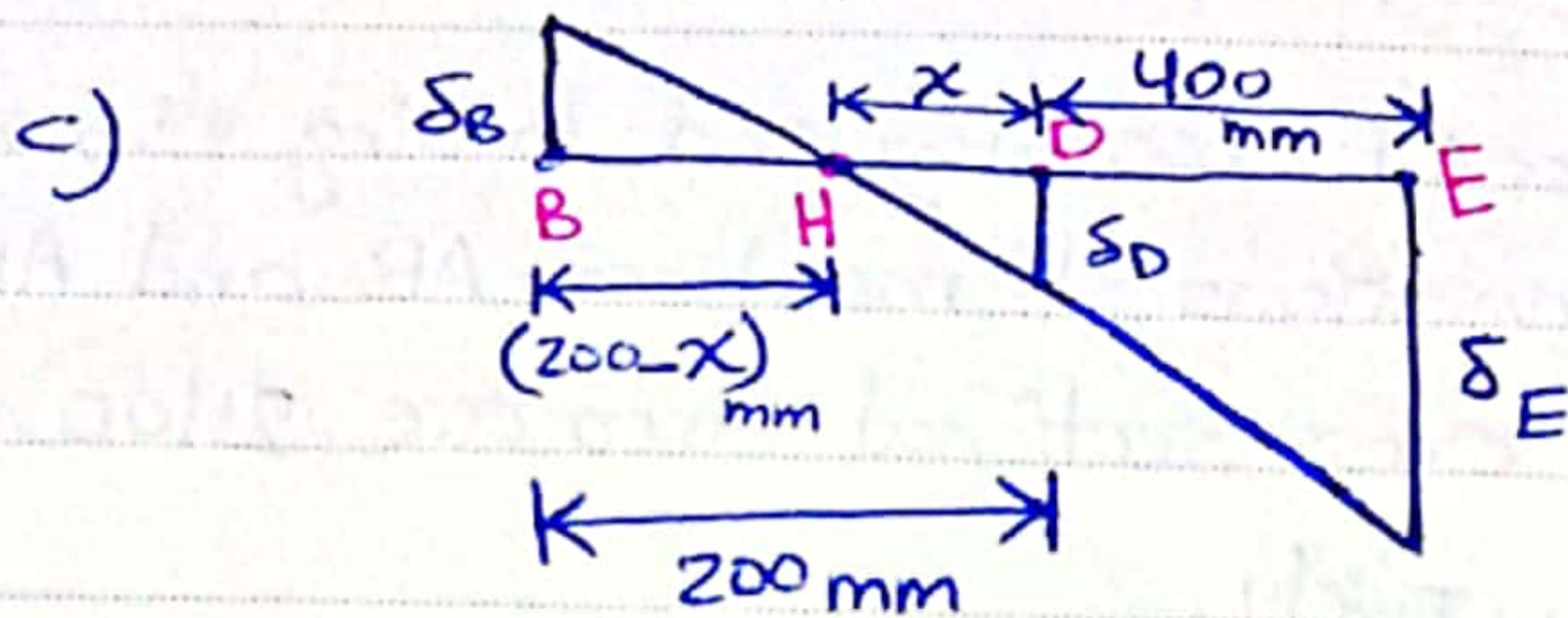
$$B_y = F_{AB} = -60\text{ kN} \quad \text{compression}$$

$$\curvearrowright \sum M_B = 0, \quad -D_y(0.2) + 30,000(0.6) = 0$$

$$D_y = F_{CD} = 90\text{ kN} \quad \text{tension}$$

$$a) \delta_B = \frac{P_{AB} L_{AB}}{A_{AB} E_{AB}} = \frac{-60 \times 10^3 \times 0.3}{500 \times 10^{-6} \times 70 \times 10^9} = -5.14 \times 10^{-4} \text{ m}$$

$$b) \delta_D = \frac{P_{co} L_{co}}{A_{co} E_{co}} = \frac{90 \times 10^3 \times 0.4}{600 \times 10^{-6} \times 200 \times 10^9} = 3 \times 10^{-4} \text{ m}$$



$$\delta_D = 0.3 \text{ mm}$$

$$\delta_B = 0.514 \text{ mm}$$

$$\frac{\delta_B}{\delta_D} = \frac{200-x}{x}$$

[قانون
تساوي المثلثات]

$$0.514 x = 0.3 (200-x)$$

$$(0.514 + 0.3) x = 0.3 \times 200$$

$$x = 73.7 \text{ mm}$$

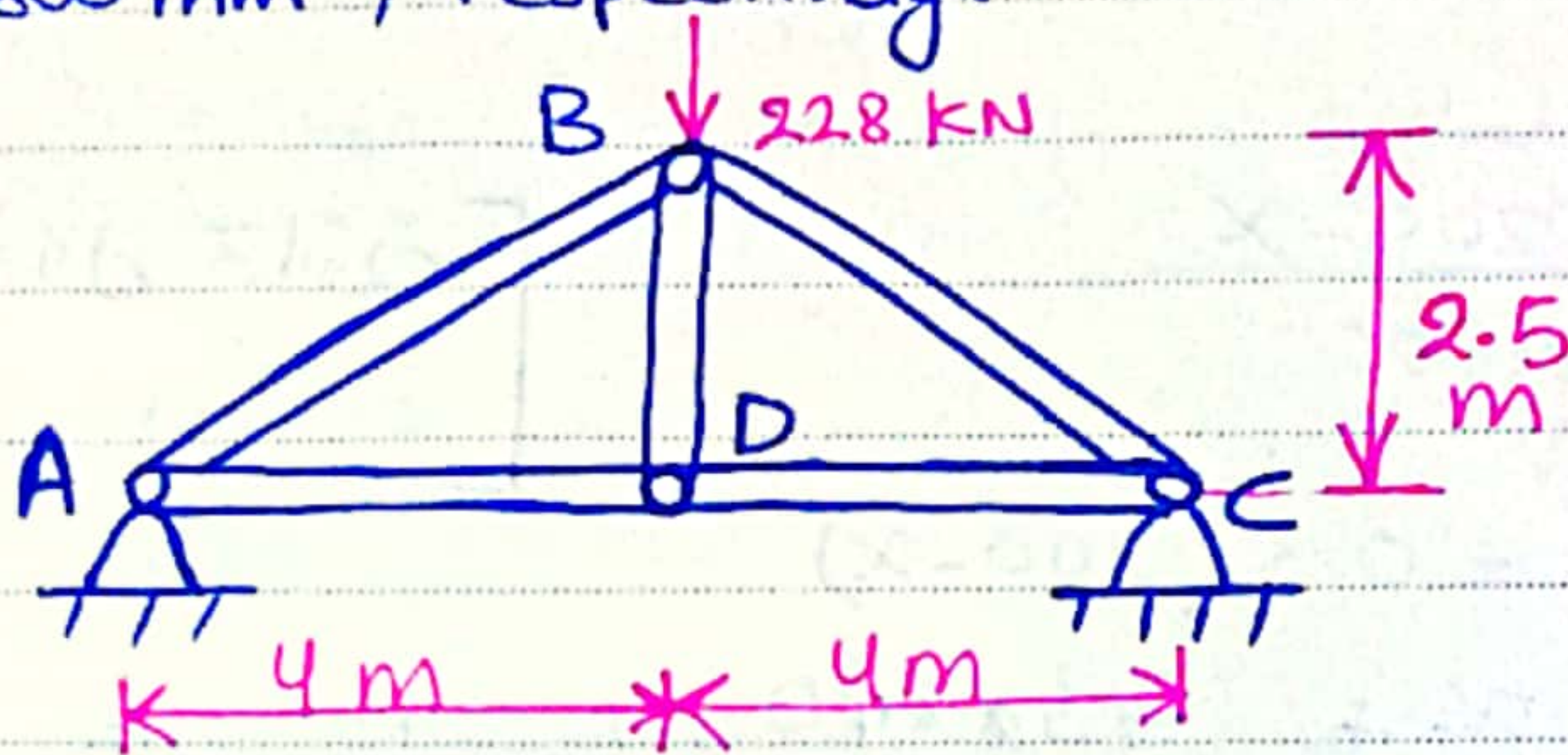
$$\frac{\delta_E}{\delta_D} = \frac{400 + 73.7}{73.7} \Rightarrow$$

$$\delta_E = 1.928 \text{ mm}$$

#

Problems

2.23) For the steel truss ($E=200 \text{ GPa}$) and loading shown, determine the deformations of members AB and AD, knowing that their cross sectional area are 2400 mm^2 and 1800 mm^2 , respectively.



Statics : Reactions are 114 kN upward at A and C
Member BD is a zero force member.

$$L_{AB} = \sqrt{4^2 + 2.5^2} = 4.717 \text{ m}$$

Use joint A as a free body

$$\uparrow \sum F_y = 0 : 114 + \frac{2.5}{4.717} F_{AB} = 0$$

$$F_{AB} = -215.10 \text{ kN}$$

$$\rightarrow \sum F_x = 0 : F_{AD} + \frac{4}{4.717} F_{AB} = 0$$

$$F_{AD} = 182.4 \text{ kN}$$

Member AB :

$$\delta_{AB} = \frac{F_{AB} L_{AB}}{E A_{AB}}$$

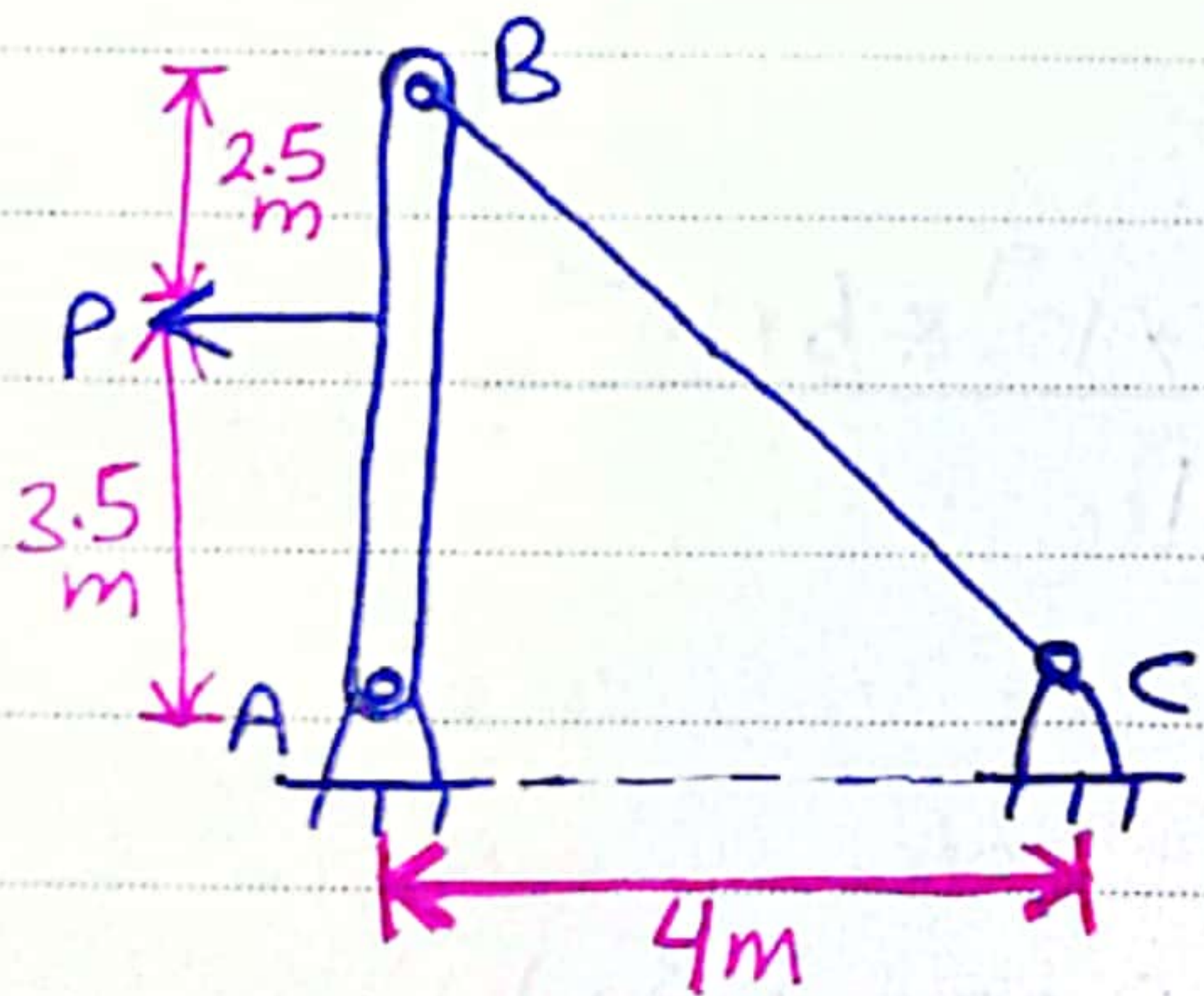
$$= \frac{-215.10 \times 10^3 \times 4.717}{200 \times 10^9 \times 2400 \times 10^{-6}} = -2.11 \times 10^{-3} \text{ m}$$

Member AD :

$$\delta_{AD} = \frac{F_{AD} L_{AD}}{E A_{AD}}$$

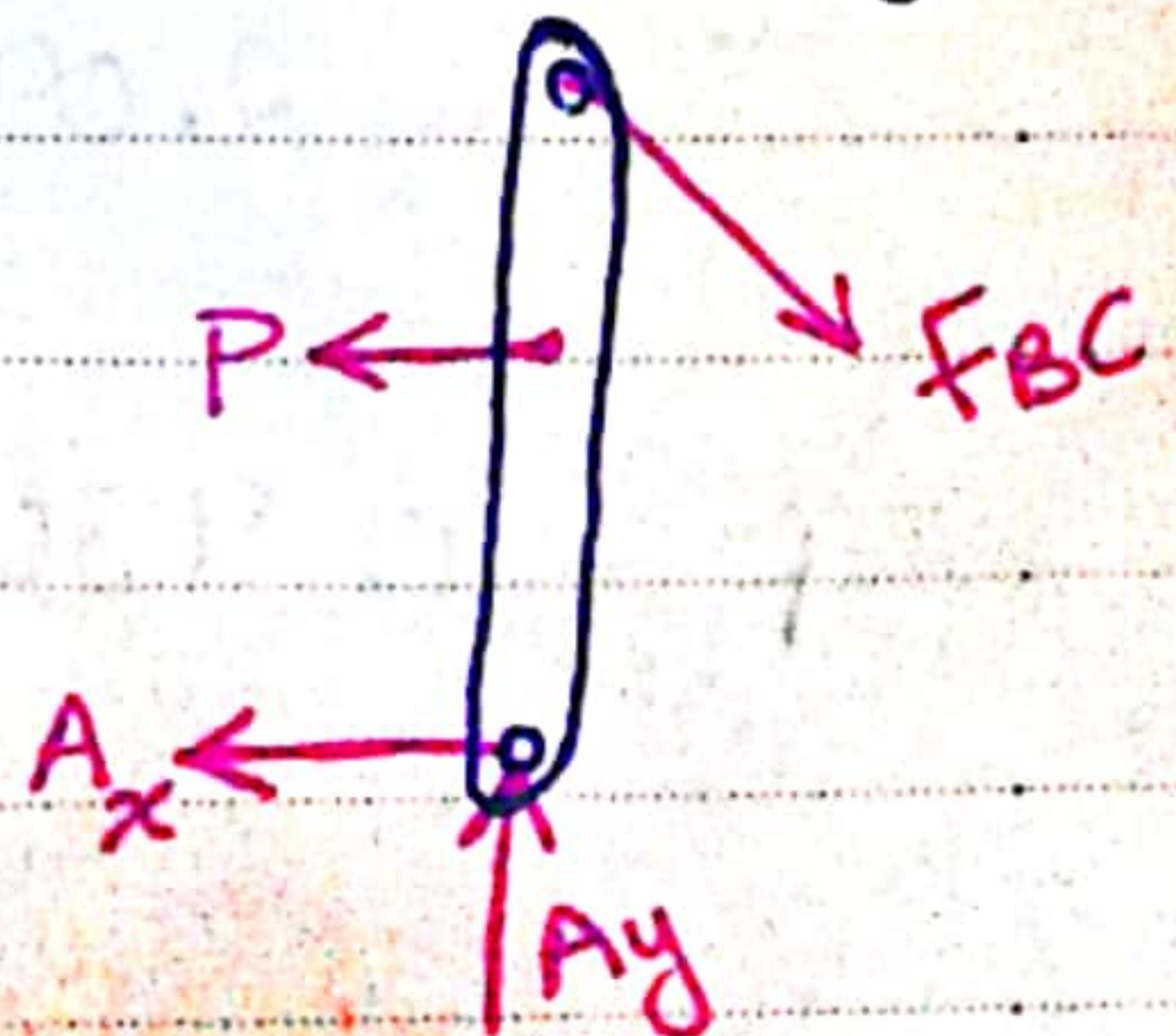
$$= \frac{182.4 \times 10^3 \times 4}{200 \times 10^9 \times 1800 \times 10^{-6}} = 2.03 \times 10^{-3} \text{ m}$$

2.13) The 4mm diameter cable BC is made of a steel with $E = 200 \text{ GPa}$. knowing that the max. stress in the cable must not exceed 190 MPa and that the elongation of the cable must not exceed 6 mm , find the max. load p that can be applied as shown :



$$L_{BC} = \sqrt{6^2 + 4^2} = 7.211 \text{ m}$$

use AB as a free body :



$$\text{K} \rightarrow \sum M_A = 0$$

$$3.5P - 6 * \frac{4}{7.2111} F_{BC} = 0$$

$$P = 0.9509 F_{BC}$$

* Considering allowable stress: $\sigma = 190 \times 10^6 \text{ Pa}$

$$A = \frac{\pi}{4} d^2 = \frac{\pi}{4} (0.004)^2 = 12.566 \times 10^{-6} \text{ m}^2$$

$$\sigma = \frac{F_{BC}}{A}$$

$$\therefore F_{BC} = \sigma A = 190 \times 10^6 * 12.566 \times 10^{-6} \\ = 2.388 \times 10^3 \text{ N}$$

* Considering allowable elongation: $\delta = 6 \times 10^{-3} \text{ m}$

$$\delta = \frac{F_{BC} L_{BC}}{E A_{BC}}$$

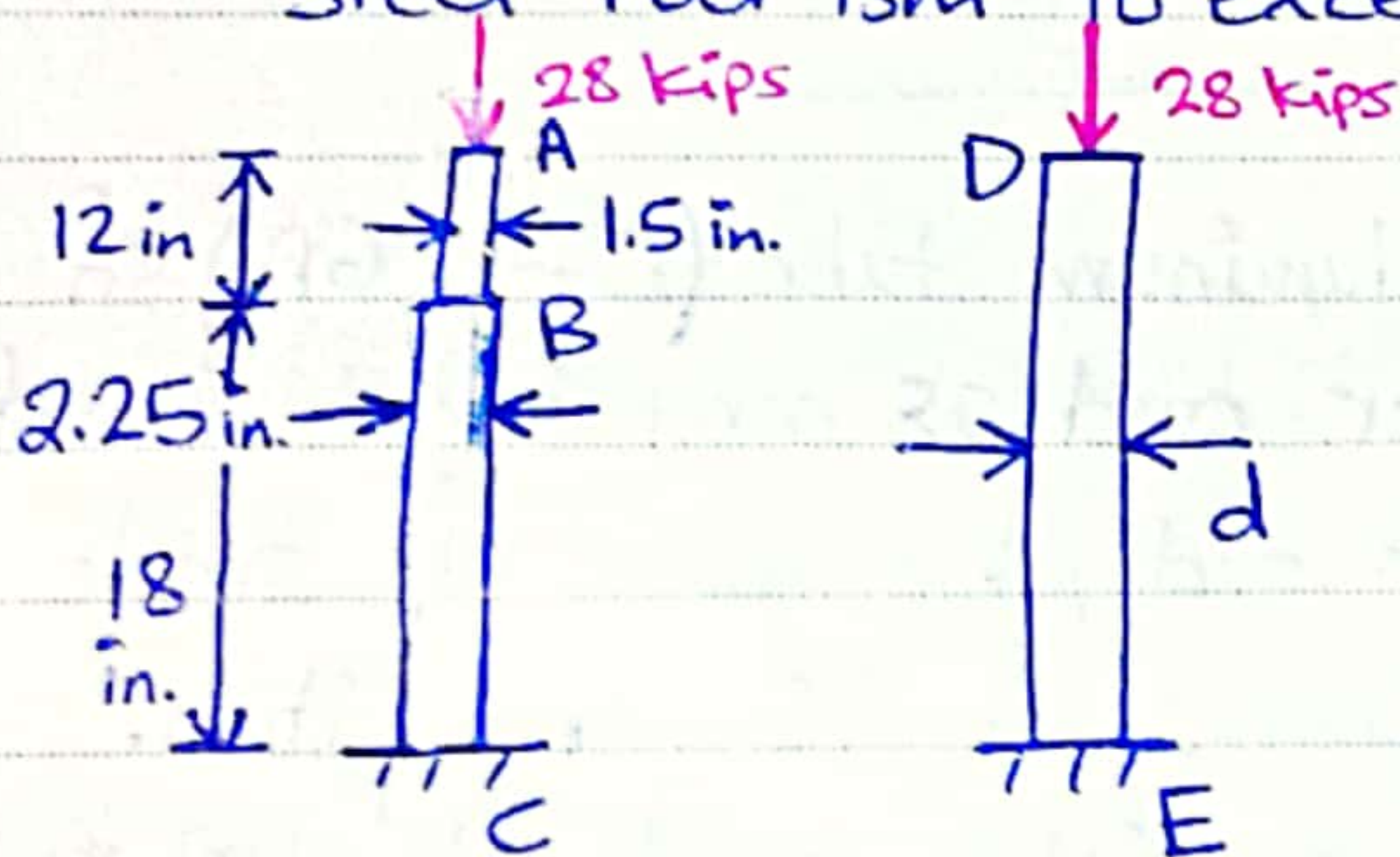
$$F_{BC} = \frac{12.566 \times 10^{-6} * 200 \times 10^9 * 6 \times 10^{-3}}{7.2111}$$

$$= 2.091 \times 10^3 \text{ N}$$

$$P = 0.9509 F_{BC} = 1.988 \times 10^3 \text{ N}$$

#

2.14) The aluminum rod ABC ($E = 10.1 \times 10^6$ psi), which consists of two cylindrical portions AB and BC, is to be replaced with a cylindrical steel rod DE ($E = 29 \times 10^6$ psi) of the same overall length. Determine the min. required diameter d of the steel rod if its vertical deformation isn't to exceed the deformation of the aluminum rod under the same load and if the allowable stress in the steel rod isn't to exceed 24 ksi.



→ Deformation of aluminum rod

$$\begin{aligned} \delta_A &= \frac{PL_{AB}}{A_{AB}E} + \frac{PL_{BC}}{A_{BC}E} \\ &= \frac{28 \times 10^3}{10.1 \times 10^6} \left[\frac{12}{\frac{\pi}{4}(1.5)^2} + \frac{18}{\frac{\pi}{4}(2.25)^2} \right] \\ &= 0.031376 \text{ in.} \end{aligned}$$

Steel rod, $[\delta = 0.031376 \text{ inch}]$

$$\delta = \frac{PL}{EA} \Rightarrow A = \frac{28 \times 10^3 \times 30}{29 \times 10^6 \times 0.031376} = 0.92317 \text{ in}^2$$

$$\sigma = \frac{P}{A} \Rightarrow A = \frac{28 \times 10^3}{24 \times 10^3} = \underline{1.1667 \text{ in}^2}$$

Required area is the larger value

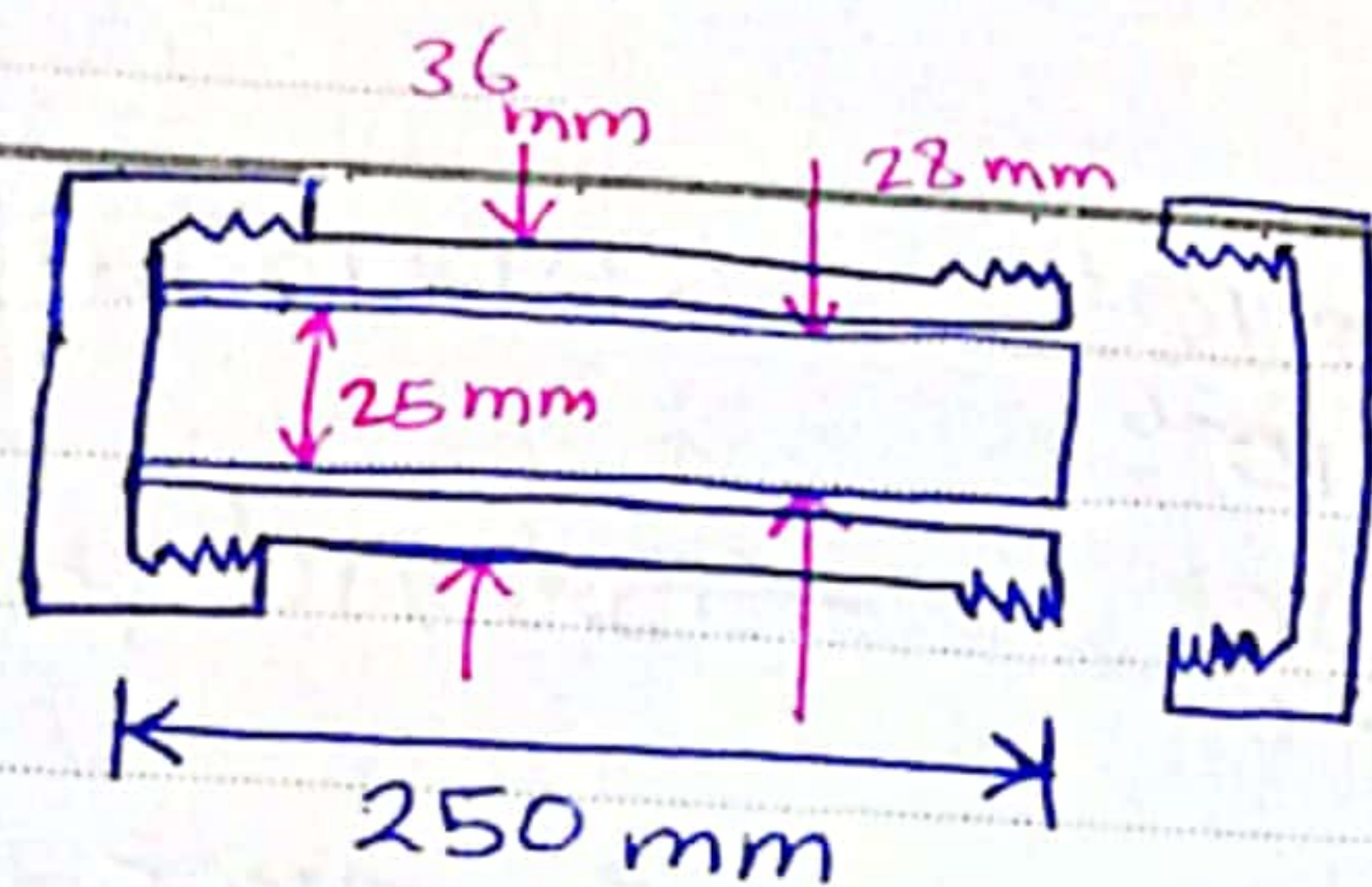
$$\text{Diameter} \Rightarrow d = \sqrt{\frac{4A}{\pi}} = \sqrt{\frac{4(1.1667)}{\pi}}$$

$$d = 1.219 \text{ in.}$$

2.17) A 250 mm long aluminum tube ($E = 70 \text{ GPa}$) of 36 mm outer diameter and 28 mm inner diameter can be closed at both ends by means of single threaded screw-on covers of 1.5 mm pitch.

With one cover screwed on tight, a solid brass rod ($E = 105 \text{ GPa}$) of 25 mm diameter is placed inside the tube and the second cover is screwed on. Since the rod is slightly longer than the tube, it's observed that the cover must be forced against the rod by rotating it one quarter of a turn before it can be tightly closed. Determine:

- the average normal stress in the tube and in the rod?
- the deformations of the tube and of the rod?



$$\rightarrow A_{\text{tube}} = \frac{\pi}{4} (d_o^2 - d_i^2) = \frac{\pi}{4} (36^2 - 28^2) = 402.12 \text{ mm}^2$$

$$A_{\text{rod}} = \frac{\pi}{4} d^2 = \frac{\pi}{4} (25^2) = 490.87 \text{ mm}^2$$

$$\delta_{\text{tube}} = \frac{P L}{E_{\text{tube}} A_{\text{tube}}} = \frac{P(0.25)}{70 \times 10^9 \times 402.12 \times 10^{-6}} = 8.8815 \times 10^{-9} P$$

$$\delta_{\text{rod}} = \frac{-P L}{E_{\text{rod}} A_{\text{rod}}} = \frac{P(0.25)}{105 \times 10^6 \times 490.87 \times 10^{-6}} = -4.8505 \times 10^{-9} P$$

$$* \delta^* = \frac{1}{4} \text{ turn} \times 1.5 \text{ mm} = 0.375 \text{ mm} = 375 \times 10^{-6} \text{ m}$$

$$[\delta_{\text{tube}} = \delta^* + \delta_{\text{rod}}] \quad \underline{\text{or}} \quad [\delta_{\text{tube}} - \delta_{\text{rod}} = \delta^*]$$

$$8.8815 \times 10^{-9} P + 4.8505 \times 10^{-9} P = 375 \times 10^{-6}$$

$$P = \frac{0.375 \times 10^{-3}}{(8.8815 + 4.8505) \times 10^{-9}} = 27.308 \times 10^3 \text{ N}$$

$$a) \sigma_{\text{tube}} = \frac{27.308 \times 10^3}{402.12 \times 10^{-6}} = 67.9 \times 10^6 \text{ Pa}$$

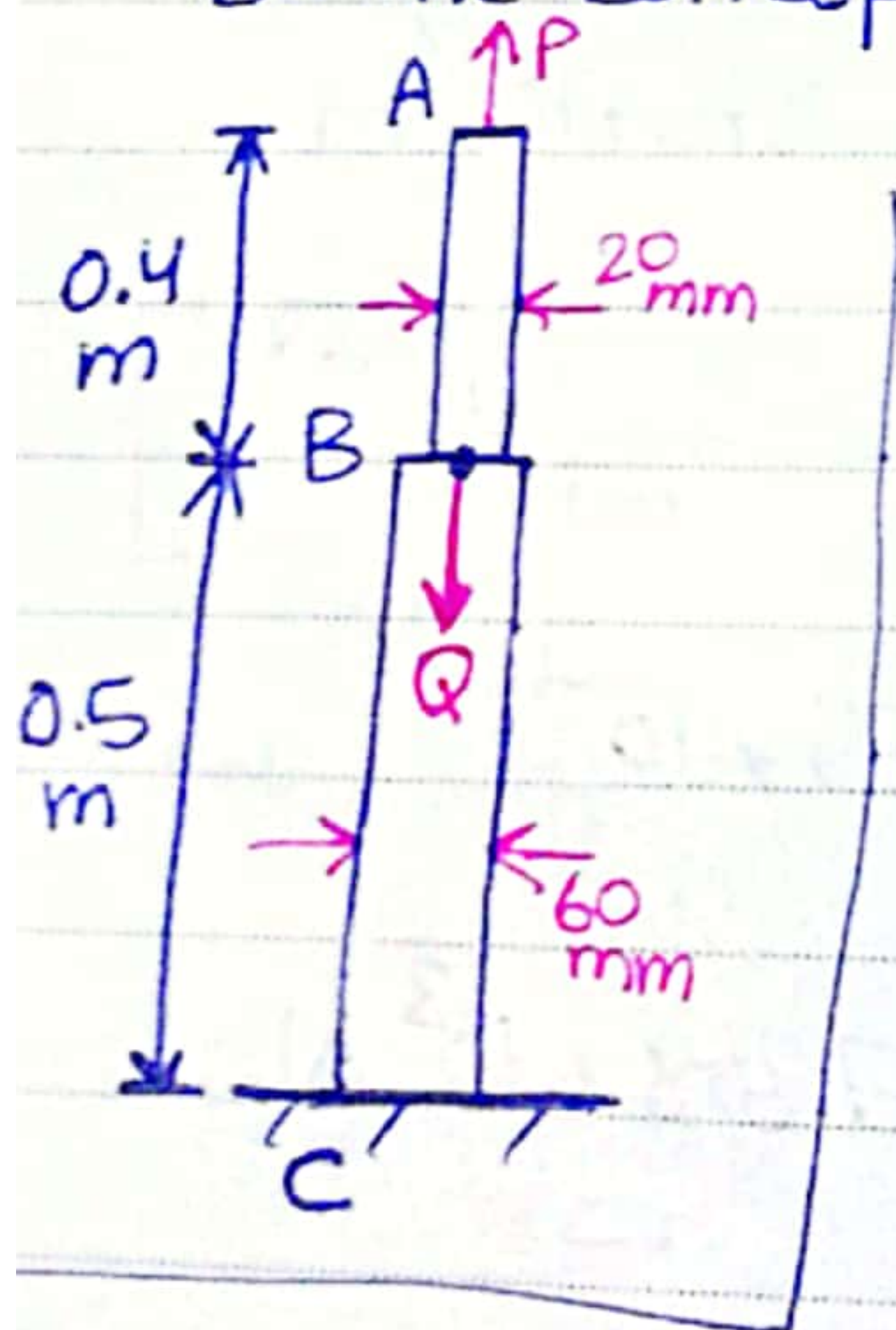
$$\sigma_{\text{rod}} = \frac{-27.308 \times 10^3}{490.87 \times 10^{-6}} = -55.6 \times 10^6 \text{ Pa}$$

$$b) \delta_{\text{tube}} = 8.8815 \times 10^{-9} \times 27.308 \times 10^3 = 242.5 \times 10^{-6} \text{ m}$$

$$\delta_{\text{rod}} = -(4.8505 \times 10^{-9}) (27.308 \times 10^3) \\ = -132.5 \times 10^{-6} \text{ m}$$

2.19) Both portions of the rod ABC are made of an aluminum for which $E = 70 \text{ GPa}$. Knowing that the magnitude of P is 4 kN , determine:

- the value of Q so that the deflection at A is zero
- the corresponding deflection of B



$$a) A_{AB} = \frac{\pi}{4} d_{AB}^2 = \frac{\pi}{4} (0.02)^2 \\ = 314.16 \times 10^{-6} \text{ m}^2$$

$$A_{BC} = \frac{\pi}{4} d_{BC}^2 = \frac{\pi}{4} (0.06)^2 \\ = 2.8274 \times 10^{-3} \text{ m}^2$$

- force in member AB is tension (P)

Elongation \rightarrow

$$\delta_{AB} = \frac{P L_{AB}}{E A_{AB}} = \frac{4 \times 10^3 \times 0.4}{70 \times 10^9 \times 314.16 \times 10^{-6}} = 72.756 \times 10^{-6} \text{ m}$$

- force in member BC is $(Q-P)$ compression

Shortening \rightarrow

$$\delta_{BC} = \frac{(Q-P) L_{BC}}{E A_{BC}} = \frac{(Q-P)(0.5)}{70 \times 10^9 \times 2.8274 \times 10^{-3}}$$

$$= 2.5263 \times 10^{-9} (Q-P)$$

\Rightarrow for zero deflection at A, $\delta_{BC} = \delta_{AB}$

$$2.5263 \times 10^{-9} (Q-P) = 72.756 \times 10^{-6}$$

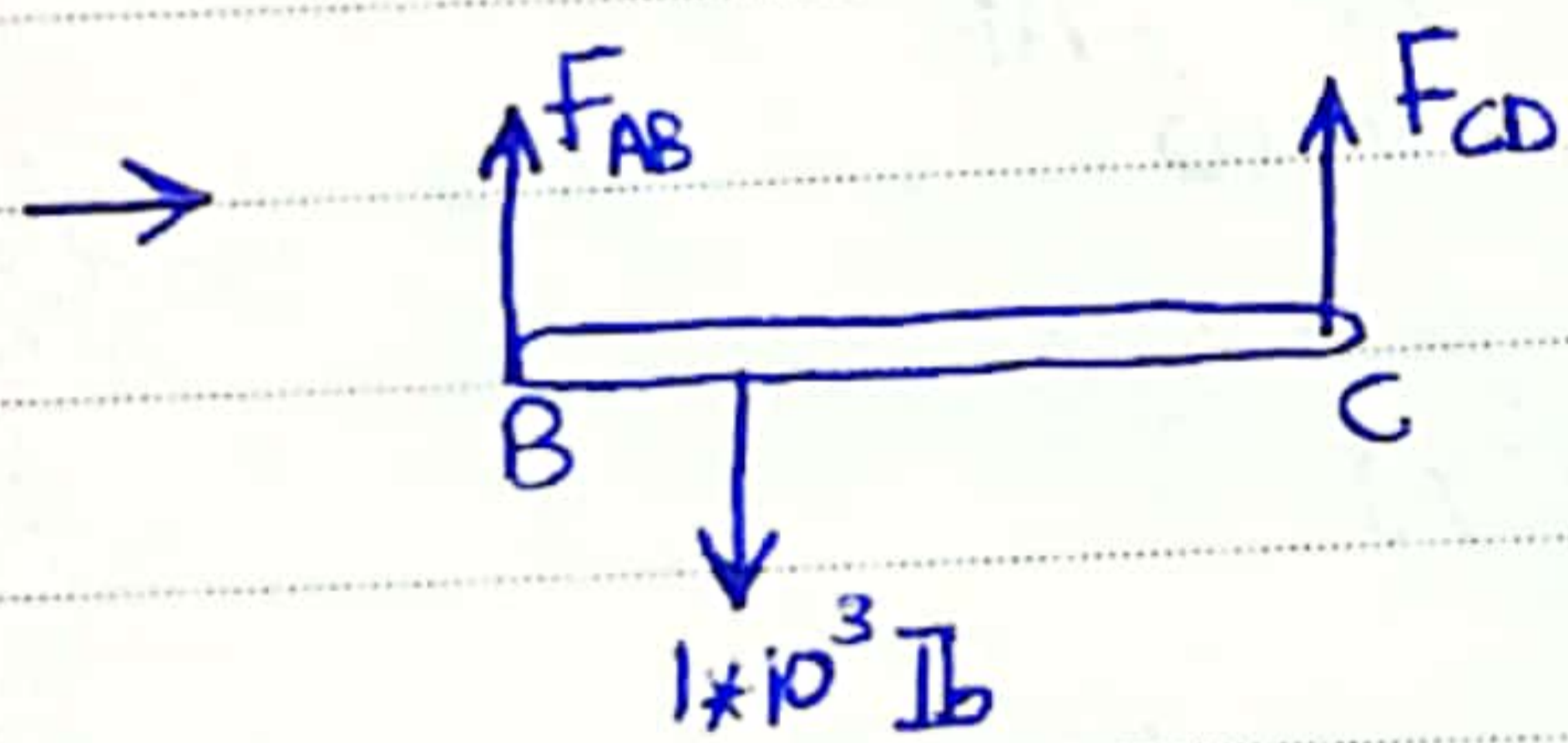
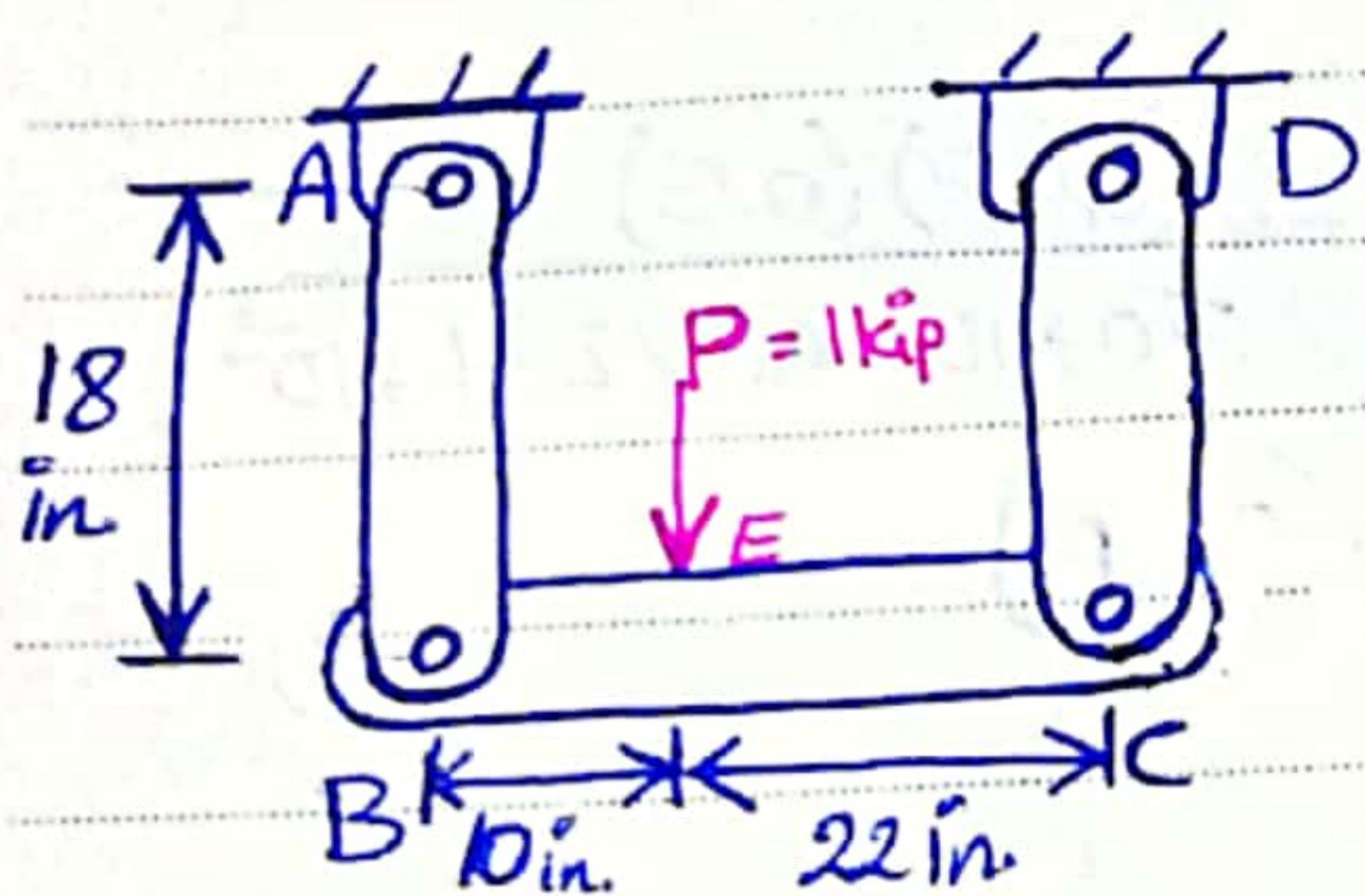
$$\therefore Q-P = 28.8 \times 10^3 \text{ N}$$

$$Q = 28.8 \times 10^3 + 4 \times 10^3 = 32.8 \times 10^3 \text{ N}$$

b) $\delta_{AB} = \delta_{BC} = \delta_B = 72.756 \times 10^{-6} \text{ m}$

$$\delta_{AB} = 0.0728 \text{ mm} \downarrow$$

2.25) Each of the links AB and CD is made of aluminum ($E = 10.9 \times 10^6$ psi) and has a cross-sectional area of 0.2 in^2 . Knowing that they support the rigid member BC, determine the deflection of point E?



$$\sum M_C = 0 :$$

$$-(32) F_{AB} + 22(1 \times 10^3) = 0$$

$$F_{AB} = 687.5 \text{ lb}$$

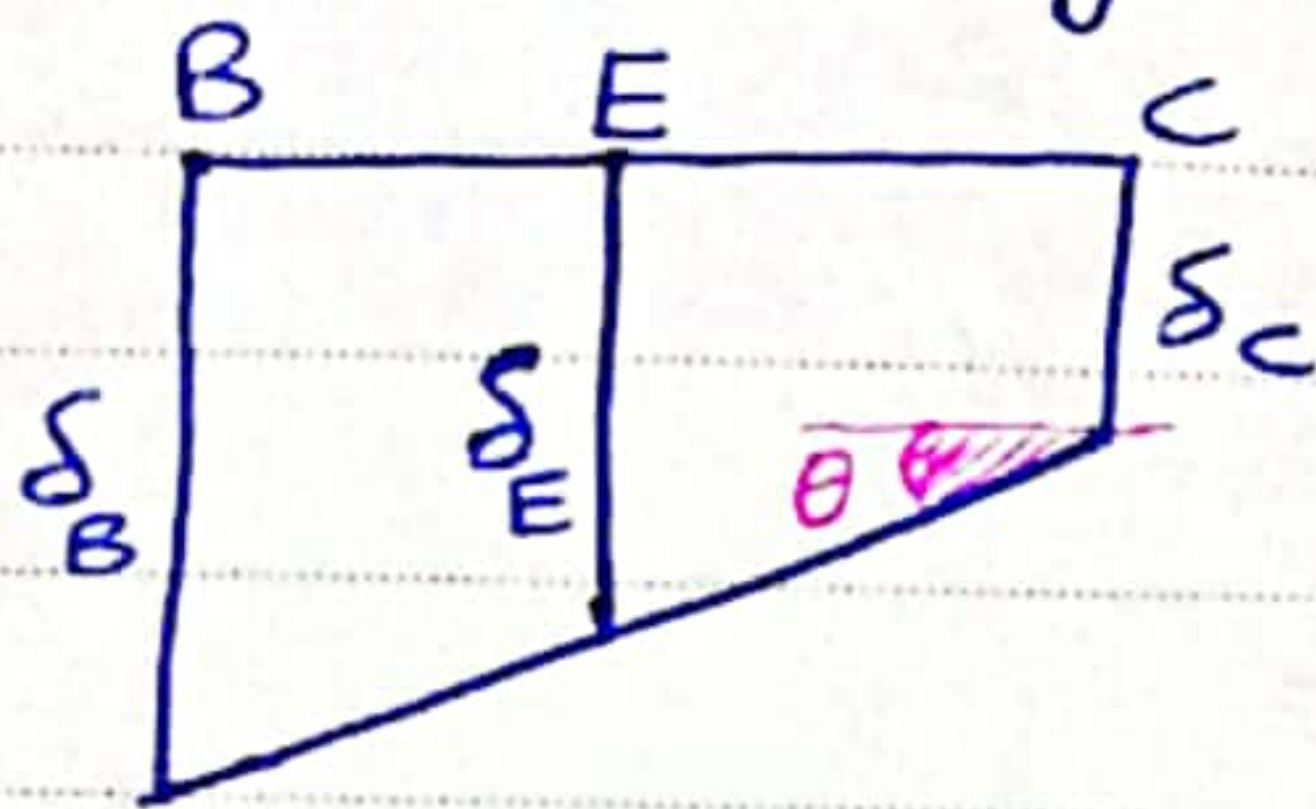
$$\sum F_y = 0 : 687.5 - 1 \times 10^3 + F_{CD} = 0$$

$$F_{CD} = 312.5 \text{ lb}$$

$$\delta_{AB} = \frac{F_{AB} L_{AB}}{EA} = \frac{687.5 \times 18}{10.9 \times 10^6 \times 0.2} = 5.6766 \times 10^{-3} \text{ in.} = \delta_B$$

$$\delta_{CD} = \frac{F_{CD} L_{CD}}{EA} = \frac{312.5 \times 18}{10.9 \times 10^6 \times 0.2} = 2.5803 \times 10^{-3} \text{ in.} = \delta_C$$

Deformation diagram:



$$\text{Slope } \theta = \frac{\delta_B - \delta_C}{L_{BC}} = \frac{3.0963 \times 10^{-3}}{32}$$

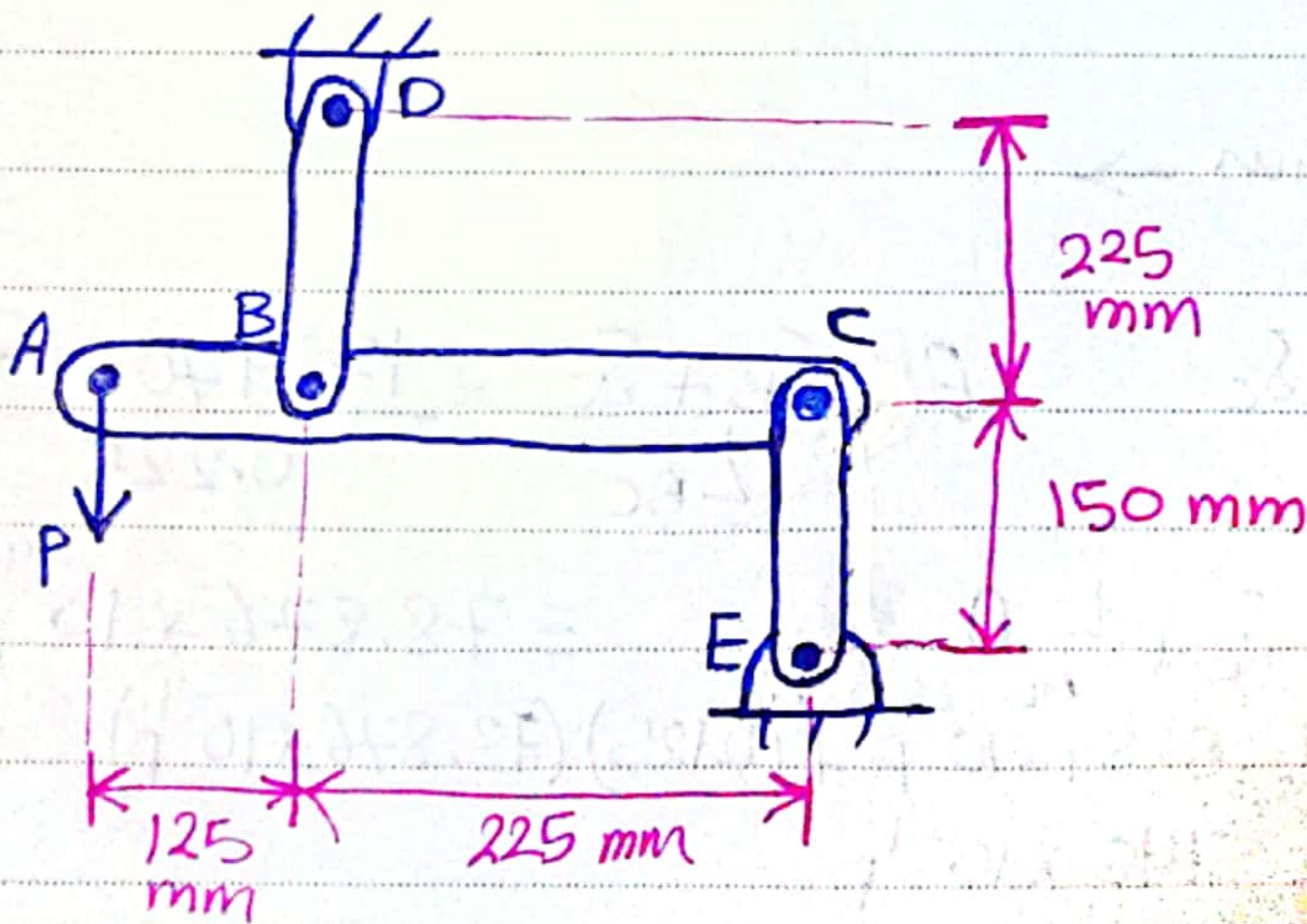
$$= 96.759 \times 10^{-6} \text{ rad}$$

$$\delta_E = \delta_C + L_{EC} \theta$$

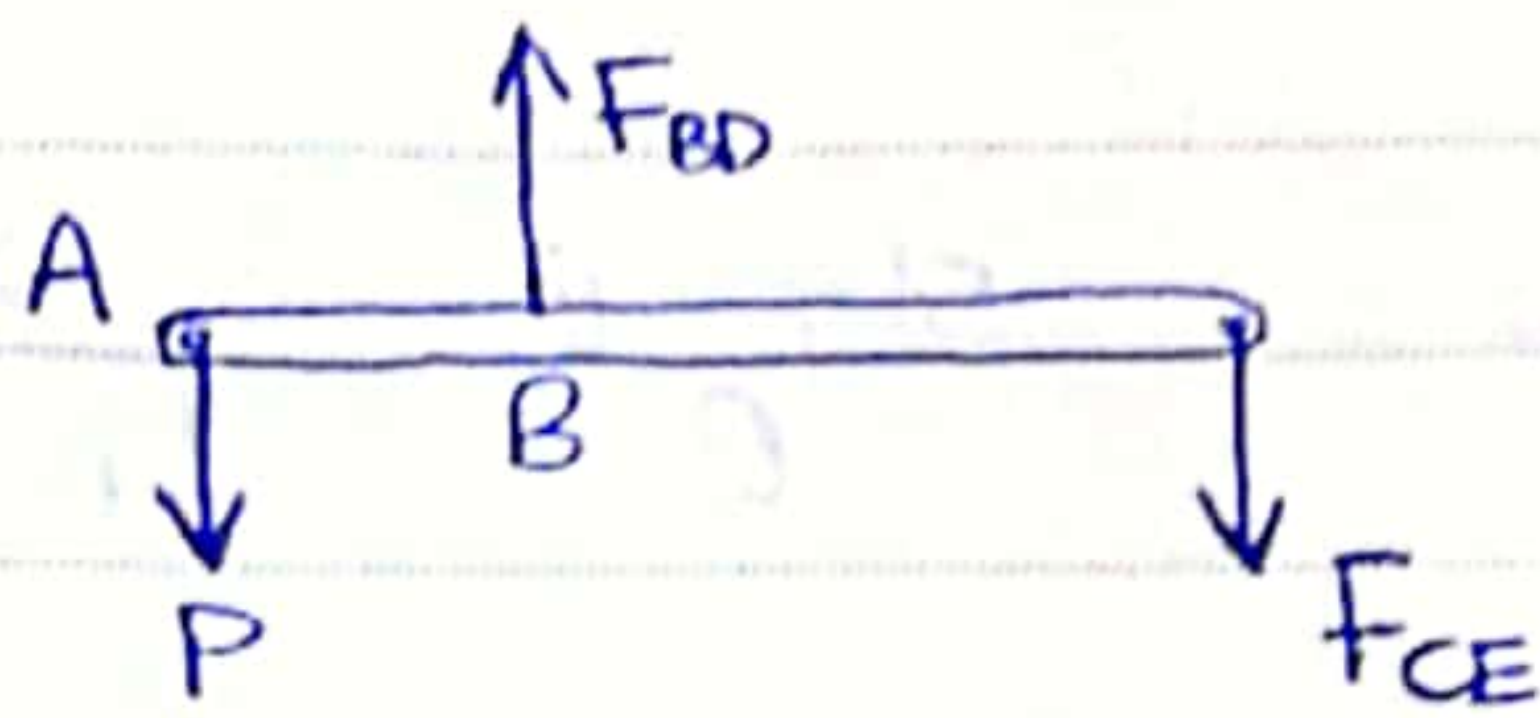
$$= 2.5803 \times 10^{-3} + (22)(96.759 \times 10^{-6})$$

$$= 4.7090 \times 10^{-3} \text{ in. } \downarrow$$

2.27 Link BD is made of brass ($E = 105 \text{ GPa}$) and has a cross sectional area of 240 mm^2 . Link CE is made of aluminum ($E = 72 \text{ GPa}$) and has a cross sectional area of 300 mm^2 . Knowing that they support rigid member ABC, determine the max. force P that can be applied vertically at point A if the deflection of A is not to exceed 0.35 mm ?



FBD for member AC :



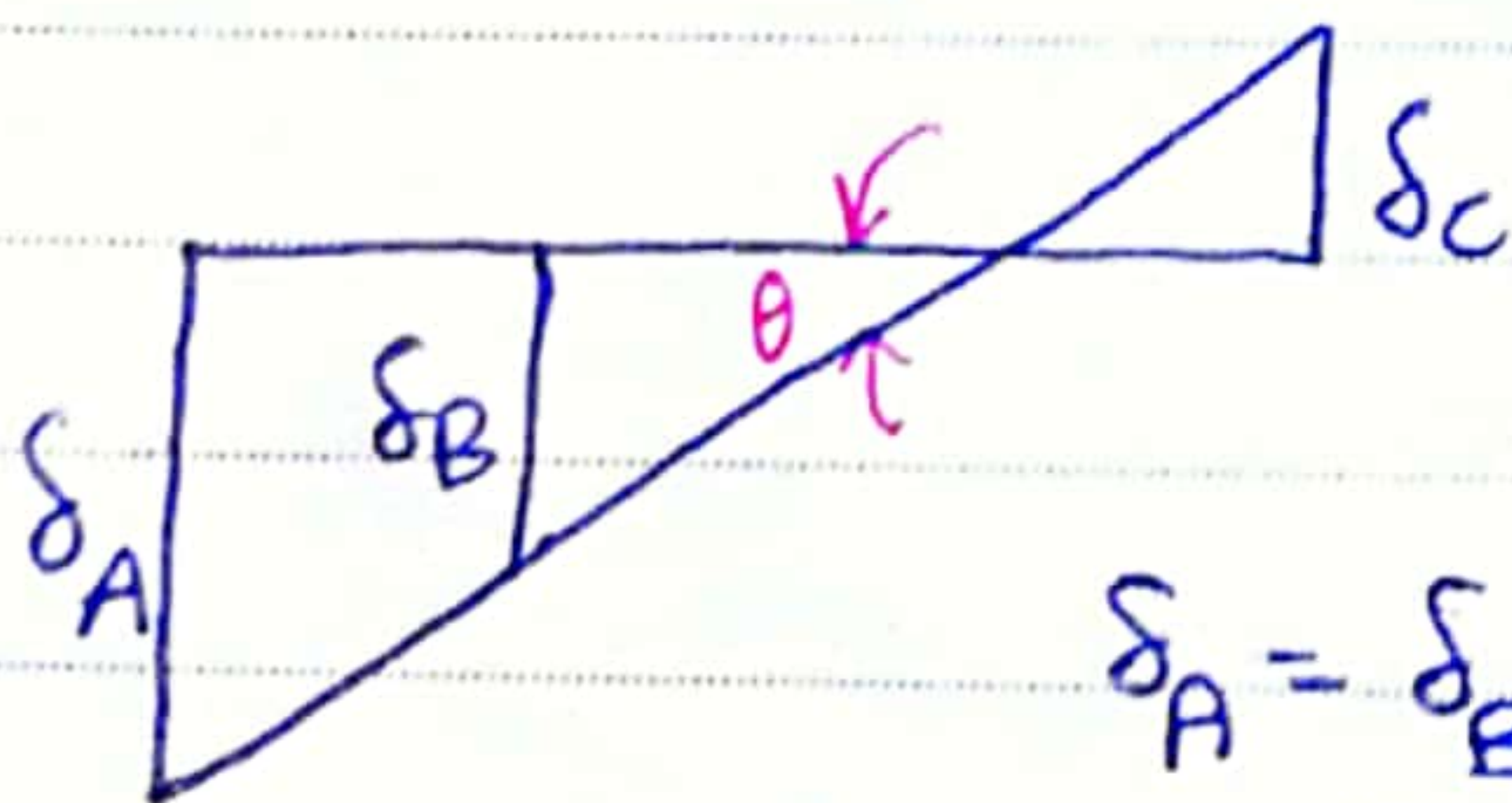
$$\begin{aligned} \sum M_C = 0 &: 0.35P - 0.225F_{BD} = 0 \\ F_{BD} &= 1.5556P \end{aligned}$$

$$\begin{aligned} \sum M_B = 0 &: 0.125P - 0.225F_{CE} = 0 \\ F_{CE} &= 0.55556P \end{aligned}$$

$$\begin{aligned} \delta_B = \delta_{BD} &= \frac{F_{BD} L_{BD}}{E_{BD} A_{BD}} = \frac{1.55556P \times 0.225}{105 \times 10^9 \times 240 \times 10^{-6}} \\ &= 13.88 \times 10^{-9} P \end{aligned}$$

$$\begin{aligned} \delta_C = \delta_{CE} &= \frac{F_{CE} L_{CE}}{E_{CE} A_{CE}} = \frac{0.55556P \times 0.150}{72 \times 10^9 \times 300 \times 10^{-6}} \\ &= 3.8581 \times 10^{-9} P \end{aligned}$$

Deformation Diagram \rightarrow



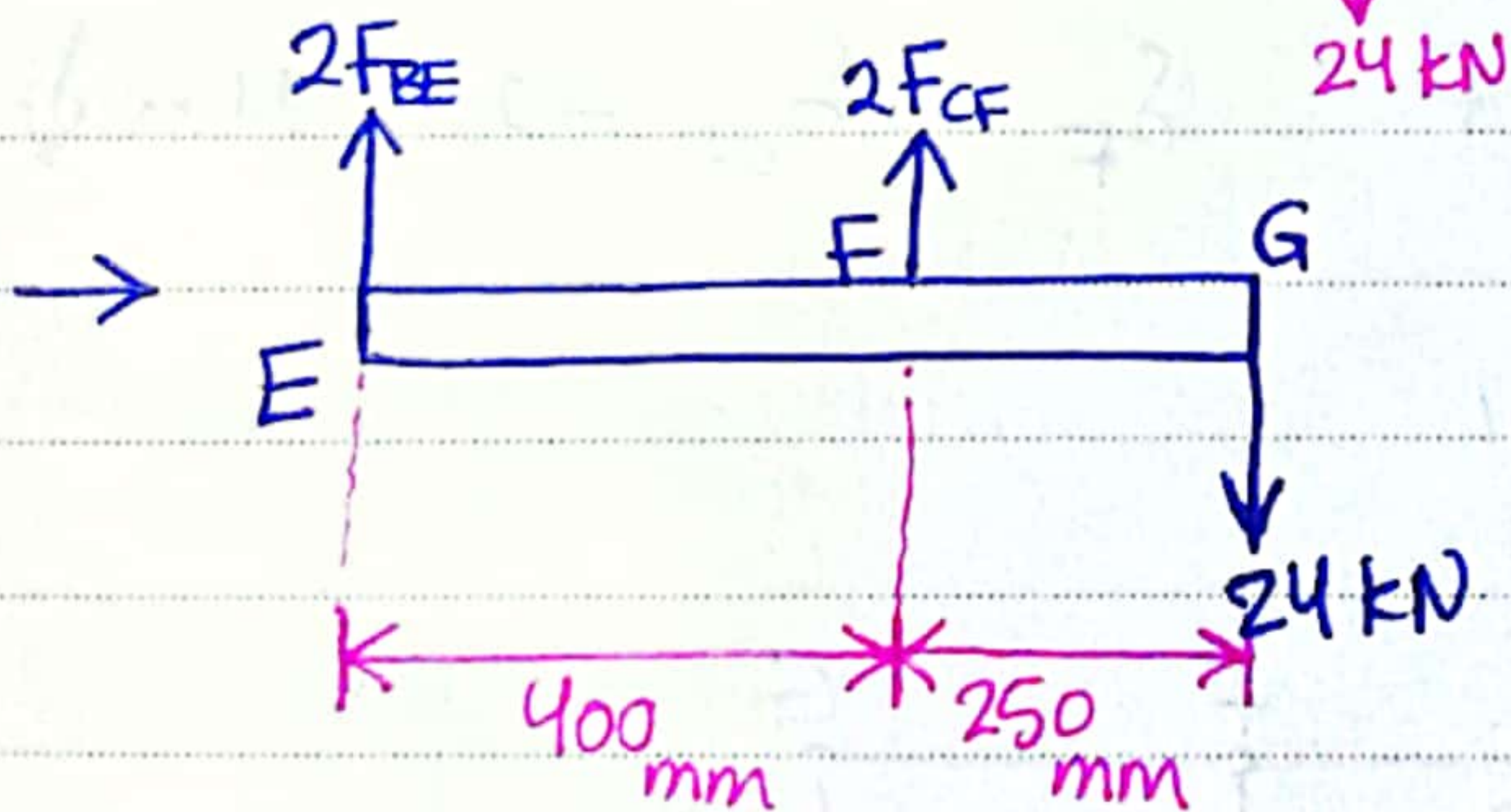
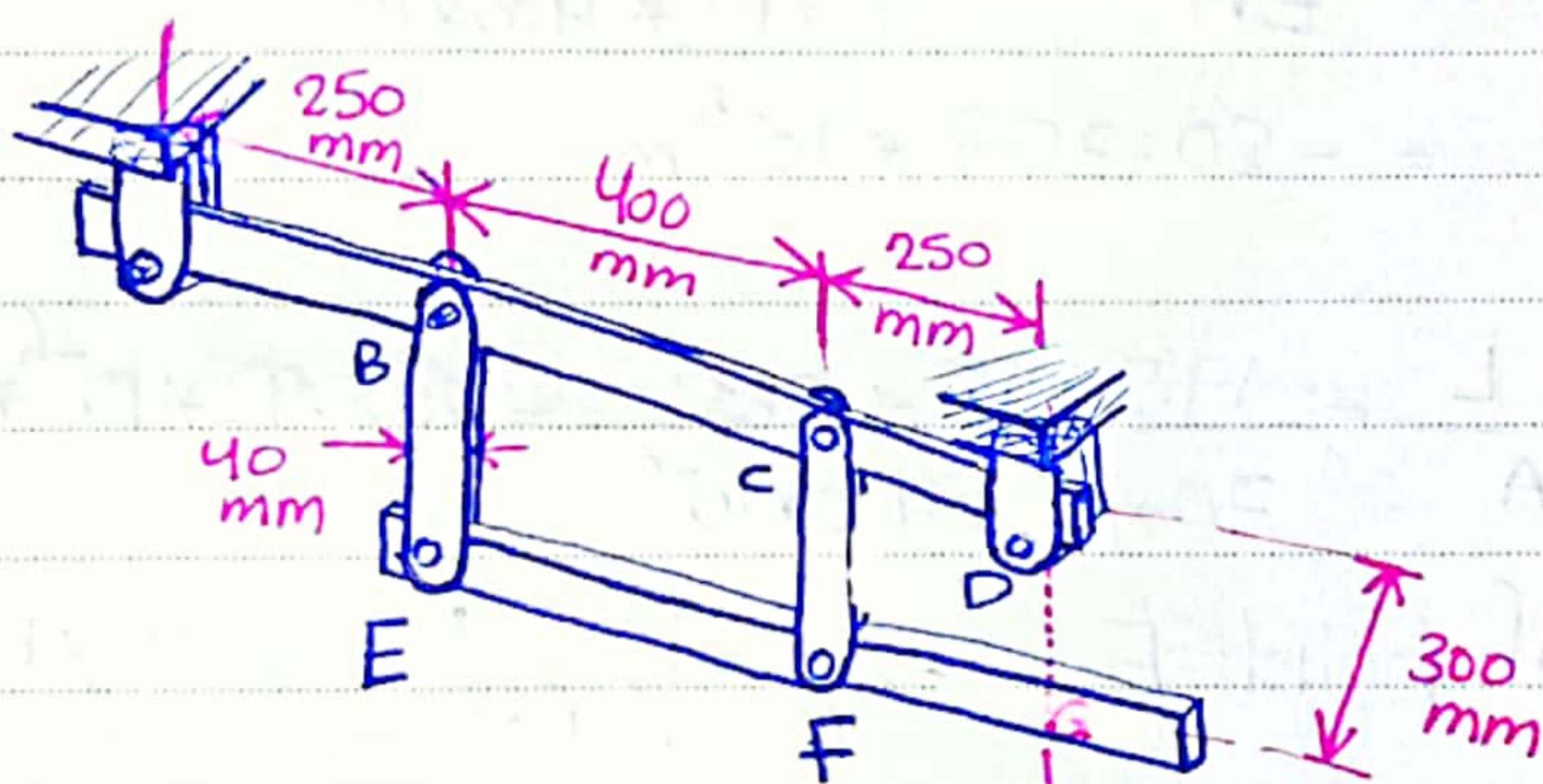
$$\theta = \frac{\delta_B + \delta_C}{L_{BC}} = \frac{17.7470 \times 10^{-9} P}{0.225}$$

$$\begin{aligned} \delta_A &= \delta_B + L_{AB} \theta = 78.876 \times 10^{-9} P \\ &= 13.8889 \times 10^{-9} P + (0.125)(78.876 \times 10^{-9} P) \\ &= 23.748 \times 10^{-9} P \end{aligned}$$

ALADIB apply displacement limit : $\delta_A = 0.35 \times 10^{-3} \text{ m} = 23.748 \times 10^{-9} P$

$$P = 14.7381 \times 10^3 \text{ N}$$

- 228) Each of the four vertical links connecting the two rigid horizontal members is made of aluminum $E = 70 \text{ GPa}$ and has a uniform rectangular cross section of $10 \times 40 \text{ mm}$. For the loading shown, determine the deflection of:
- point A
 - point F
 - point G



$$\begin{aligned} \sum M_F = 0 &: -(400)(2F_{BE}) - (250)(24) = 0 \\ F_{BE} &= -7.5 \text{ kN} \end{aligned}$$

$$\begin{aligned} \sum M_E = 0 &: 400(2F_{CF}) - (650)(24) = 0 \\ F_{CF} &= 19.5 \text{ kN} \end{aligned}$$

Area of one link :

$$A = 10 \times 40 = 400 \text{ mm}^2 \\ = 400 \times 10^{-6} \text{ m}^2$$

Length : $L = 300 \text{ mm}$

Deformations

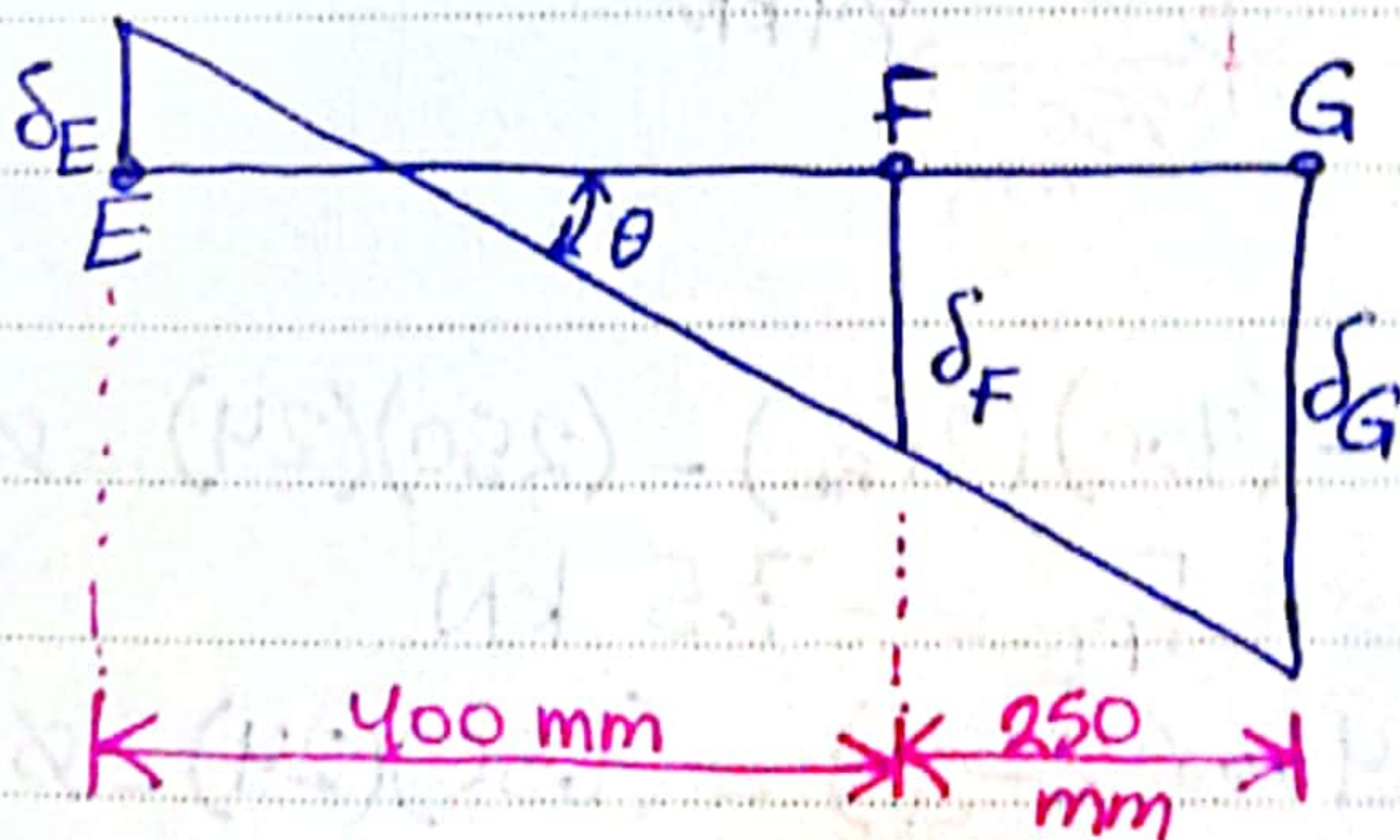
$$\delta_{BE} = \frac{F_{BE} L}{EA} = \frac{-7.5 \times 10^3 \times 0.3}{70 \times 10^9 \times 400 \times 10^{-6}} \\ = -80.357 \times 10^{-6} \text{ m}$$

$$\delta_{CF} = \frac{F_{CF} L}{EA} = \frac{19.5 \times 10^3 \times 0.3}{70 \times 10^9 \times 400 \times 10^{-6}} = 208.93 \times 10^{-6} \text{ m}$$

a) Deflection of point E $\delta_E = |\delta_{BE}| = 80.4 \mu\text{m} \uparrow$

b) Deflection of point F $\delta_F = \delta_{CF} = 209 \mu\text{m} \downarrow$

Geometry Change \Rightarrow



Let θ be the small change in slope angle

$$\theta = \frac{\delta_E + \delta_F}{L_{EF}} = \frac{80.357 \times 10^{-6} + 208.93 \times 10^{-6}}{0.4} = 723.22 \times 10^{-6} \text{ rad}$$

c) Deflection of point G:

$$\delta_G = \delta_F + L_{FG} \theta$$

$$= 208.93 \times 10^{-6} + (0.25)(723.22 \times 10^{-6})$$

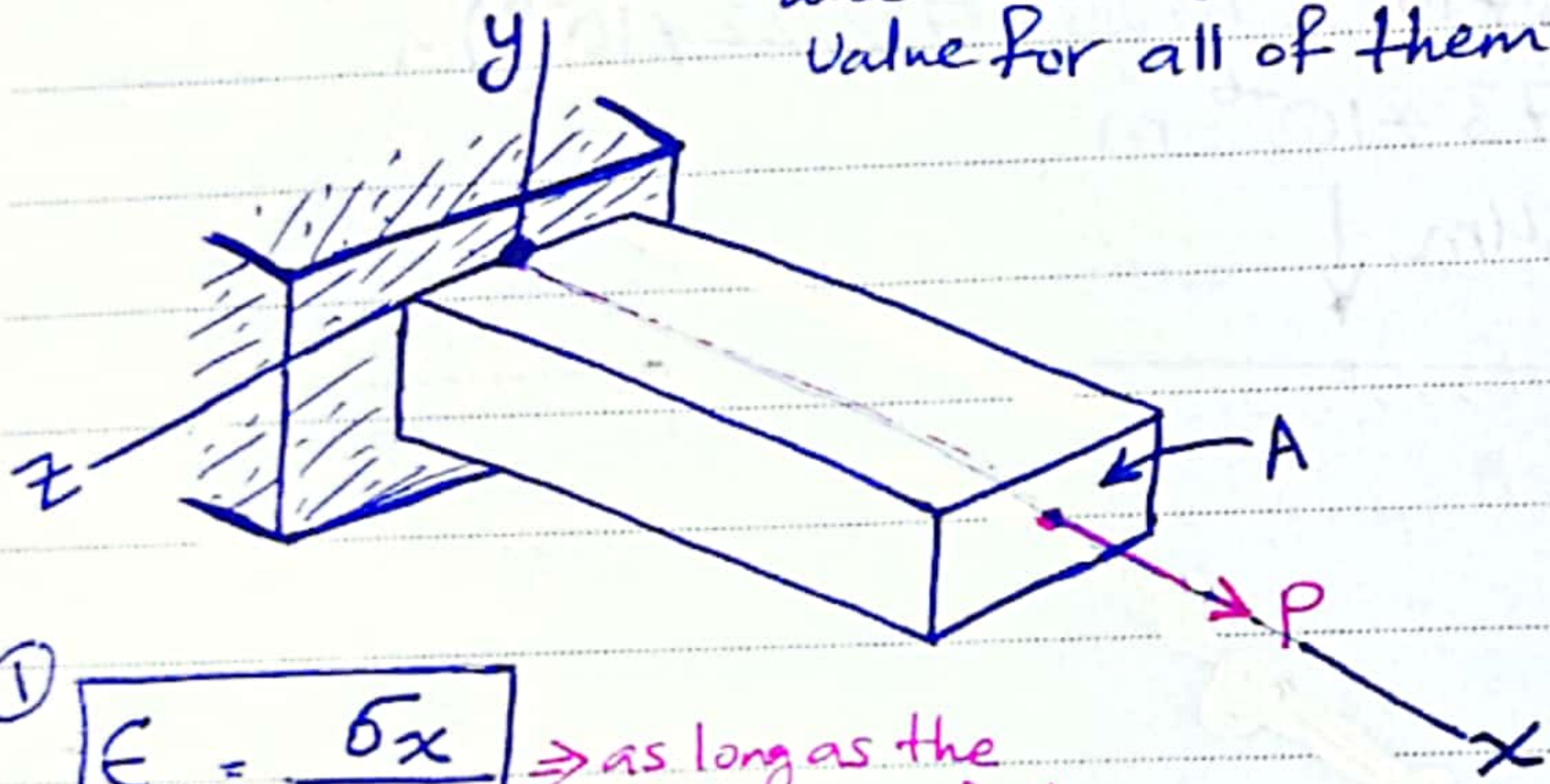
$$= 389.73 \times 10^{-6} \text{ m}$$

$$\approx 390 \mu\text{m} \downarrow$$

2.11 Poisson's Ratio

When an axial load P is applied to a homogeneous, slender bar, it causes a strain.

Strain \rightarrow along the axis of the bar $[\epsilon_x]$ \leftarrow axial strain
 \rightarrow along any transverse direction $[\epsilon_y], [\epsilon_z]$ \leftarrow lateral strain
 which must be the same value for all of them $\rightarrow \epsilon_y = \epsilon_z$



$$[\sigma_y = \sigma_z = \emptyset \text{ due to zero force}]$$

① $\epsilon_x = \frac{\sigma_x}{E} \Rightarrow$ as long as the elastic limit of the material is not exceeded.

$$\epsilon_y \neq \epsilon_z \neq \epsilon_x \neq \emptyset$$

$$[\nu = - \frac{\text{lateral strain}}{\text{axial strain}}] \rightarrow \text{poisson's ratio}$$

② $\nu = \frac{-\epsilon_y}{\epsilon_x} = \frac{-\epsilon_z}{\epsilon_x}$
 used to get a (+ve) value for ν

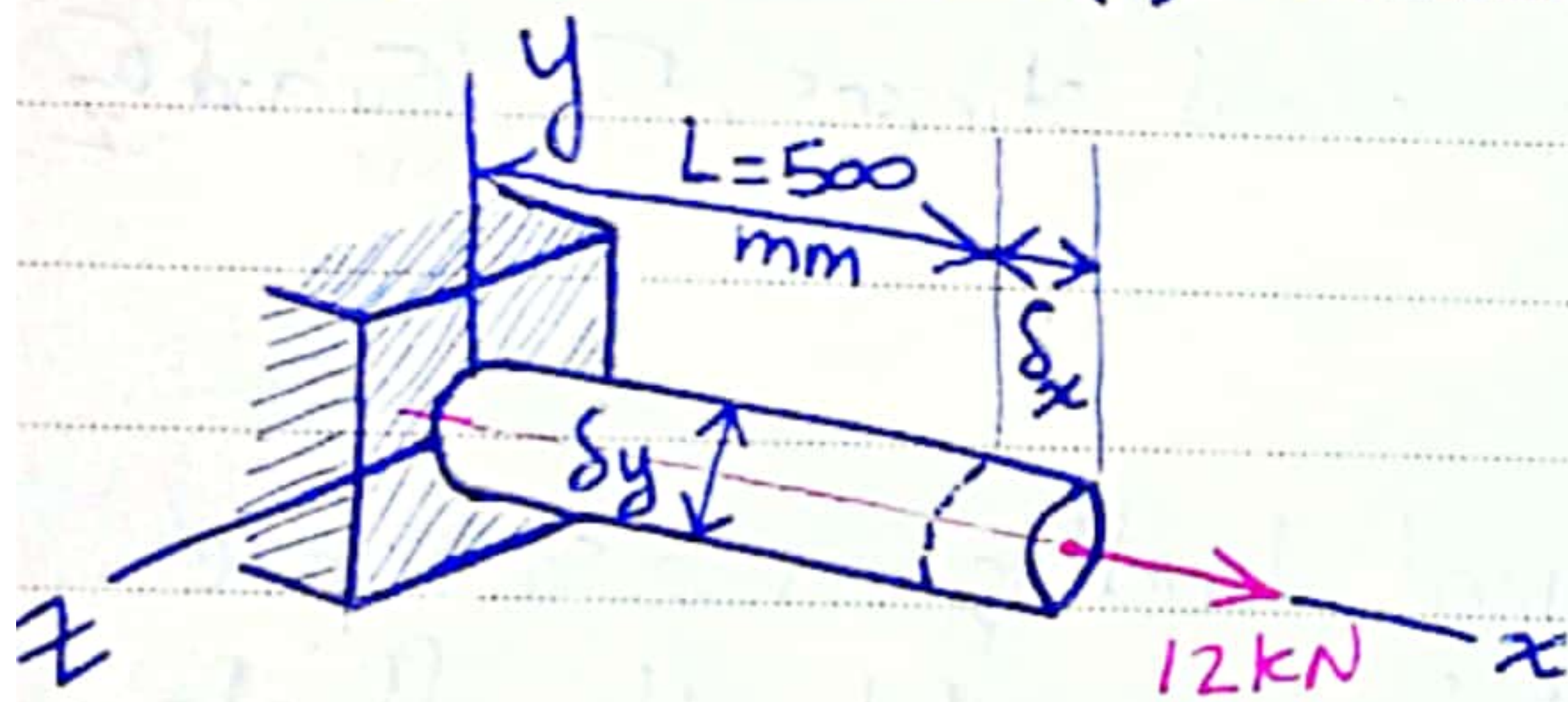
From ① & ② we get \rightarrow

$$\left[\epsilon_y = \epsilon_z = -\nu \epsilon_x = -\frac{\nu \sigma_x}{E} \right]$$

describes the condition of strain under an axial load applied in a direction parallel to the x axis.

Ex(2.07)

A 500 mm long, 16 mm diameter rod made of a homogeneous, isotropic material is observed to increase in length by 300 μm , and to decrease in diameter by 2.4 μm when subjected to an axial **12 kN** load. Determine the modulus of elasticity (E) & Poisson's ratio (ν) of the material?



$$d_{\text{rod}} = 16 \text{ mm}$$

$$\delta_y = -2.4 \mu\text{m}$$

$$\delta_x = 300 \mu\text{m}$$

$$\rightarrow A_{\text{rod}} = \pi r^2 = \pi (8 \times 10^{-3} \text{ m})^2 = 201 \times 10^{-6} \text{ m}^2$$

$$\sigma_x = \frac{P}{A} = \frac{12 \times 10^3 \text{ N}}{201 \times 10^{-6} \text{ m}^2} = 59.7 \text{ MPa}$$

$$\epsilon_x = \frac{\delta_x}{L} = \frac{300 \times 10^{-6}}{500 \times 10^{-3}} = 600 \times 10^{-6}$$

$$\epsilon_y = \frac{\delta_y}{d} = \frac{-2.4 \times 10^{-6}}{16 \times 10^{-3}} = -150 \times 10^{-6}$$

From Hooke's law, $\sigma_x = E \epsilon_x$ we obtain:

$$E = \frac{\sigma_x}{\epsilon_x} = \frac{59.7 \times 10^6 \text{ Pa}}{600 \times 10^{-6}} = 99.5 \text{ GPa}$$

$$\nu = \frac{-\epsilon_y}{\epsilon_x} = \frac{-150 \times 10^{-6}}{600 \times 10^{-6}} = \underline{0.25}$$

2.12 Multiaxial loading ; Generalized Hooke's Law

In this section, we'll consider structural elements subjected to loads acting in the directions of the three coordinate axes and producing normal stresses, σ_x , σ_y and σ_z that they aren't zeros ..

The effect of a given combined loading on a structure can be obtained by determining separately the effects of the various loads and combining the results obtained, provided that the following conditions are satisfied:

- ① Each effect is linearly related to the load that produces it
- ② The deformation resulting from any given load is small & doesn't affect the conditions of application of the other loads.

In the multiaxial loading, the 1st condition will be satisfied if the stresses don't exceed the proportional limit of the material. the 2nd condition will be satisfied if the stress on any given face doesn't cause deformations of the other faces that are large enough to affect the computation of the stresses on those faces...

$$\epsilon_x = \frac{\sigma_x}{E} - \frac{\nu \sigma_y}{E} - \frac{\nu \sigma_z}{E}$$

$$\epsilon_y = -\frac{\nu \sigma_x}{E} + \frac{\sigma_y}{E} - \frac{\nu \sigma_z}{E}$$

$$\epsilon_z = -\frac{\nu \sigma_x}{E} - \frac{\nu \sigma_y}{E} + \frac{\sigma_z}{E}$$

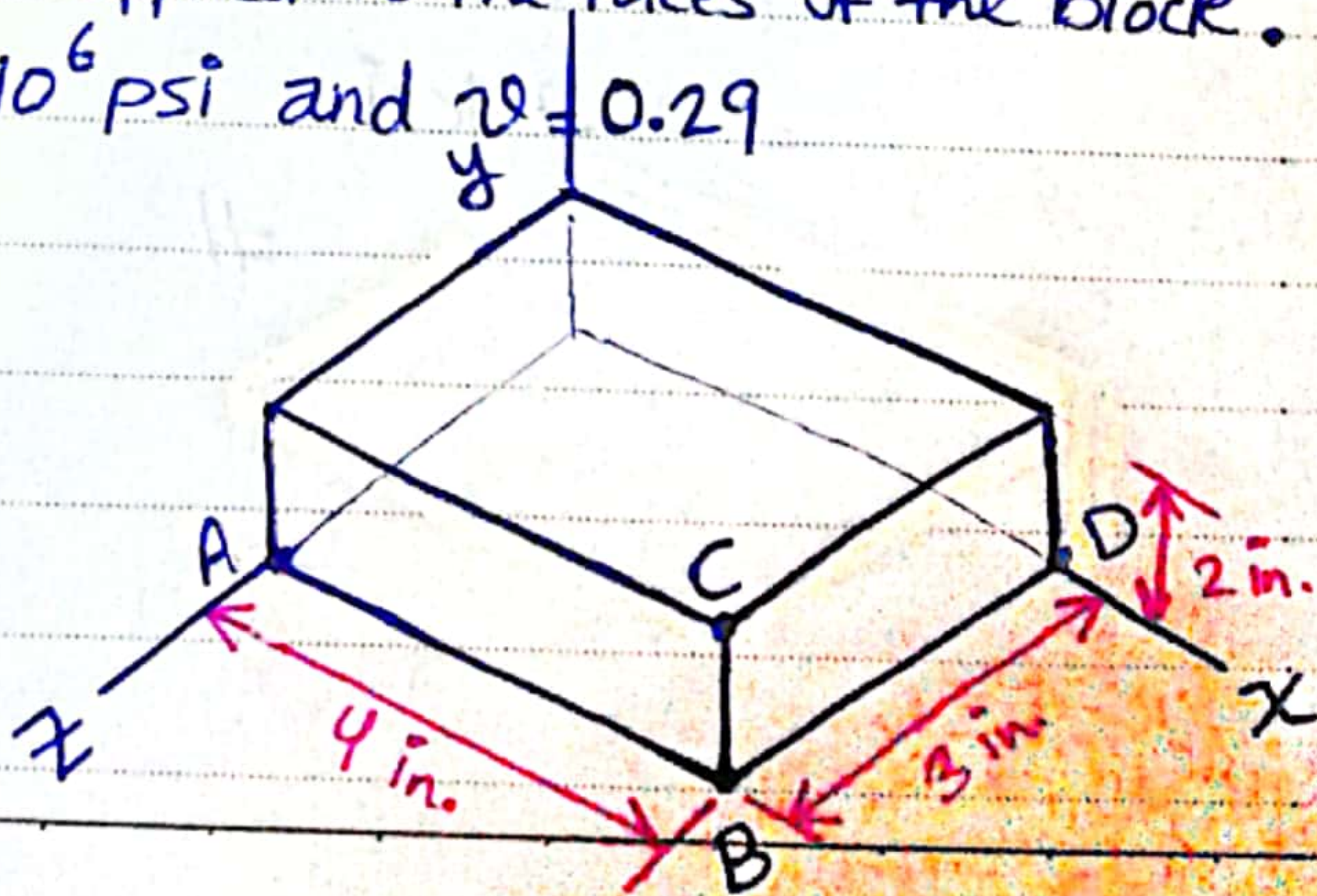
→ Referred to as the generalized Hooke's Law for (the multiaxial loading of a homogenous isotropic material).

Ex(2.08)

The steel block shown below is subjected to a uniform pressure on all its faces. Knowing that the change in length of edge AB is -1.2×10^{-3} inch, determine:

- the change in length of the other two edges
- the pressure P applied to the faces of the block.

Assume $E = 29 \times 10^6$ psi and $\nu = 0.29$



a) Change in length of other edges %

$$\epsilon_x = \epsilon_y = \epsilon_z = \frac{-P}{E} (1 - 2\nu)$$

$$\text{Since } \Rightarrow \epsilon_x = \frac{\delta_x}{AB} = \frac{-1.2 \times 10^{-3} \text{ in.}}{4 \text{ in.}}$$

$$= -300 \times 10^{-6}$$

$$\text{we get // } \epsilon_y = \epsilon_z = \epsilon_x = -300 \times 10^{-6}$$

from which it follows that

$$\delta_y = \epsilon_y (BC) = (-300 \times 10^{-6}) (2) = -600 \times 10^{-6} \text{ in.}$$

$$\delta_z = \epsilon_z (BD) = (-300 \times 10^{-6}) (3) = -900 \times 10^{-6} \text{ in.}$$

b) Pressure %

$$P = \frac{E \epsilon_x}{1 - 2\nu}$$

$$= \frac{-(29 \times 10^6 \text{ psi})(-300 \times 10^{-6})}{1 - 0.58}$$

$$= 20.7 \text{ ksi}$$

$$= 20.7 \text{ ksi}$$

#



Ch.3 Torsion

3.1 Introduction

Torsion is twisting of an object due to an applied torque.

3.2 Discussion of the stresses in a shaft

p : distance

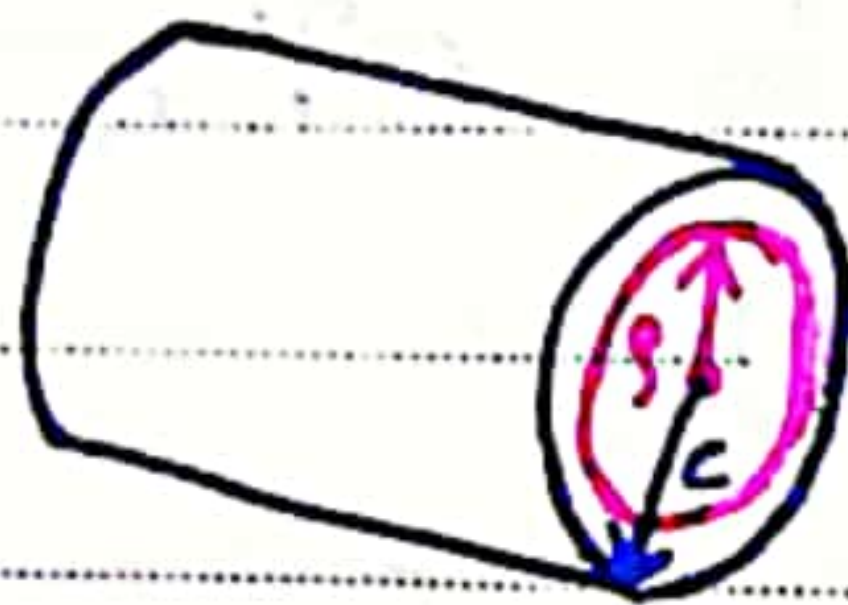
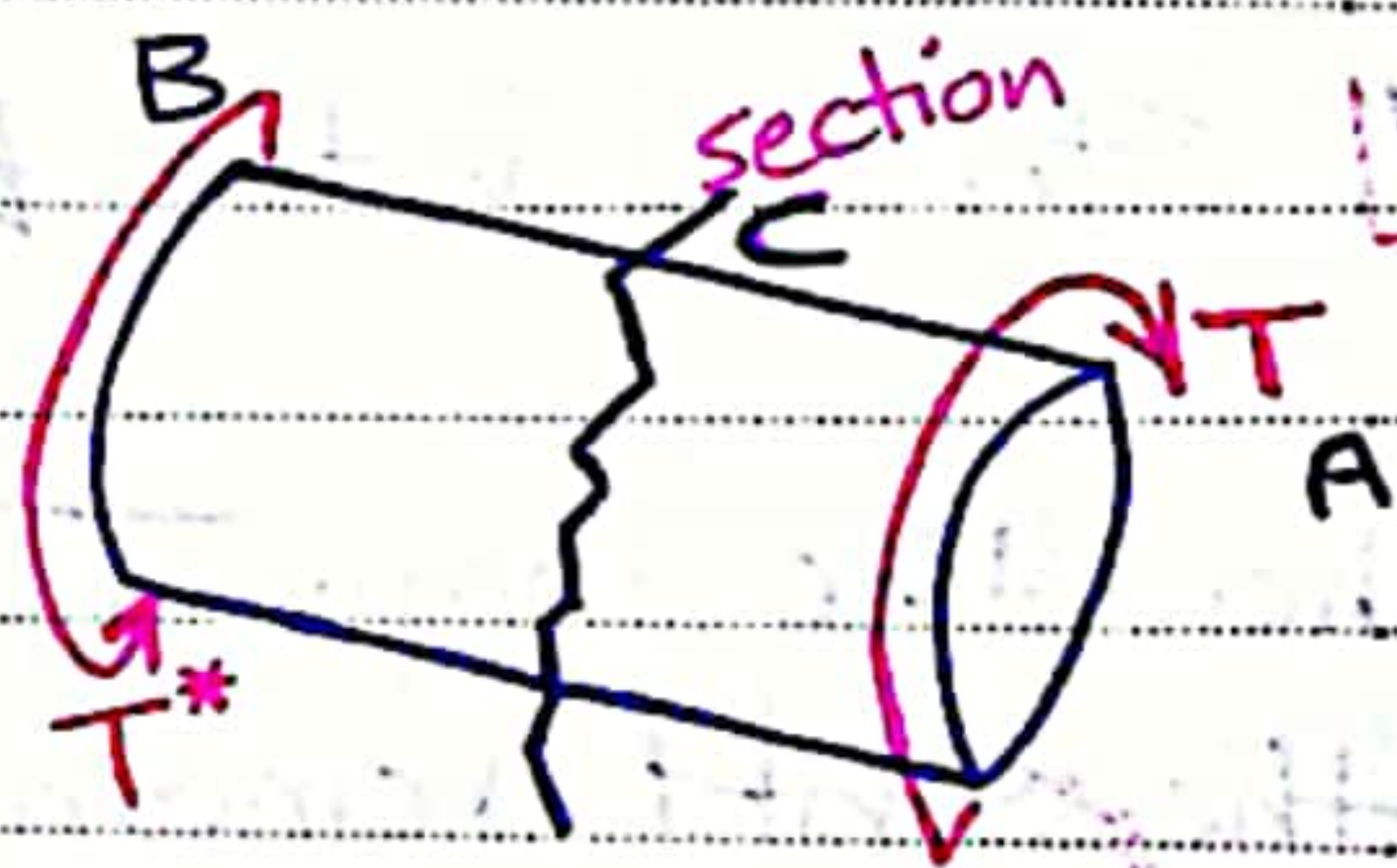
T : torque

dA : area

τ : the shearing stress on the element

$$T = \int p \cdot dF \quad \text{--- (1)}$$

$$dF = \tau dA$$

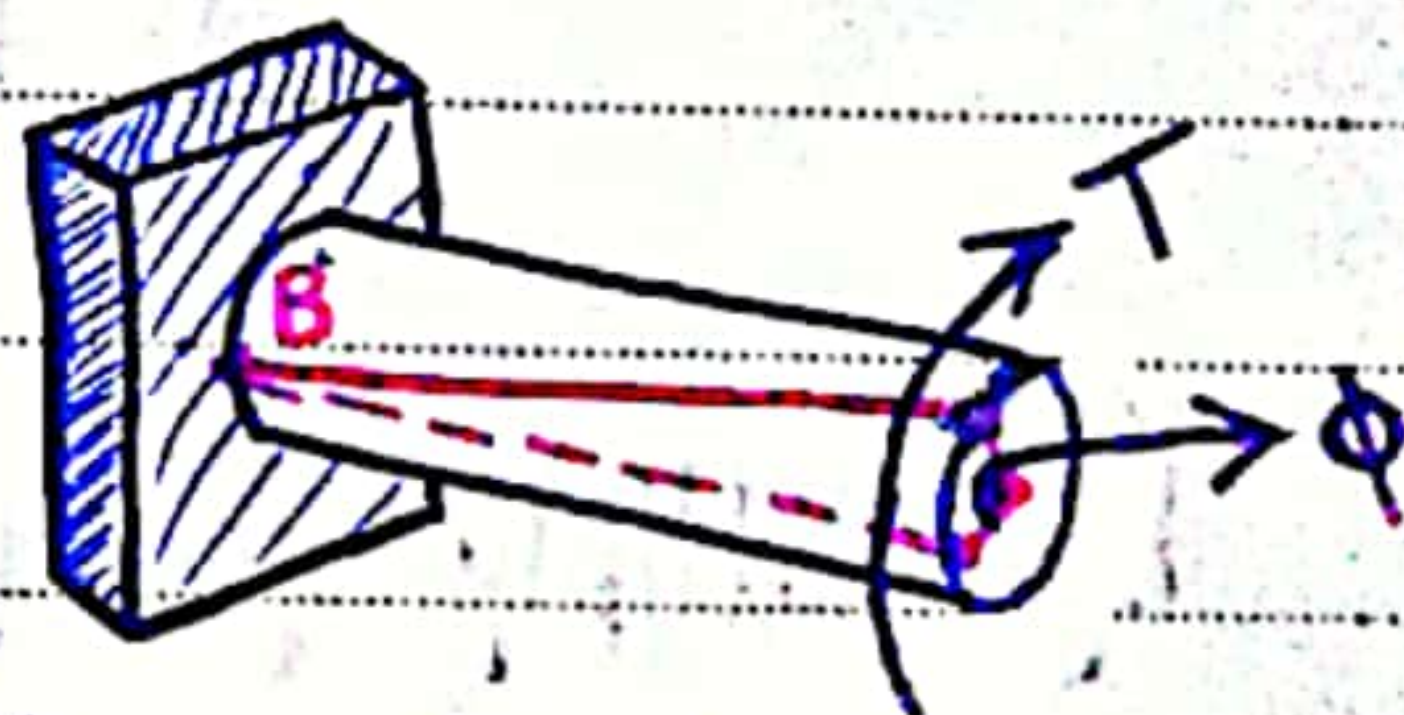


c : shaft radius

3.3 Deformations in a circular shaft

$$\gamma \text{ (shear strain) } = \frac{p \phi}{L} \quad \text{--- (2)}$$

[rad]



the shearing strain in a circular shaft varies linearly with the distance from the axis of the shaft.

where : ϕ , twisting angle
 L , shaft length

- the shearing strain (γ) is max. on the surface of the shaft at $\rho = c$

$$\gamma_{max} = \frac{c \phi}{L} \quad \text{--- (3)}$$

$$\therefore \boxed{\gamma = \frac{\rho}{c} \gamma_{max}} \quad \text{--- (4)}$$

3.4 Stresses in the Elastic Range

When the torque T is such that all shearing stresses in the shaft remain below the yield strength (τ_y). Hooke's law will apply and there will be no permanent deformation.



$$\boxed{\tau = G \gamma}$$

Where G : the modulus of rigidity or shear modulus of the material

- by multiplying equ. (4) by G , we get:

$$G \gamma = \frac{\rho}{c} G \gamma_{max}$$

$$\boxed{\tau = \frac{\rho}{c} \tau_{max}}$$

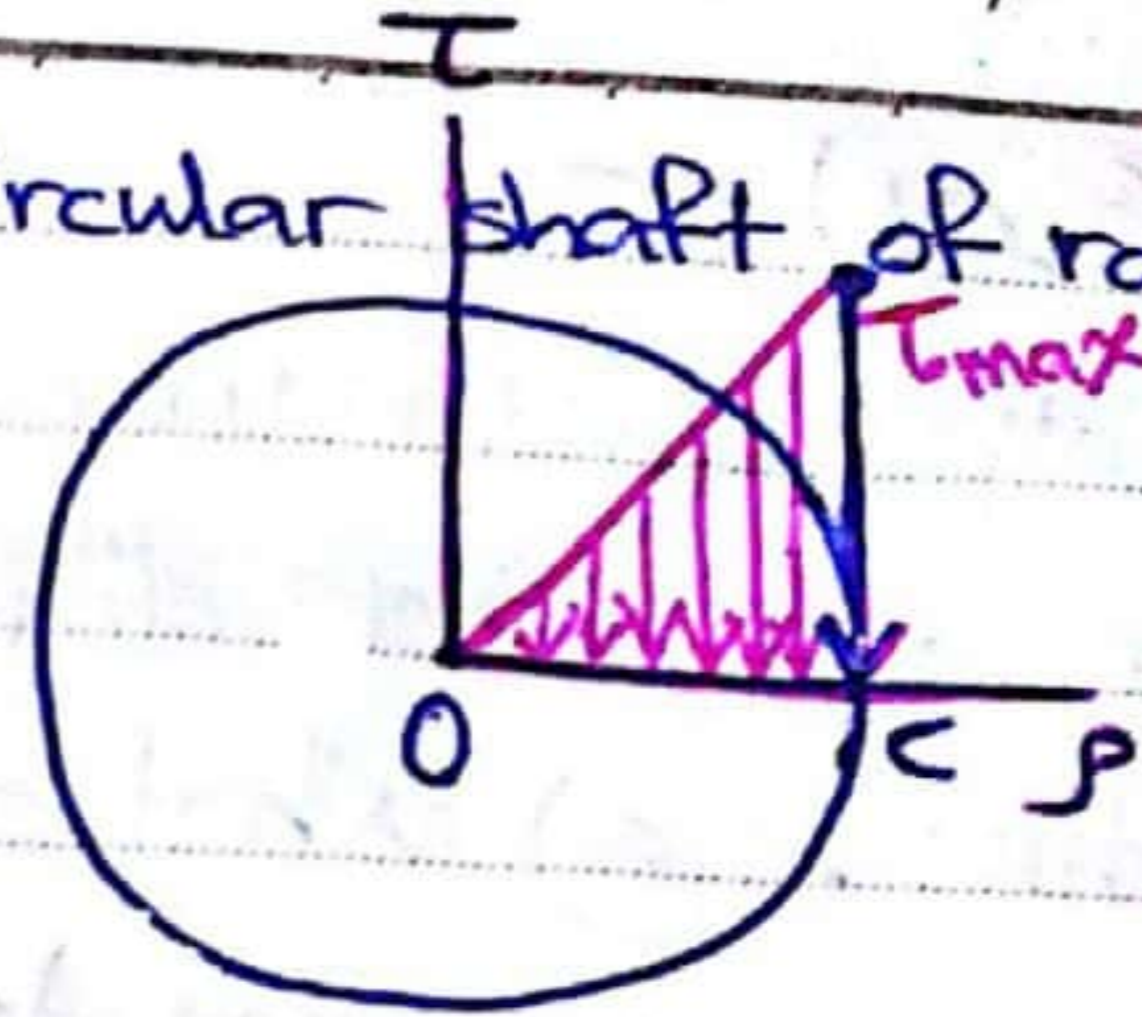
* ما هي العلاقة بين τ و γ (shearing stress τ) على الـ shaft بين ρ

بشكل خطي مع ρ من الـ axis of the shaft...

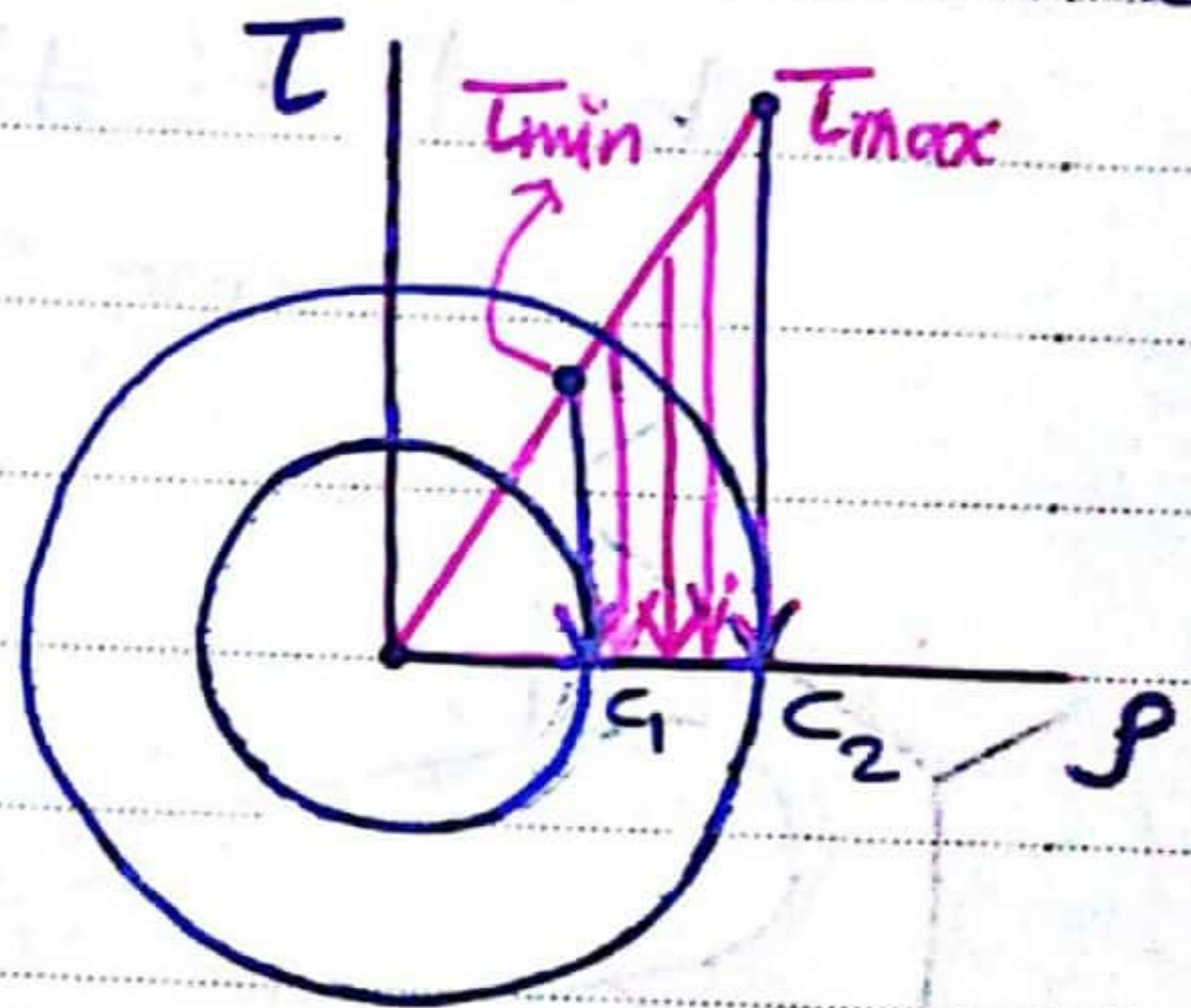
within the elastic region \Leftarrow

from the axis of the shaft...

* the stress distribution in a solid circular shaft of radius C.



* the stress distribution in a hollow circular shaft of inner radius C₁ and outer radius C₂.



$$\tau_{min} = \frac{C_1}{C_2} \tau_{max}$$

Remark) • τ_{max} occur in center
 τ_{min} " " = $R = C$

• polar moment of inertia (J) of a circle of radius (C) is \therefore $J = \frac{\pi}{2} C^4$

For a hollow circular shaft C_1 \leftarrow inner radius & C_2 \leftarrow outer radius \therefore

$$J = \frac{\pi}{2} (C_2^4 - C_1^4)$$

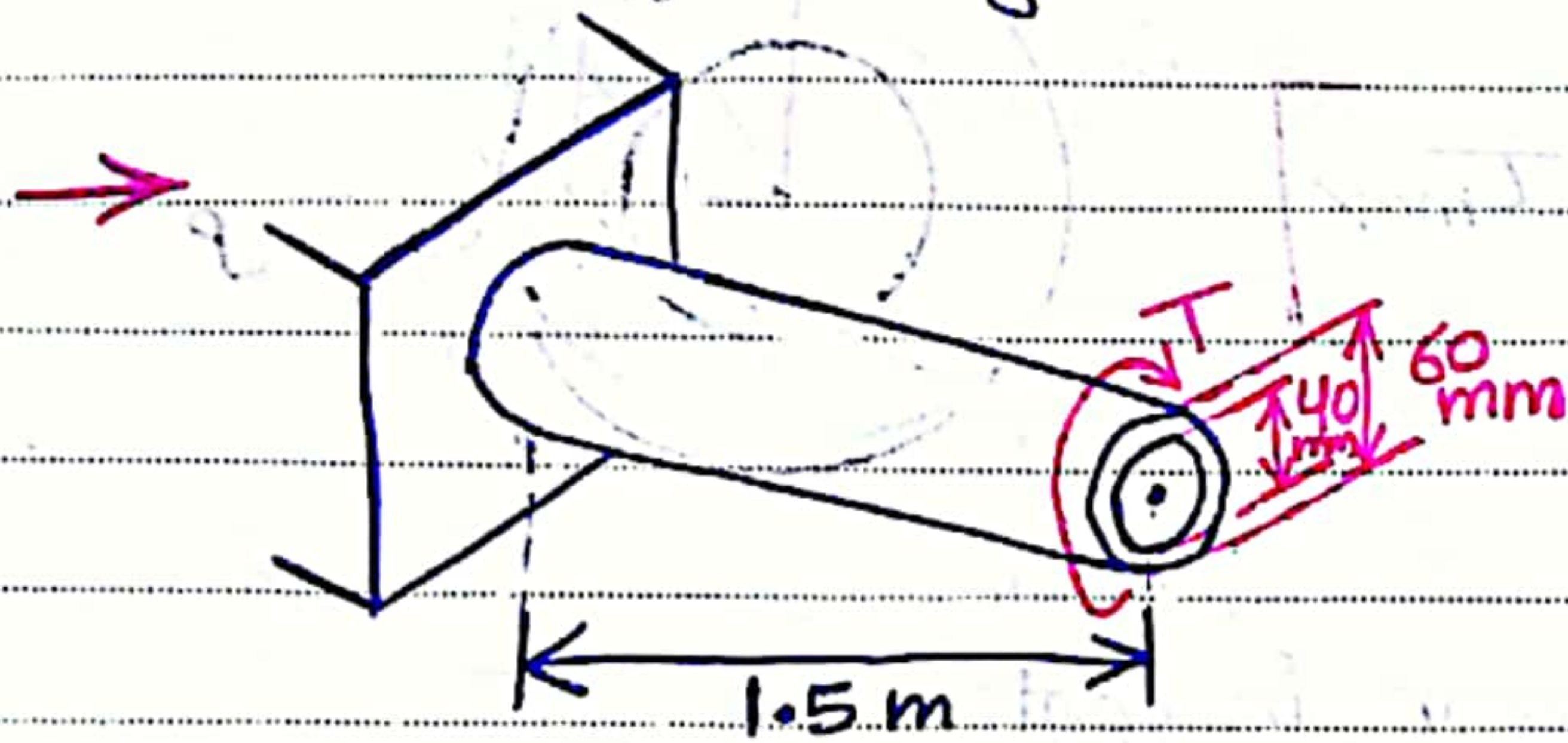
$\rightarrow T = \frac{\tau \cdot J}{\rho}$, Elastic Torsion Formula to express the shearing stress at any distance ρ from the axis of the shaft...

Ex(3.01) 80

A hollow cylindrical steel shaft is 1.5 m long and has inner and outer diameters respectively equal to 40 and 60 mm.

a) What's the largest torque that can be applied to the shaft if the shearing stress isn't to exceed 120 MPa?

b) What's the corresponding min. value of the shearing stress in the shaft?



$$C_1 = 0.02 \text{ m}$$

$$C_2 = 0.03 \text{ m}$$

a-

$$T = \frac{J}{C} \tau_{\max}$$

$$= \frac{\frac{\pi}{2} (C_2^4 - C_1^4) * 120 * 10^6}{C_2}$$

$$= \frac{\frac{\pi}{2} (0.03^4 - 0.02^4) * 120 (10^6)}{0.03 \text{ m}}$$

$$= 4.08 \text{ kN.m}$$

b- the min. value of the shearing stress occurs on the inner surface of the shaft.

$$\tau_{\min} = \frac{0.02}{0.03} (120 * 10^6) = 80 \text{ MPa}$$

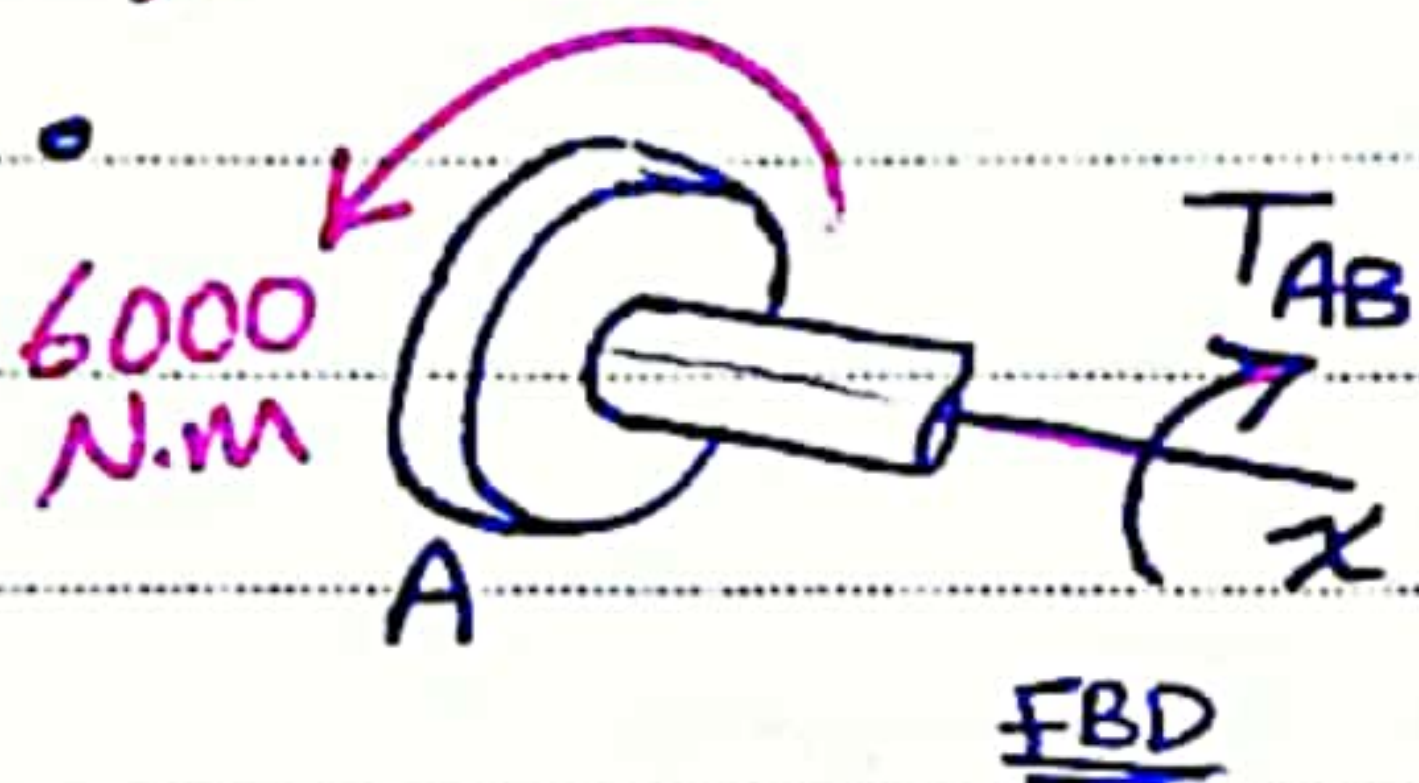
τ_{\min} for solid shaft = zero **NOTE**

Sample Problem (3.1)

Shaft BC is hollow with inner & outer diameters of 90 mm and 120 mm, respectively. Shafts AB and CD are solid and of diameter d . For the loading shown, determine:

- the max. and min. shearing stress in shaft BC?
- the required diameter d of shafts AB and CD if the allowable shearing stress in these shafts is 65 MPa?

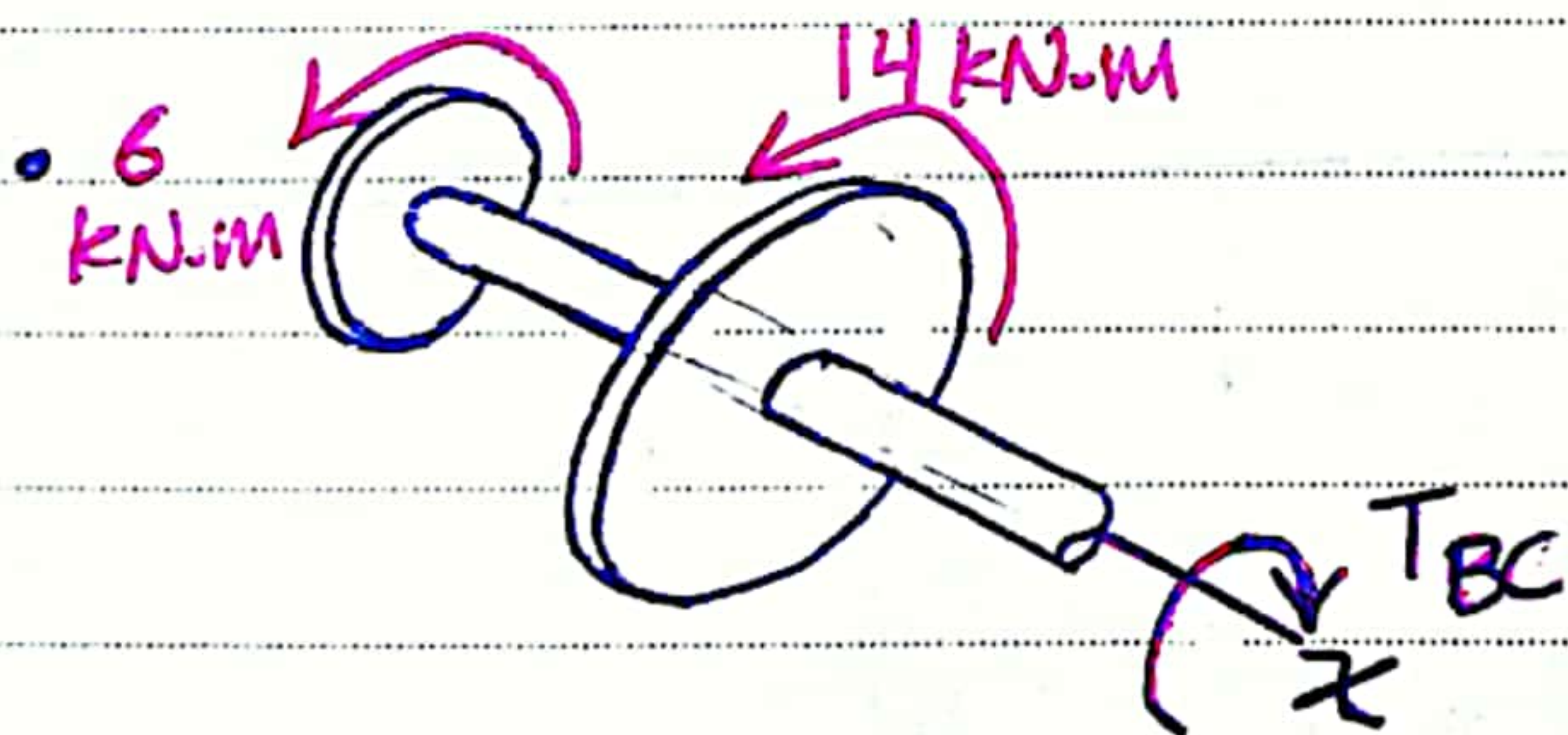
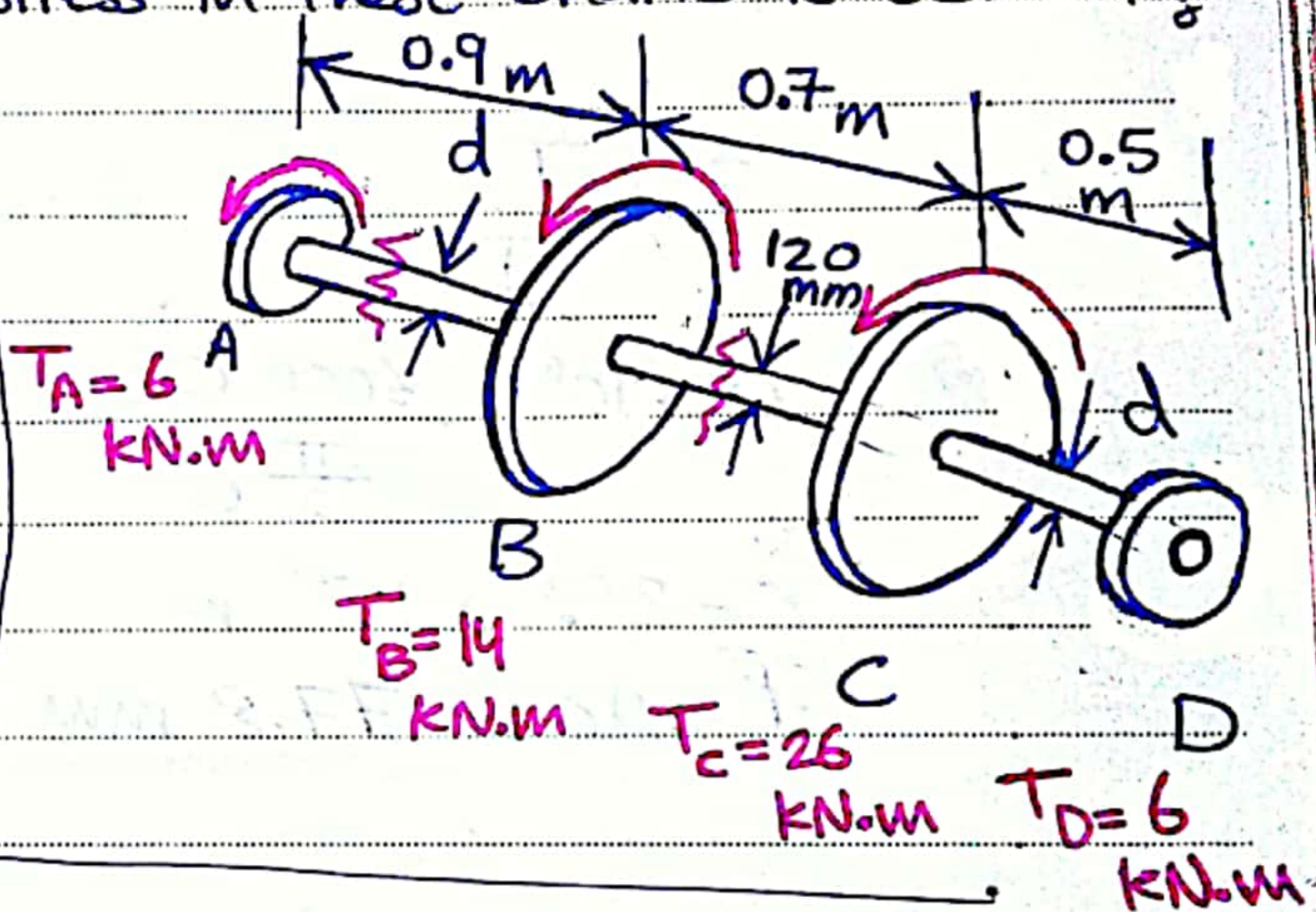
→ Solu.



$$\sum M_x = 0$$

$$6000 - T_{AB} = 0$$

$$T_{AB} = 6 \text{ kN.m}$$



$$\sum M_x = 0$$

$$6000 + 14,000 - T_{BC} = 0$$

$$T_{BC} = 20 \text{ kN.m} \quad \#$$

a- Shaft BC ⇒

$$\tau_{max} = \frac{T_{BC} C_2}{J} = \frac{20,000 \times 0.06}{\frac{\pi}{2} (0.06^4 - 0.045^4)} = 86.2 \times 10^6 \text{ [Pa]}$$

$$\tau_{\min} = \frac{C_1}{C_2} \tau_{\max}$$

$$= \frac{0.045}{0.06} (86.2 \times 10^6)$$

$$= 64.7 \text{ MPa}$$

b- Shafts AB and CD \Rightarrow

$$\tau = \frac{T C}{J}$$

$$65 \times 10^6 = \frac{6000 C}{\frac{\pi}{2} C^4}$$

$$C = 38.9 \times 10^{-3} \text{ m}$$

$$d = 2C = \underline{\underline{77.8 \text{ mm}}}$$

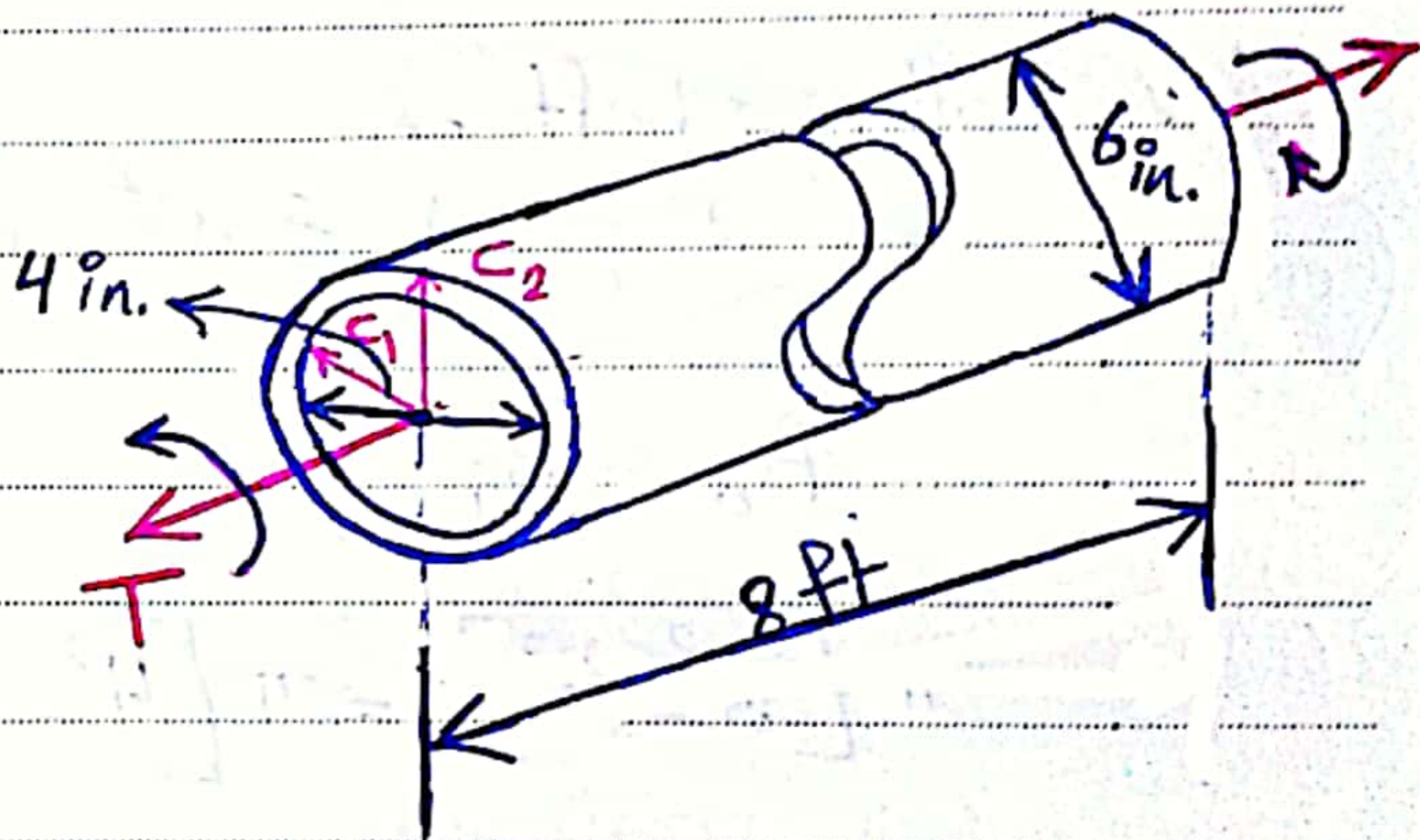
Sample Problem (3.2)

The preliminary design of a large shaft connecting a motor to a generator calls for the use of a hollow shaft with inner and outer diameters of 4 in. and 6 in., respectively. Knowing that the allowable shearing stress is 12 ksi, determine the max. torque that can be transmitted

- by the shaft as designed?
- by a solid shaft of the same weight?
- by a hollow shaft of the same weight and of 8 in. outer diameter?

$$C_2 = 3 \text{ in.}$$

$$C_1 = 2 \text{ in.}$$



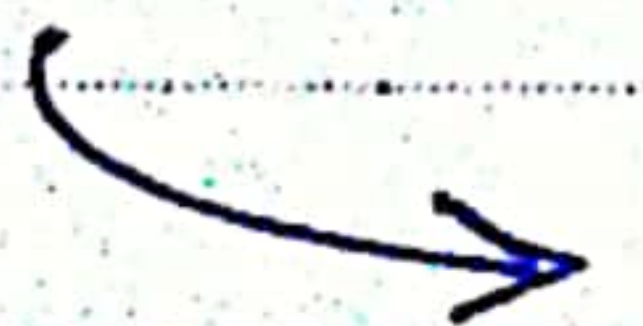
a- as designed...

$$T_{\max} = \frac{T C_2}{J} \rightarrow 12 \text{ ksi} = \frac{T (3 \text{ in.})}{\frac{\pi}{2} (3^4 - 2^4)}$$

$$T = 408 \text{ [kip}\cdot\text{in]}$$

b- solid shaft...

their cross sectional area must be equal



$$A_a = A_b$$

$$\pi [3^2 - 2^2] = \pi C_3^2 \Rightarrow C_3 = 2.24 \text{ in.}$$

$$\text{Since } \tau_{\text{all}} = 12 \text{ ksi, } \tau_{\text{max}} = \frac{T C_3}{J}$$

$$12 \text{ ksi} = \frac{T (2.24)}{\frac{\pi}{2} (2.24)^4}$$

$$\underline{T = 211 \text{ [kip}\cdot\text{in]}}$$

c. hollow shaft ∞ ∞

بعض مكونات المسامير متساوية قبل القطع التي قبل

$$A_a = A_b$$

$$\pi [3^2 - 2^2] = \pi [4^2 - C_5^2]$$

$$C_5 = 3.317 \text{ in.}$$

$$\therefore J = \frac{\pi}{2} [4^4 - 3.317^4] = 212 \text{ [in}^4\text{]}$$

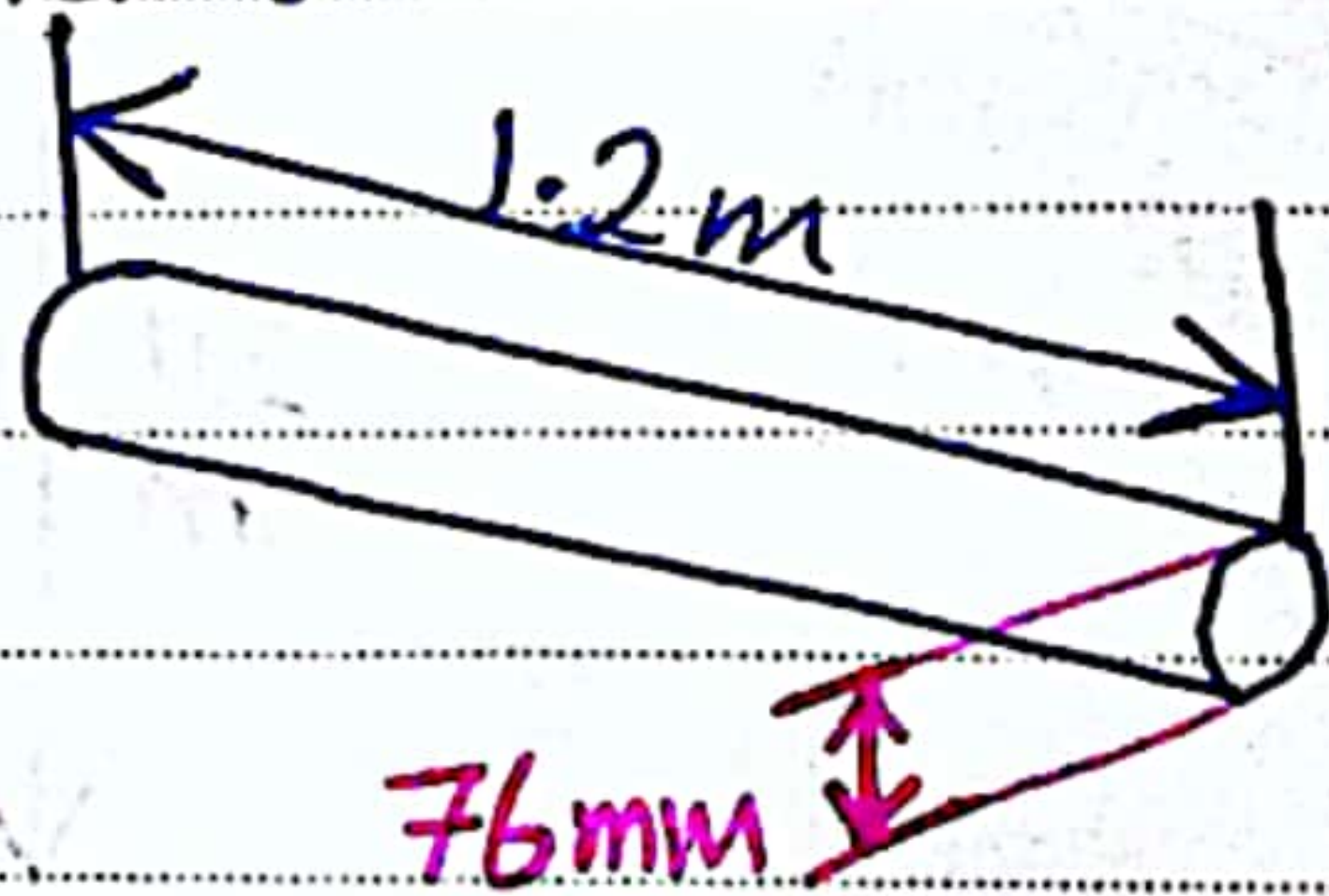
$$\Rightarrow 12 \text{ ksi} = \frac{T (4)}{212}$$

$$T = 636 \text{ [kip}\cdot\text{in]} \#$$

Suggested Problems

3.1) a) Determine the max. shearing stress caused by a 4.6 kN.m torque T in the 76 mm diameter solid aluminum shaft shown.

b) Solve part a, assuming that the solid shaft has been replaced by a hollow shaft of the same outer diameter and of 24 mm inner diameter.



→ a-

$$\tau = \frac{Tc}{J}$$

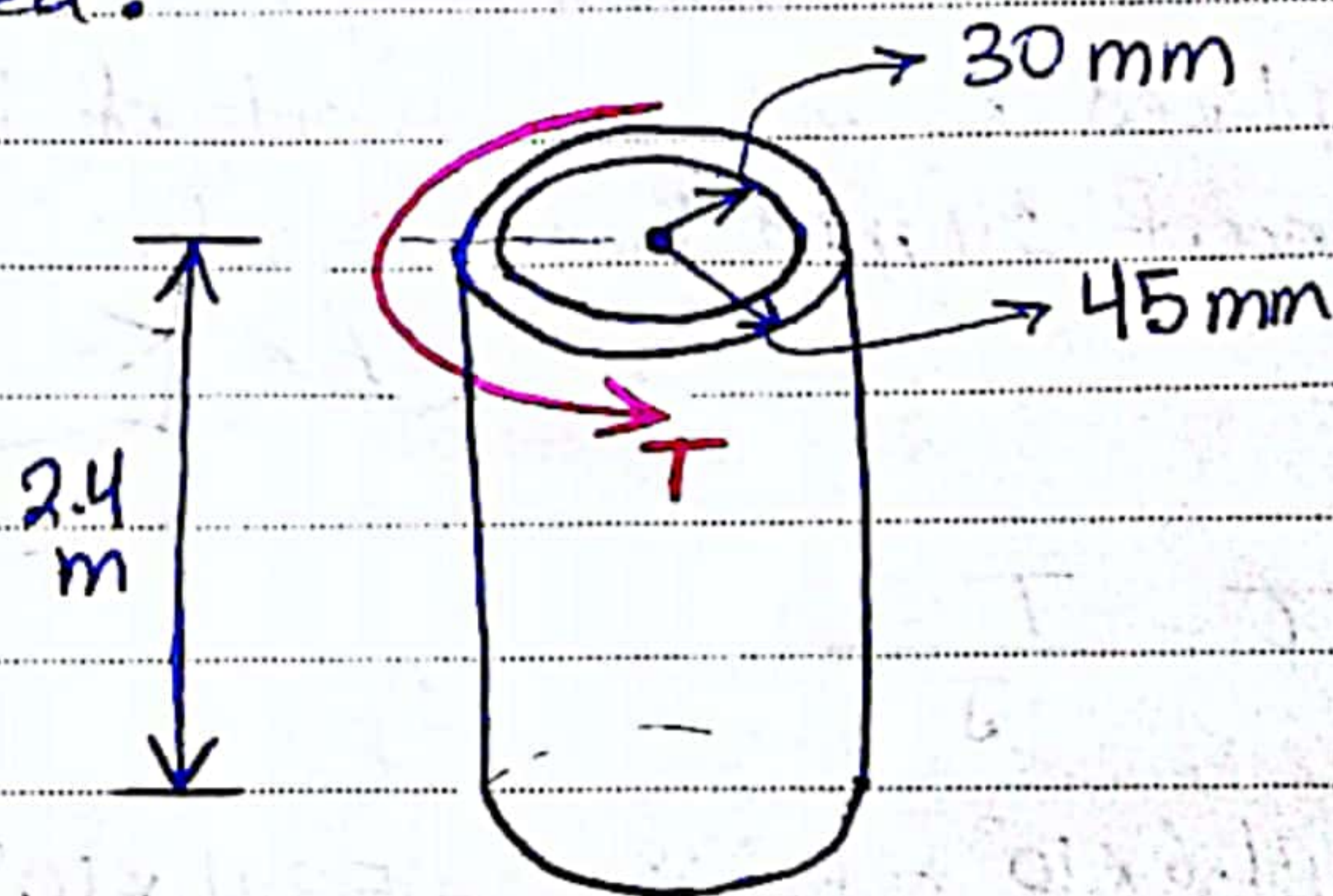
$$= \frac{4.6 \times 10^3 \times 0.038}{3.2753 \times 10^{-6}} = 53.4 \times 10^6 \text{ [Pa]}$$

b- $\tau = \frac{Tc}{J}$

$$= \frac{4.6 \times 10^3 \times 0.038}{\frac{\pi}{2} (0.038^4 - 0.012^4)} = 53.9 \times 10^6 \text{ [Pa]}$$

3.2 a) Determine the torque T that causes a max. shearing stress of 45 MPa in the hollow cylindrical steel shaft shown.

b) Determine the max. shearing stress caused by the same torque T in a solid cylindrical shaft of the same cross sectional area.



a-

$$T = \frac{\tau J}{c}$$

$$= \frac{45 \times 10^6 \times \frac{\pi}{2} (45^4 - 30^4) \times 10^{-6}}{45 \times 10^{-3}}$$

$$= 5.17 \text{ kN}\cdot\text{m}$$

b-

$$\tau = \frac{2T}{\pi c^3} = \frac{Tc}{J}$$

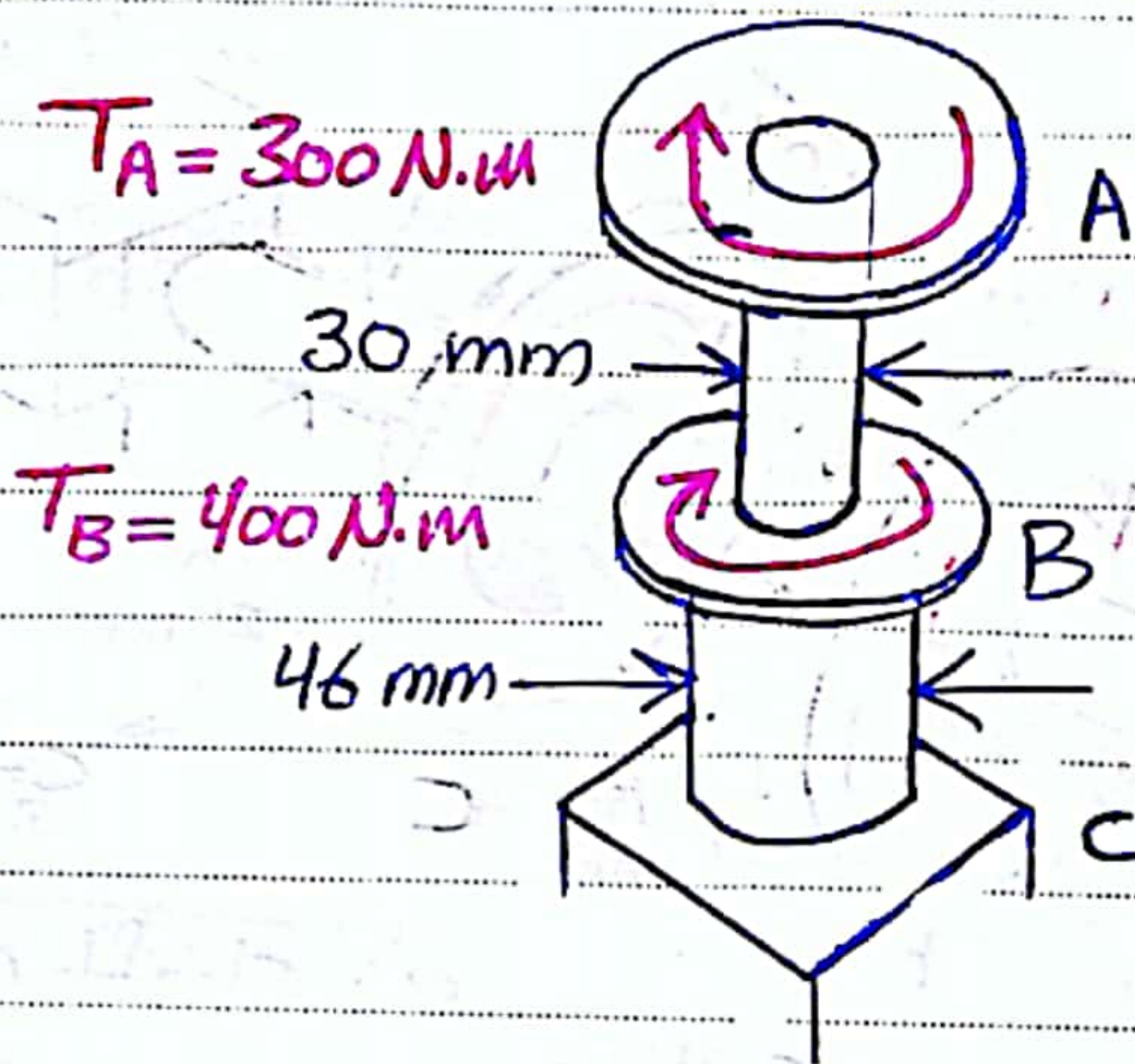
$$A = \pi (c_2^2 - c_1^2)$$

$$= 3.5343 \times 10^3 \text{ mm}^2$$

$$c = \sqrt{\frac{A}{\pi}} = 33.541 \text{ mm}$$

$$= \frac{2 \times 5.1689 \times 10^3}{\pi (0.03354)^3} = 87.2 (10^6) \text{ Pa}$$

3.9) The torques shown are exerted on pulleys A and B. Knowing that both shafts are solid, determine the max shearing stresses in a) shaft AB b) shaft BC



a- $T_{AB} = 300 \text{ N.m}$
 $d = 0.03 \text{ m}$
 $c = 0.015 \text{ m}$

$$\tau_{\max} = \frac{Tc}{J} = \frac{2T}{\pi c^3}$$

$$= \frac{2(300)}{\pi(0.015)^3}$$

$$= 56.588 \times 10^6 \text{ Pa}$$

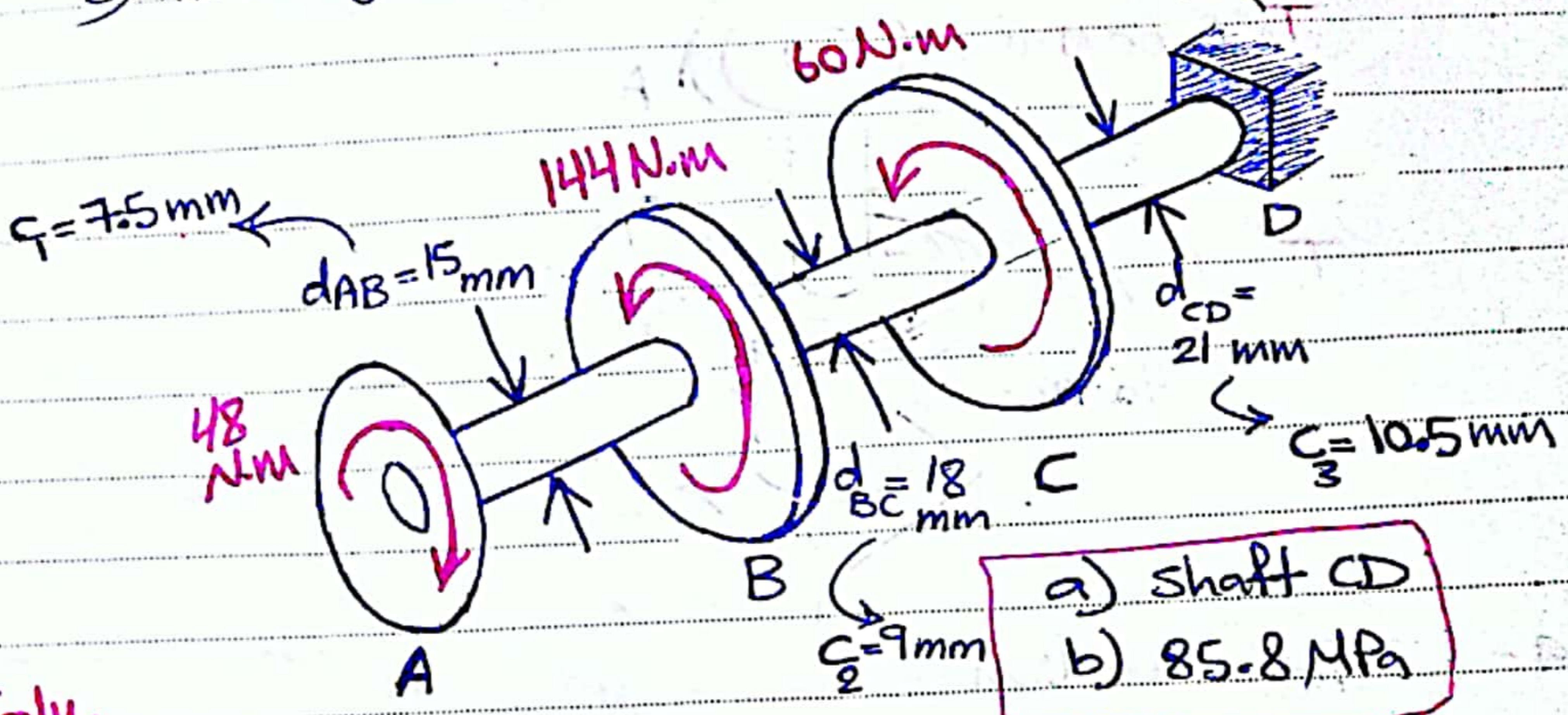
b- $T_{BC} = 300 + 400$
 $= 700 \text{ N.m}$

$d = 0.046 \text{ m}$, $c = 0.023 \text{ m}$

$$\tau_{\max} = \frac{Tc}{J} = \frac{2(T)}{\pi c^3} = \frac{2(700)}{\pi(0.023)^3} = 36.6 \text{ MPa}$$

3.11) Knowing that each of the shafts AB, BC, and CD consists of a solid circular rod, determine:

- a) the shaft in which the τ_{max} occurs
 b) the magnitude of that stress



Solu.

• Shaft AB →
 $T_{AB} = 48 \text{ N.m}$

$$\tau_{max} = \frac{Tc}{J} = \frac{2T}{\pi c^3} = \frac{2(48)}{\pi (0.0075)^3} = 72.433 \text{ MPa}$$

• Shaft BC →
 $T_{BC} = -48 + 144 = 96 \text{ N.m}$

$$\tau_{max} = \frac{2(96)}{\pi (0.009)^3} = 83.835 \text{ MPa}$$

• Shaft CD →
 $T_{CD} = -48 + 144 + 60 = 156 \text{ N.m}$

$$\tau_{max} = \frac{2(156)}{\pi (0.0105)^3} = 85.79 \text{ MPa}$$

ans. for (a)

3.12) Knowing that an 8mm diameter hole has been drilled through each of the shafts AB, BC and CD, determine:
 a- the shaft in which the max. τ occurs?
 b- the magnitude of that stress?

على نفس الشكل
 ليؤاكد

Solu.

Hole $\therefore C_h = 4 \text{ mm}$

Shaft AB $\therefore T_{AB} = 48 \text{ Nm}$

$$\tau_{max} = \frac{T C_1}{J} = \frac{(48)(0.0075)}{\frac{\pi}{2}(0.0075^4 - 0.004^4)} = 78.81 \text{ MPa}$$

Shaft BC $\therefore T_{BC} = 96 \text{ Nm}$

$$\tau_{max} = \frac{T C_2}{J} = \frac{96(0.009)}{\frac{\pi}{2}(0.009^4 - 0.004^4)} = 87.239 \text{ MPa}$$

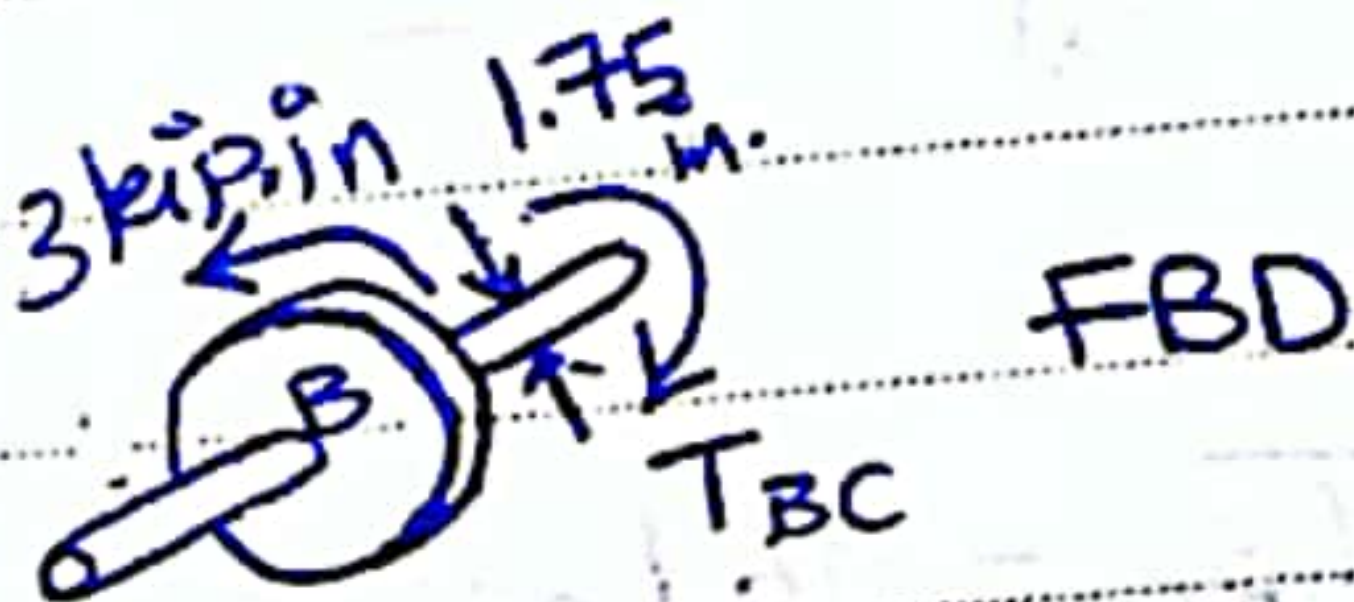
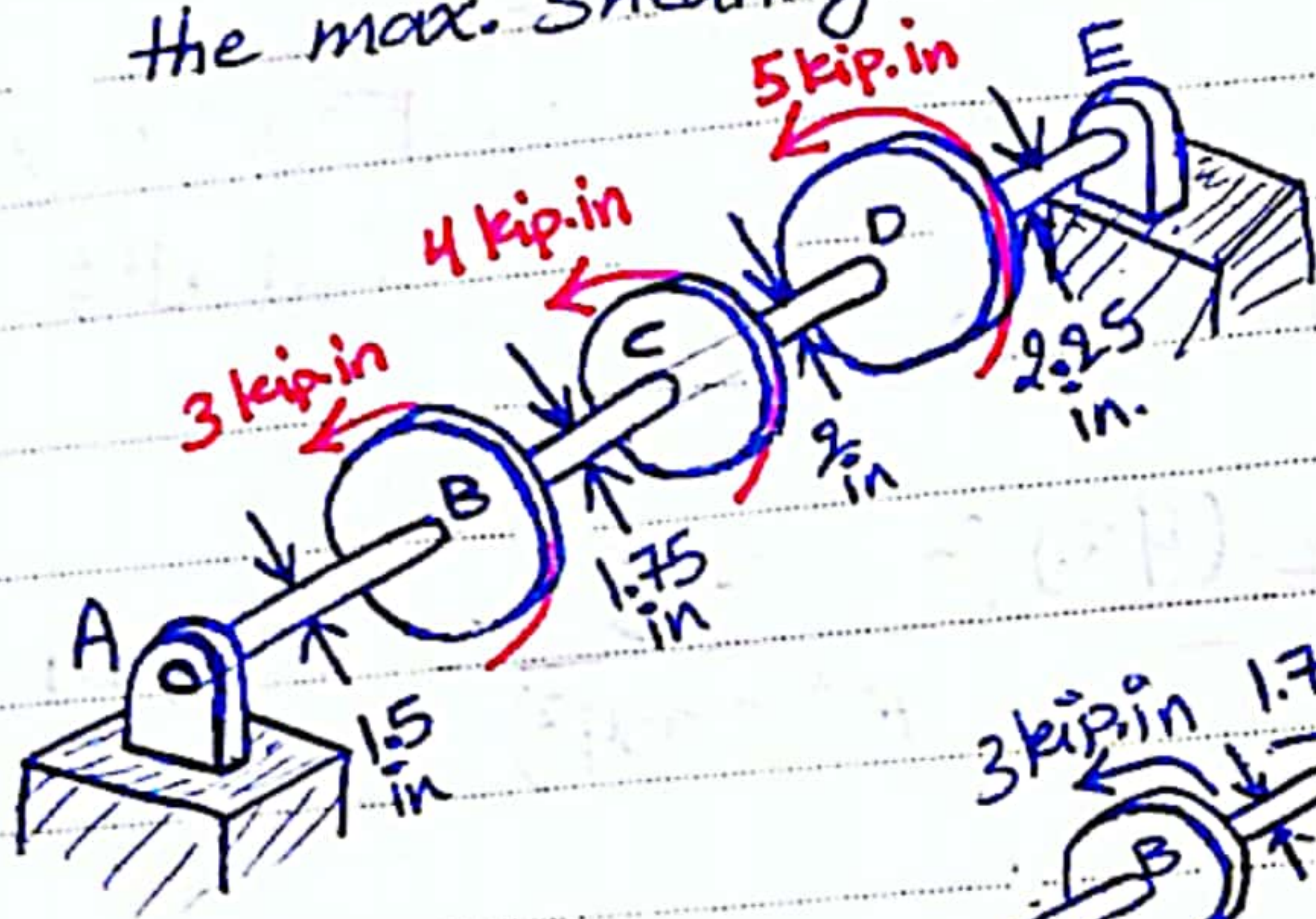
Shaft CD $\therefore T_{CD} = 156 \text{ Nm}$

$$\tau_{max} = \frac{T C_3}{J} = \frac{156(0.0105)}{\frac{\pi}{2}(0.0105^4 - 0.004^4)} = 87.636 \text{ MPa}$$

a- Shaft CD

b- 87.6 MPa

3.13) Under normal operating conditions, the electric motor exerts a 12 kip.in torque at E. Knowing that each shaft is solid, determine:
 the max. shearing stress in a) shaft BC
 b) shaft CD
 c) shaft DE



→ a) shaft BC ∞

$$T_{BC} = 3 \text{ kip.in}$$

$$\tau = \frac{T_C}{J} = \frac{T_C}{\frac{\pi}{2} C^4} = \frac{2T}{\pi C^3} = \frac{2(3) \text{ kip.in}}{\pi \left(\frac{1.75}{2}\right)^3}$$

$$\tau_{BC} = 2.85 \text{ ksi}$$

b) Shaft CD ∞ $T_{CD} = 3 + 4 = 7 \text{ kips.in}$

$$\tau = \frac{2(7)}{\pi \left(\frac{2}{2}\right)^3} = 4.46 \text{ ksi}$$

c) Shaft DE ∞ $T_{DE} = 3 + 4 + 5 = 12 \text{ kips.in}$

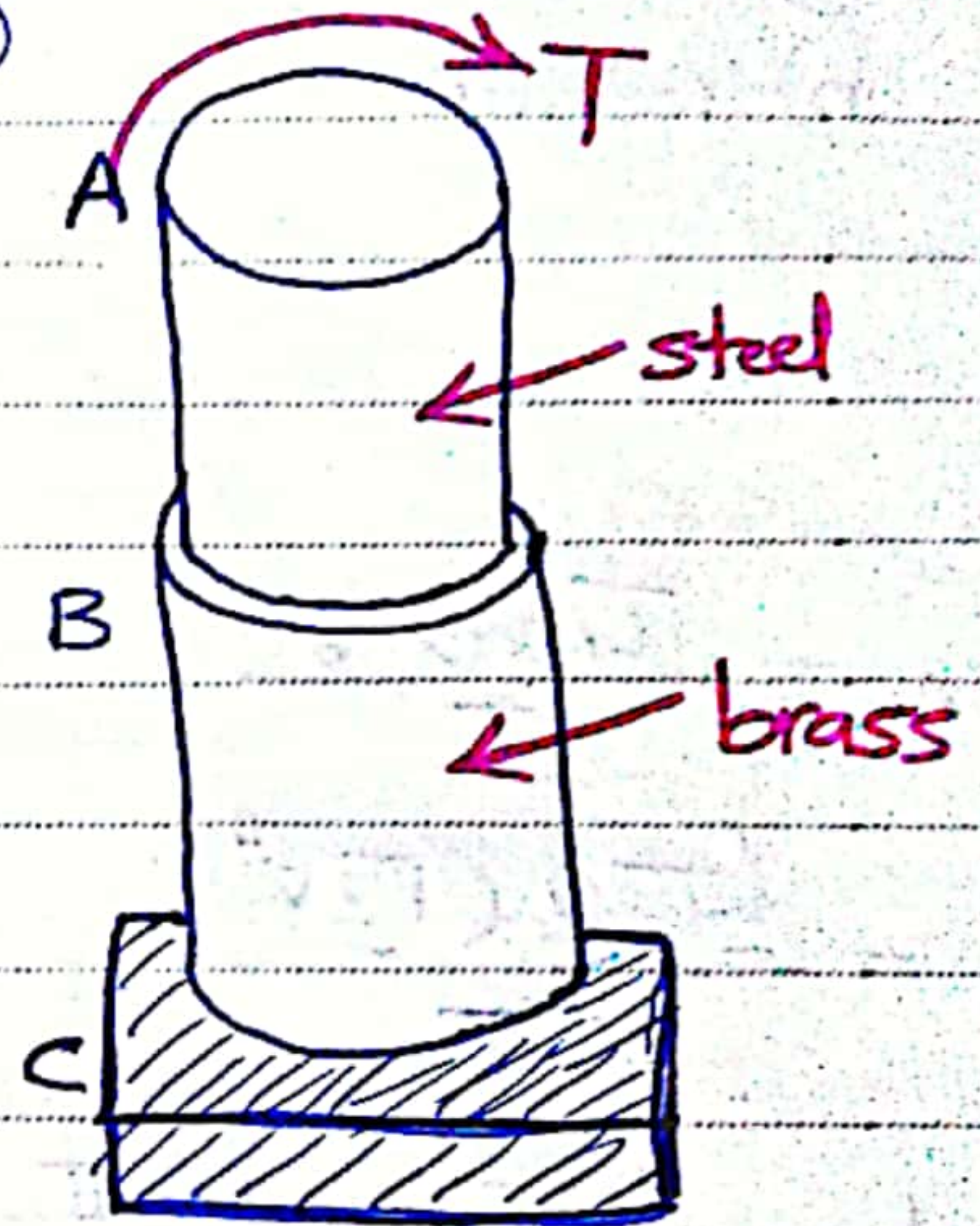
$$\tau = \frac{2(12)}{\pi \left(\frac{2.25}{2}\right)^3} = 5.37 \text{ ksi}$$

#

3.15) The allowable τ is 15 ksi in the 1.5 in. diameter steel rod AB and 8 ksi in the 1.8 in. diameter brass rod BC. Neglecting the effect of stress concentrations, determine the largest torque that can be applied at A?

$$\begin{aligned} \rightarrow T_{AB} &= \left(\frac{\pi}{2} C^3 \tau_{max} \right) \left(\frac{J \tau_{max}}{c} \right) \\ &= \frac{\pi}{2} (0.75^3) (15) \\ &= 9.94 \text{ [kip}\cdot\text{in]} \end{aligned}$$

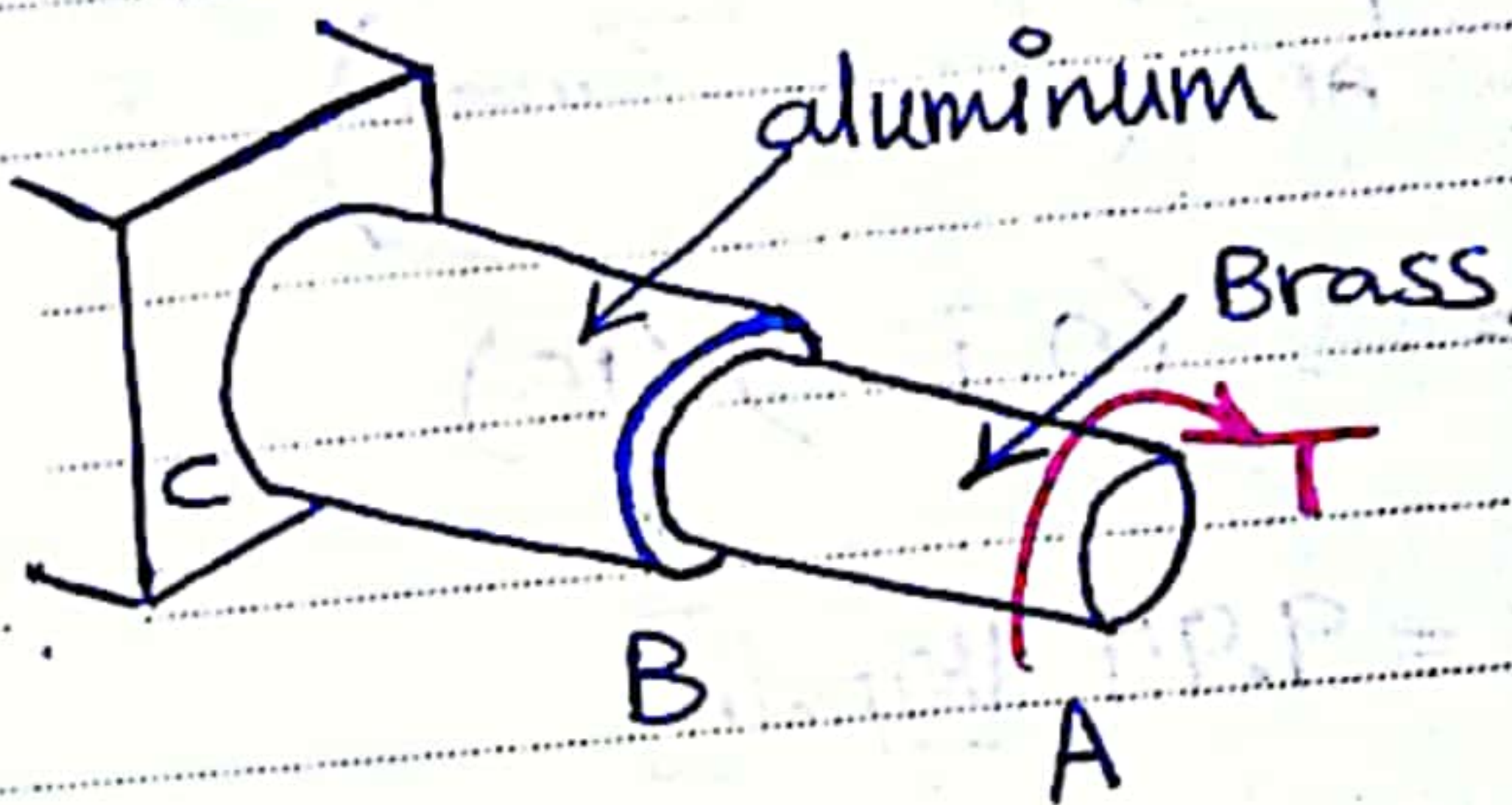
$$\begin{aligned} T_{BC} &= \frac{\pi}{2} (0.9)^3 (8) \\ &= \underline{\underline{9.16 \text{ [kip}\cdot\text{in]}}} \end{aligned}$$



→ the allowable torque is the smaller value 0.0

3.17 The allowable stress is 50 MPa in the brass rod AB and 25 MPa in the aluminium rod BC. Knowing that a torque of magnitude $T = 1250 \text{ N}\cdot\text{m}$ is applied at A, determine the required diameter of:

- rod AB
- rod BC



$$\tau = \frac{T_{\max} r}{J}$$

$$J = \frac{T_{\max} \left(\frac{\pi}{2} C^4\right)}{\tau}$$

$$J = \frac{T_{\max} \left(\frac{\pi}{2} C^4\right)}{\tau} \Rightarrow C^3 = \frac{2T}{\pi \tau_{\max}}$$

$$C_{AB}^3 = \frac{2(1250)}{\pi (50 \times 10^6)} = 15.9 (10^{-6}) \text{ m}^3$$

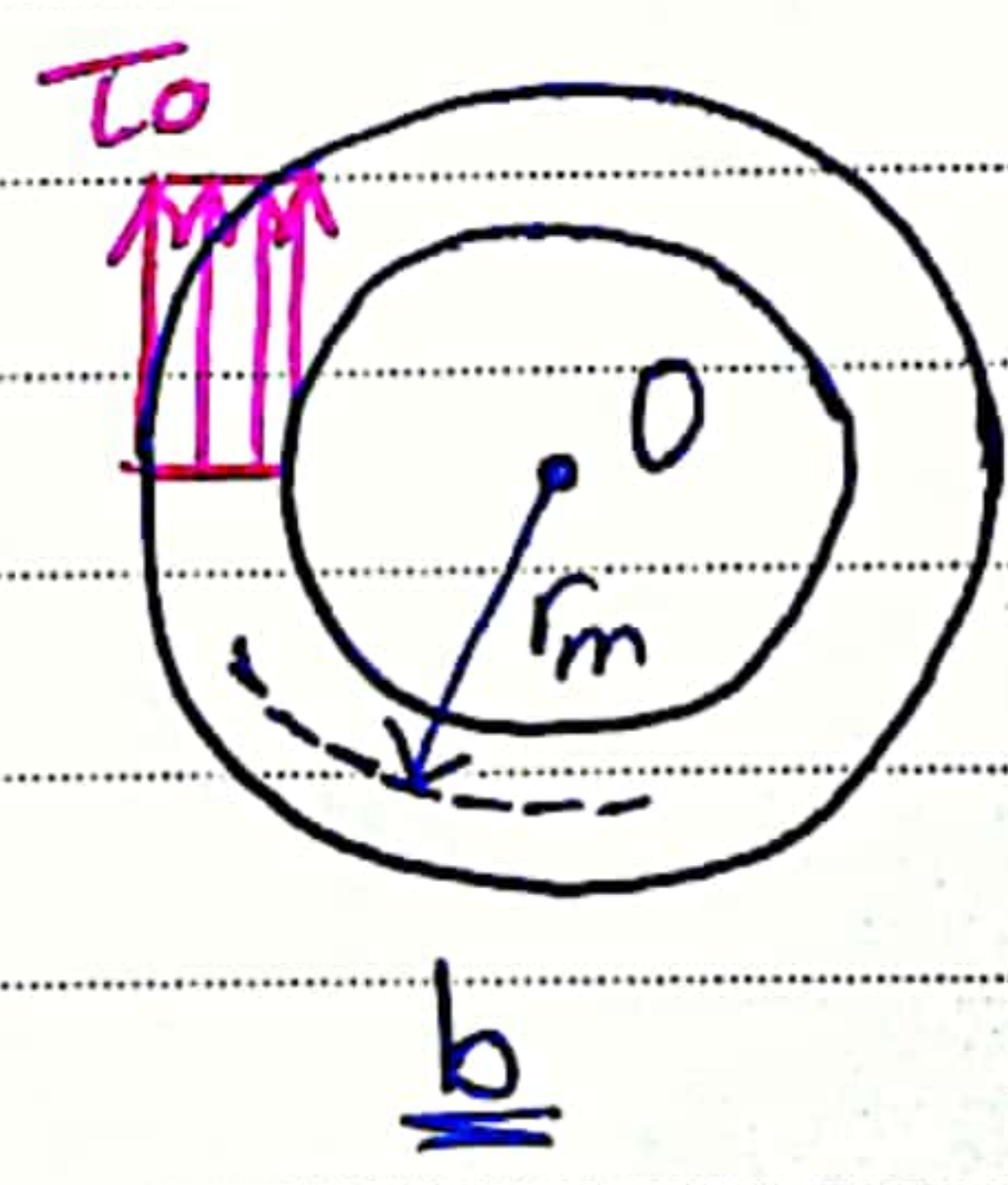
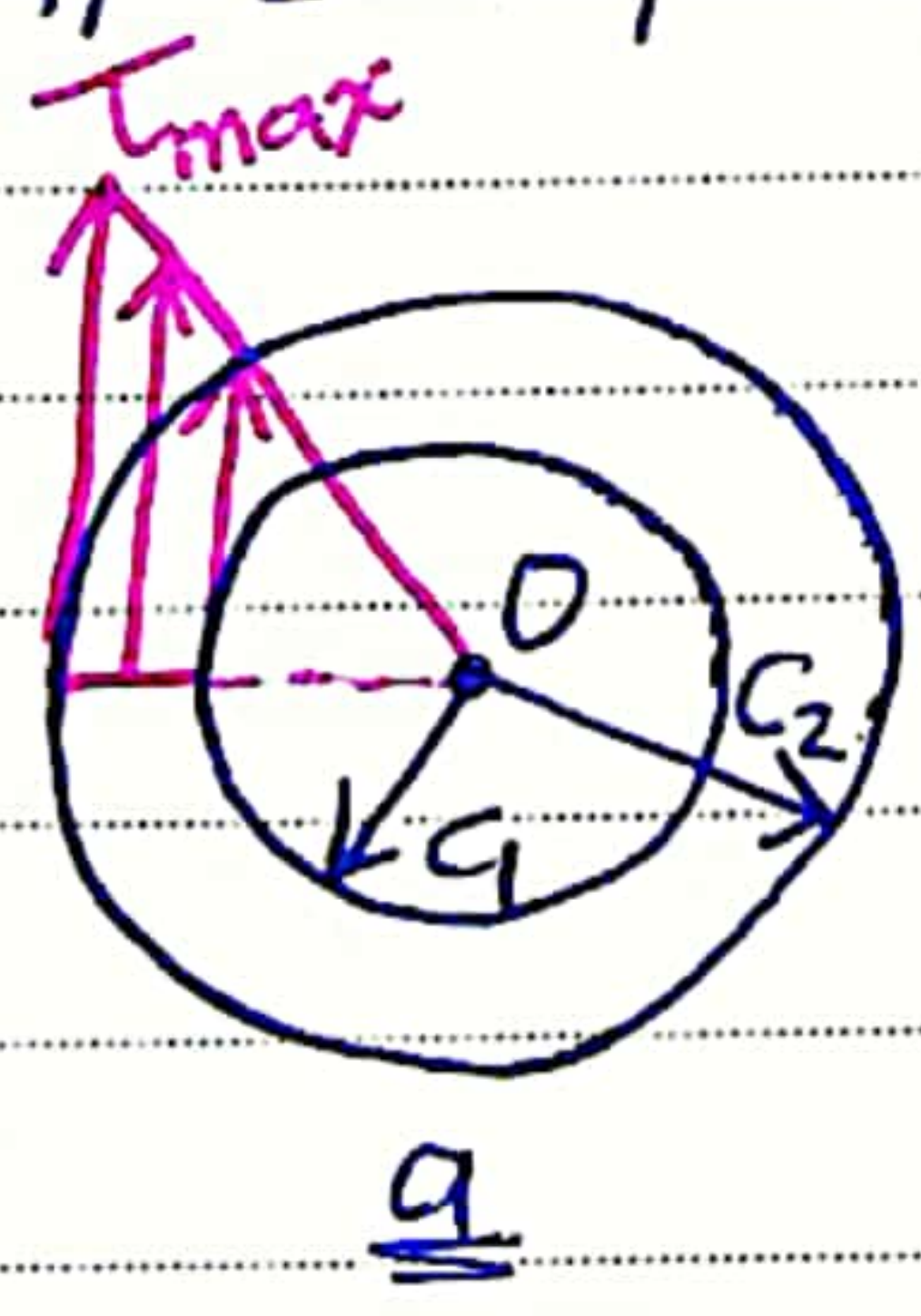
$$C_{AB} = 25.15 \text{ mm} \rightarrow d_{AB} = 50.3 \text{ mm}$$

$$C_{BC}^3 = \frac{2(1250)}{\pi (25 \times 10^6)} = 31.831 \times 10^{-6} \text{ m}^3$$

$$C_{BC} = 31.69 \text{ mm} \rightarrow d_{BC} = 63.4 \text{ mm}$$

3.30) While the exact distribution of the shearing stresses in a hollow cylindrical shaft is as shown below, an approximate value can be obtained for T_{max} by assuming that the stresses are uniformly distributed over the area A of the cross section, as shown in b below, and then further assuming that all of the elementary shearing forces act at a distance from O equal to the mean radius $\frac{1}{2}(C_1 + C_2)$ of the cross section. This approx. value $\tau_0 = T/Ar_m$, where T is applied torque.

Determine the ratio T_{max}/τ_0 of the true value of the max. T and its approximate value τ_0 for values of C_1/C_2 respectively equal to 1, 0.95, 0.75, 0.5 and ϕ



$$\begin{aligned} \rightarrow \text{For a hollow shaft : } T_{max} &= \frac{TC_2}{J} = \frac{2TC_2}{\pi(C_2^4 - C_1^4)} \\ &= \frac{2TC_2}{\pi(C_2^2 - C_1^2)(C_2^2 + C_1^2)} = \frac{2TC_2}{A(C_2^2 + C_1^2)} \end{aligned}$$

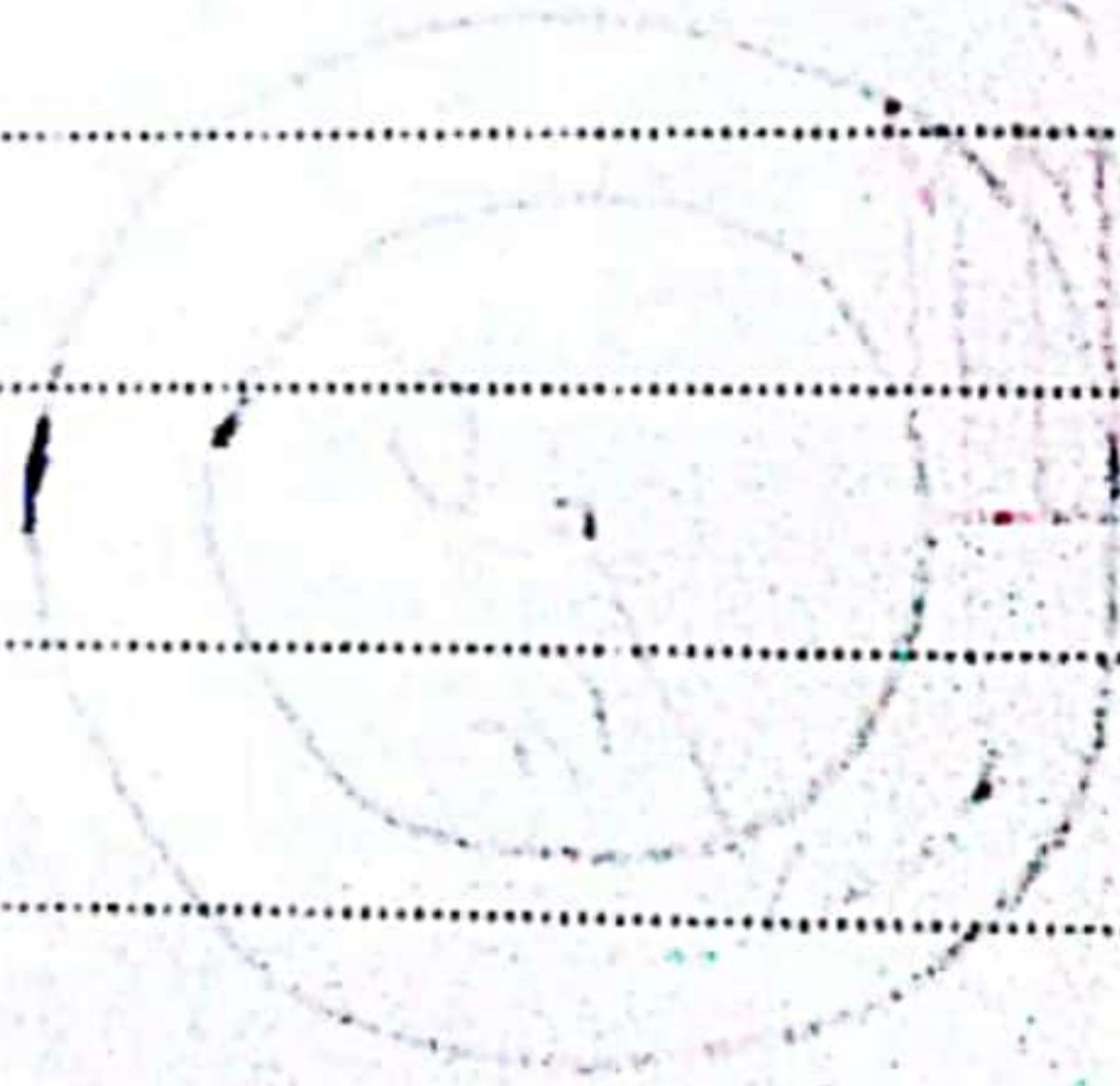
$$\text{By definition } \Rightarrow \tau_0 = \frac{T}{Ar_m} = \frac{2T}{A(C_2 + C_1)}$$

Dividing ...

$$\frac{T_{max}}{T_0} = \frac{C_2(C_2 + C_1)}{C_2^2 + C_1^2} = \frac{1 + (C_1/C_2)}{1 + (C_1/C_2)^2}$$

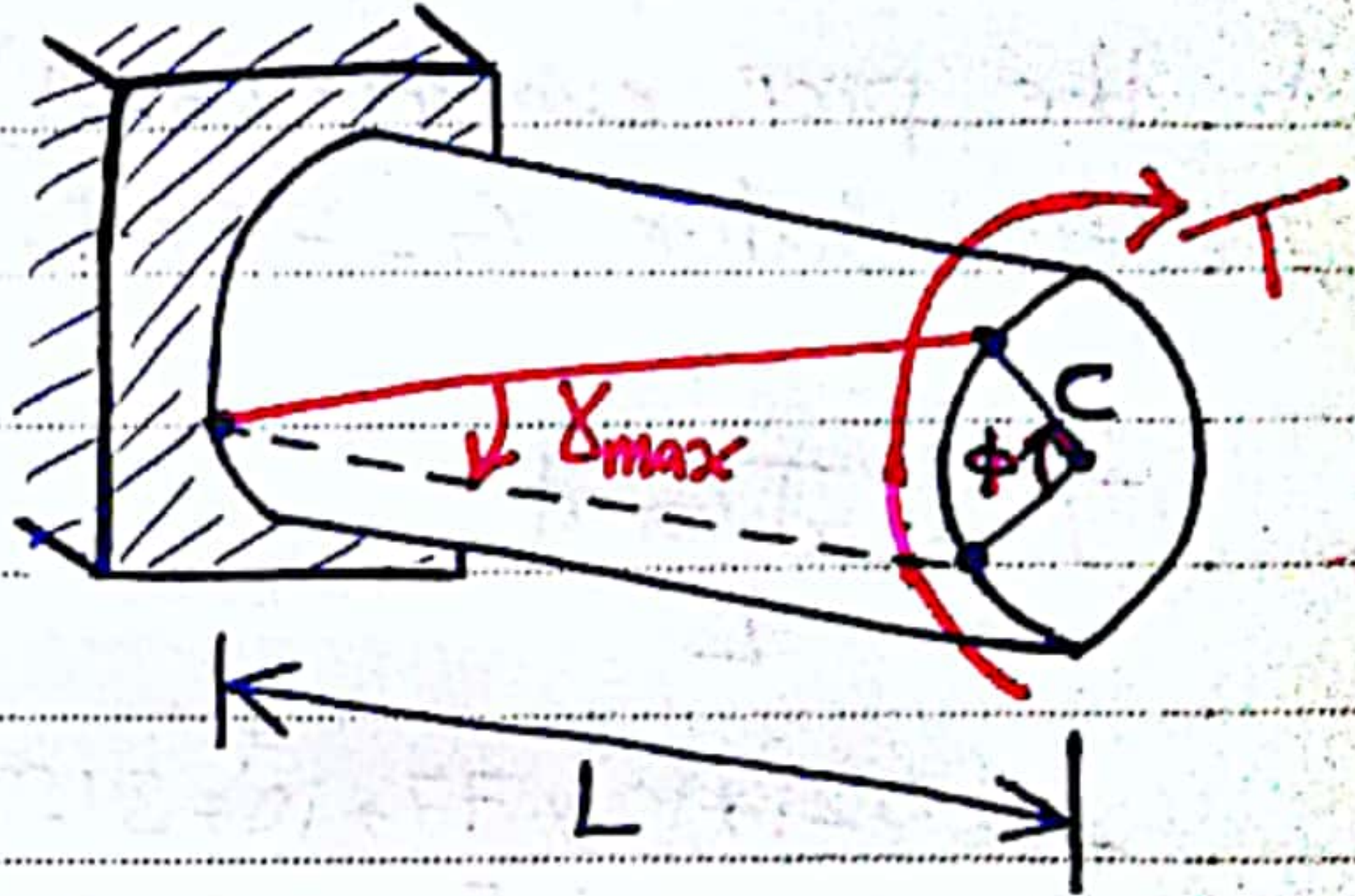
| C_1/C_2 | 1 | 0.95 | 0.75 | 0.5 | 0 |
|---------------|---|-------|-------|------|---|
| T_{max}/T_0 | 1 | 1.025 | 1.120 | 1.20 | 1 |

#



3.5 Angle of Twist in the Elastic Range

The entire shaft will be assumed to remain elastic.



Considering the case of the shaft of length (L) and of uniform cross section of radius (c) subjected to a torque (T) at its free end...

$$\gamma_{max} = \frac{\tau_{max}}{G} = \frac{Tc}{JG}$$

$$\gamma_{max} = \frac{c\phi}{L}$$

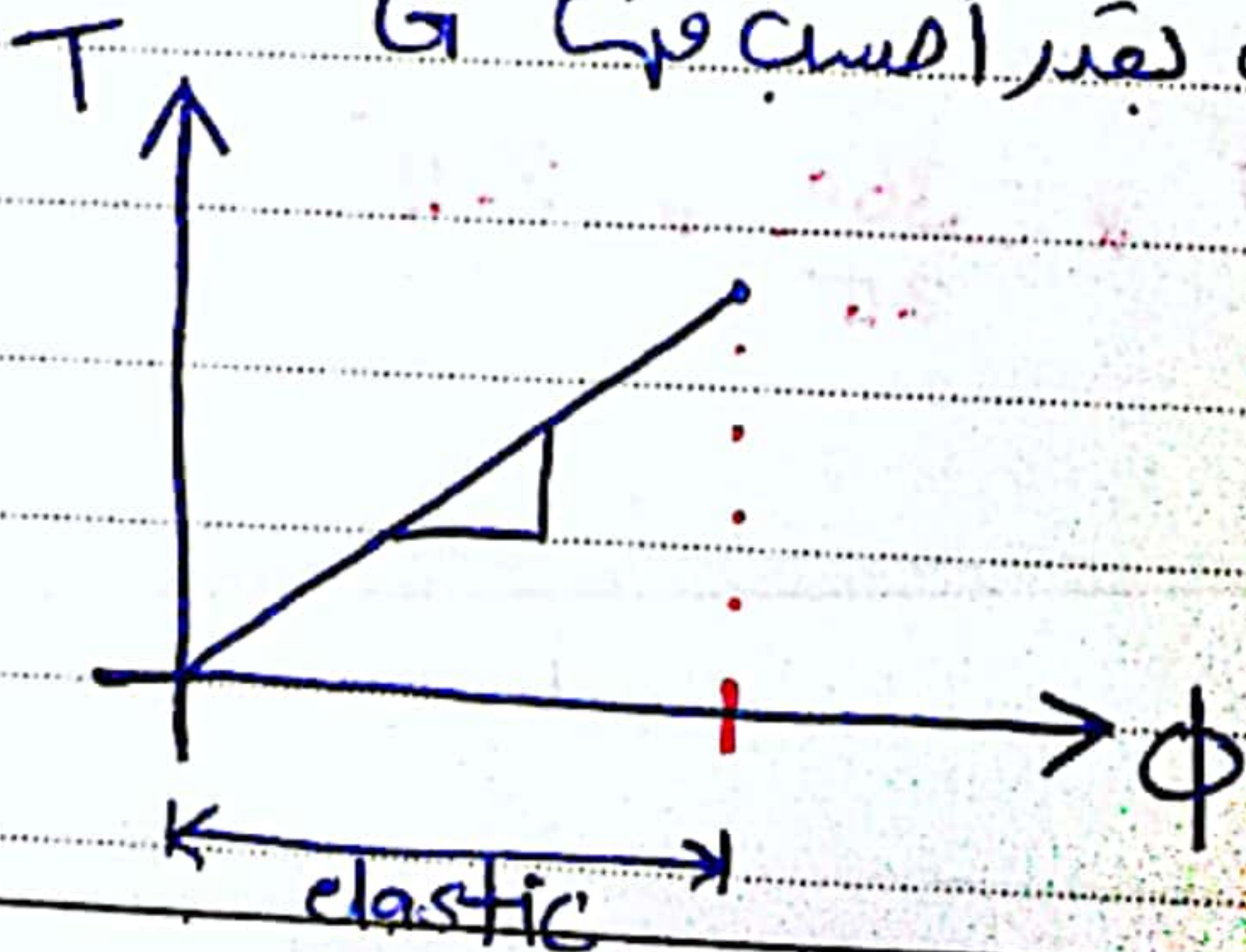
(elastic) المنطقة حيث يمكننا تطبيق هوك's law

$$\Rightarrow \frac{c\phi}{L} = \frac{Tc}{JG}$$

$$\boxed{\phi = \frac{TL}{JG}} \quad [\text{rad}]$$

for a homogenous shaft, and uniform cross section with loading at its end.

* كلما ازداد التورق يزيد ϕ
 * من ذلك معرفة لقيمة ϕ بقدر G و J



منطقة ال elastic العلاقة طردياً

منطقة بين ϕ و T

$$\left(\text{slope} = \frac{JG}{L} \right)$$

Ex (3.02)

What torque should be applied to the end of the shaft of the prev. example (3.01) - to produce a twist of 2° ? Use the value $G = 77 \text{ GPa}$ for the steel shaft.

$$\begin{aligned} \rightarrow T &= \frac{JG\phi}{L} \\ &= \frac{1.021 \times 10^{-6} \times 77 \times 10^9 \times 34.9 (10^{-3})}{1.5} \quad * \left[\phi = 2^\circ \times \frac{2\pi}{360} = 34.9 \times 10^{-3} \text{ rad} \right] \\ &= 1.829 \text{ kN.m} \quad \# \end{aligned}$$

$L = 1.5 \text{ m}$
 $G = 77 \times 10^9 \text{ Pa}$

Ex (3.03)

What angle of twist will create a shearing stress of 70 MPa on the inner surface of the hollow steel shaft of examples (3.01) (3.02) بعبارة min. $\left[\frac{\tau}{G} \right]$

$$\begin{aligned} \rightarrow \gamma_{\min} &= \frac{\tau_{\min}}{G} = \frac{70 (10^6)}{77 (10^9)} \\ &= 909 \times 10^{-6} \\ \phi &= \frac{L \gamma_{\min}}{C_1} = \frac{1500 [\text{mm}] \cdot 909 (10^{-6})}{20 [\text{mm}]} \\ &= 68.2 \times 10^{-3} [\text{rad}] \quad * \frac{360}{2\pi} = 3.91^\circ \end{aligned}$$

طبعاً أنه في التورك
التي يبلغ على هيئة
 τ و γ في التورك
بطلع ϕ

If the shaft is subjected to torques at locations other than its end, or if it consists of several portions with various cross sections of different materials ...

$$\phi = \sum_i \frac{T_i L_i}{J_i G_i}$$

de sasu ϕ o, L_i
internal torque

3.6 Statically Indeterminate Shafts

Because statics isn't sufficient in some cases to determine the external and internal torques, the shafts are said to be statically indeterminate.

Ex(3.04)

Knowing that $r_A = 2r_B$, determine the angle of rotation of end E of shaft BE when the torque T is applied at E?

→ ϕ_A بالانقلاب في A
 في A يكون مساوي لل Shaft ϕ_A
 due to $r_A = 2r_B$

$$\phi_A = \frac{TAD L}{JG} = \frac{2TL}{JG}$$

$$r_A \phi_A = r_B \phi_B \Rightarrow \phi_B = \frac{r_A}{r_B} \phi_A = 2\phi_A$$

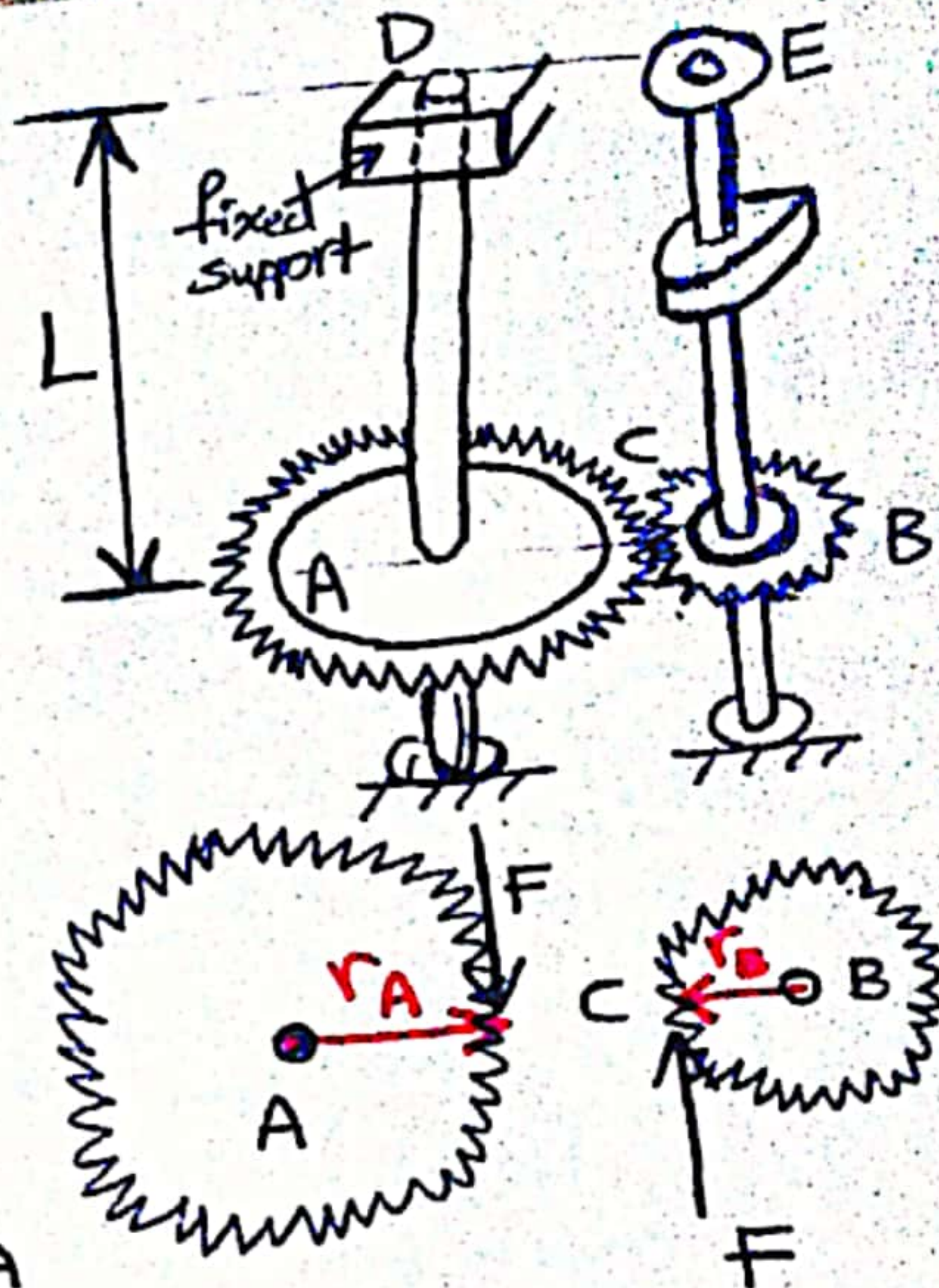
$$\phi_B = \frac{4TL}{JG}$$

$$* \phi_{E/B} = \frac{T_{BE} L}{JG}$$

∴ the angle of rotation of end E is obtained by writing ∴

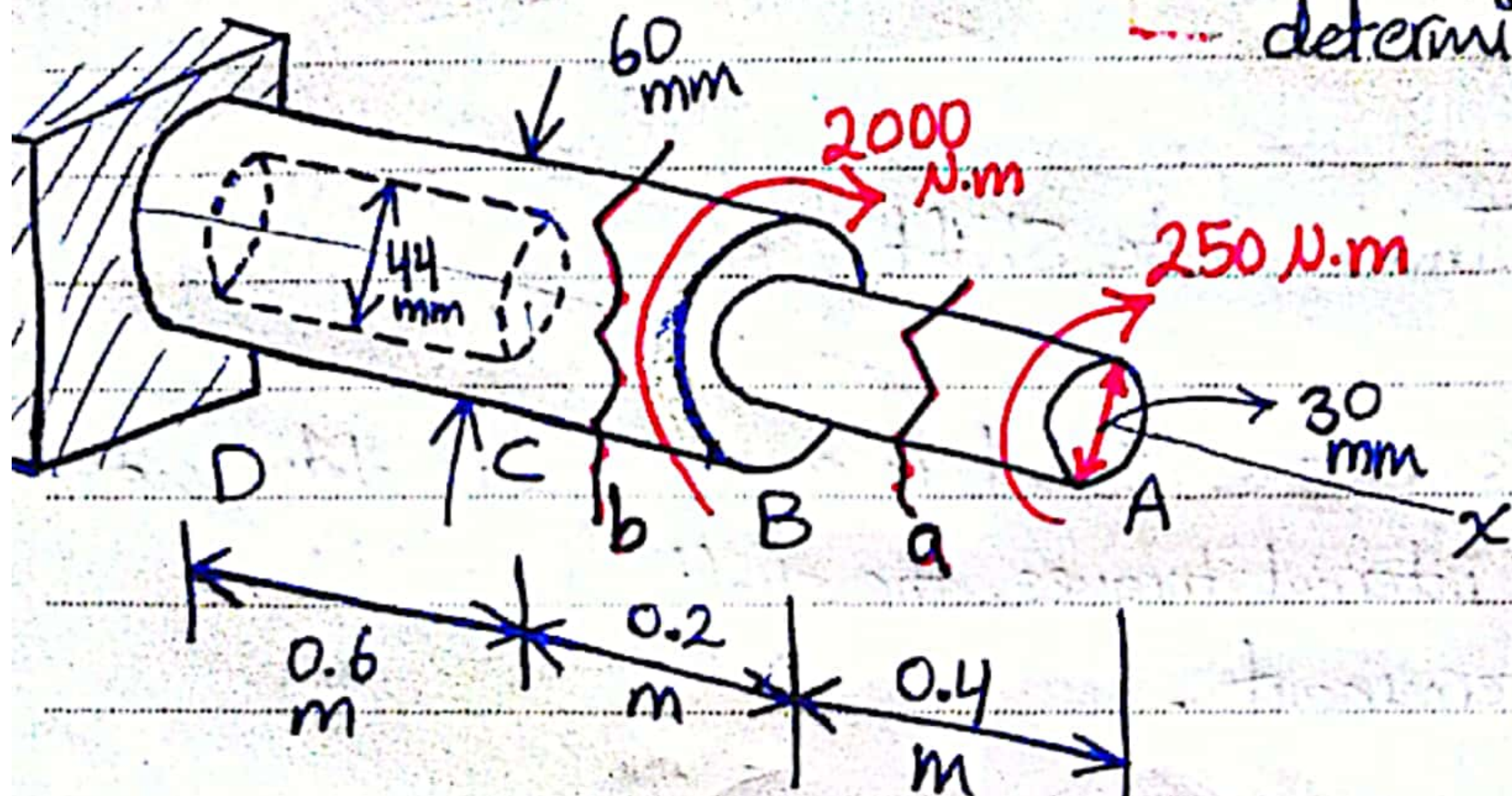
$$\phi_E = \phi_B + \phi_{E/B}$$

$$= \frac{4TL}{JG} + \frac{TL}{JG} = \frac{5TL}{JG} \quad \#$$



Sample Problem (3.3)

The horizontal shaft AD is attached to a fixed base at D and is subjected to the torques shown. A 44 mm diameter hole has been drilled into portion CD of the shaft. Knowing that the entire shaft is made of steel for which $G = 77 \text{ GPa}$, determine ϕ at end A?



→ this shaft consists of 3 portions (AB, BC, CD)

by taking sec. (a) $\Rightarrow 250 - T_{AB} = 0$ $T_{AB} = 250 \text{ N.m}$

sec. (b) $\Rightarrow 250 + 2000 - T_{BC} = 0$ $T_{BC} = 2250 \text{ N.m}$

$T_{BC} = T_{CD}$ لأنه في نقطة C القوة

في كل نقطة القوة
فوقها أقل من أدناه
في المبدأ

$$\phi_A = \sum \frac{T_i L_i}{J_i G} = \frac{1}{G} \left(\frac{T_{AB} L_{AB}}{J_{AB}} + \frac{T_{BC} L_{BC}}{J_{BC}} + \frac{T_{CD} L_{CD}}{J_{CD}} \right)$$

$$\phi_A = \frac{1}{77 \times 10^9} \left[\frac{250 \times 0.4}{\frac{\pi}{2} (0.015)^4} + \frac{2250 \times 0.2}{\frac{\pi}{2} (0.03)^4} + \frac{2250 \times 0.6}{\frac{\pi}{2} (0.03^4 - 0.022^4)} \right]$$

$$\phi_A = 0.0403 \text{ rad}$$

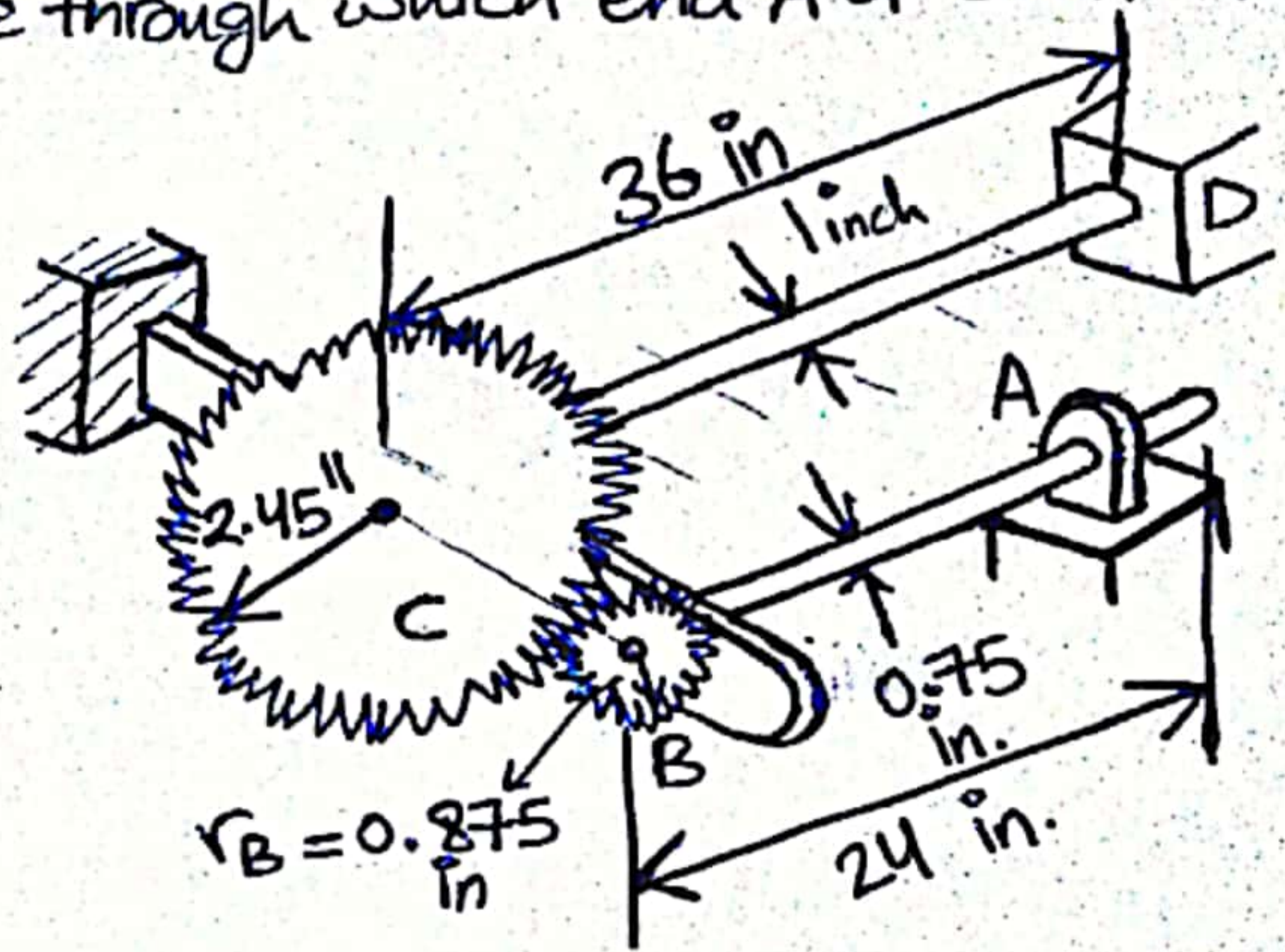
$$= 2.31^\circ$$

$$\leftarrow * \frac{360^\circ}{2\pi}$$

Sample Problem (3.4)

Two solid steel shafts are connected by the gears shown. Knowing that for each shaft $G = 11.2 \times 10^6$ psi and that the allowable shearing stress is 8 ksi. Determine:

- the largest torque T_0 that may be applied to end A of shaft AB
- the corresponding angle through which end A of shaft AB rotates?



→ Gear B: $\sum M_B = 0$

$$F(0.875) - T_0 = 0$$

Gear C: $\sum M_C = 0$

$$F(2.45) - T_{CD} = 0$$

$$T_{CD} = 2.8 T_0$$

$$r_B \phi_B = r_C \phi_C \Rightarrow \phi_B = \phi_C \frac{2.45}{0.875} = 2.8 \phi_C$$

a. $8000 \text{ psi} = \frac{T_0 (0.375)}{\frac{\pi}{2} (0.375)^4} \Rightarrow T_0 = 663 \text{ lb}\cdot\text{in}$ / Shaft AB

$8000 = \frac{2.8 T_0 (0.5)}{\frac{\pi}{2} (0.5)^4} \Rightarrow T_0 = 561 \text{ lb}\cdot\text{in}$ / Shaft CD

$$T_{AB} = T_0$$

$$T_0 (2.8) = T_{CD}$$

Max. permissible Torque: $T_0 = 561 \text{ lb}\cdot\text{in}$ *لا بد ان لا يتجاوز!*

b. $\phi_A = \phi_B + \phi_{A/B} = 2.8 \phi_C + \phi_{A/B}$

$$\phi_A = \frac{2.8 (2.8 \times 561 \times 36)}{\frac{\pi}{2} (0.5)^4 (11.2 \times 10^6)} + \frac{(561 \times 24)}{\frac{\pi}{2} (0.375)^4 (11.2 \times 10^6)}$$

$$= 8.26^\circ + 2.22^\circ = 10.48^\circ$$

#

Moment \rightarrow Torsion (ch.3) \leftarrow axial axis \leftarrow المومنت اللى صوائليه ال axial axis
 \rightarrow Bending (ch.4) \leftarrow axial \leftarrow المومنت اللى صوائليه أى محور \leftarrow lateral \leftarrow بال

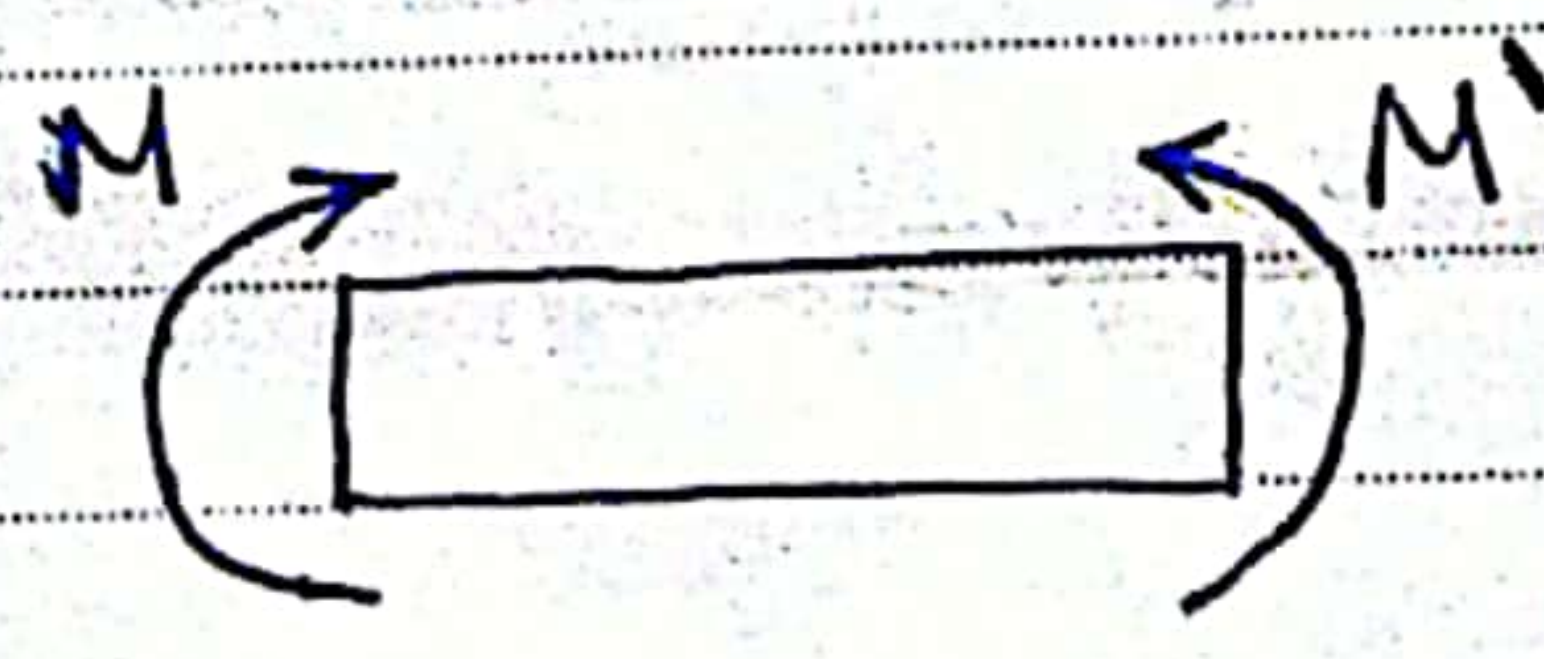
Ch.4 Pure Bending

4.1 Introduction

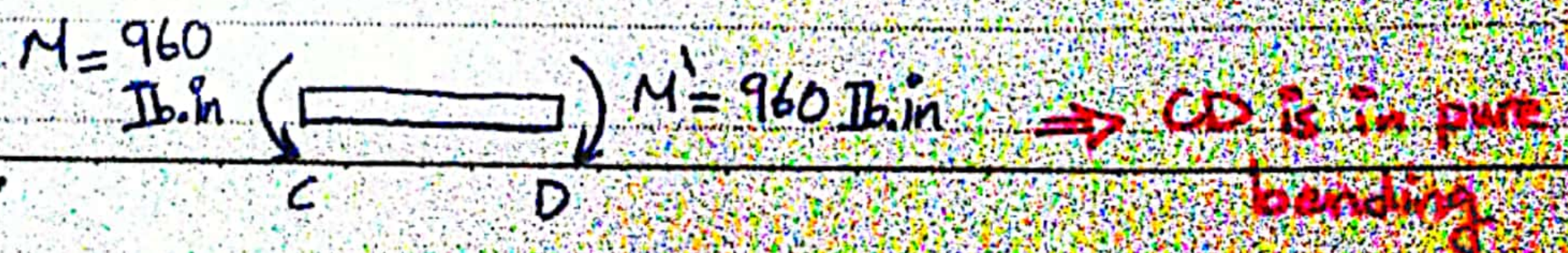
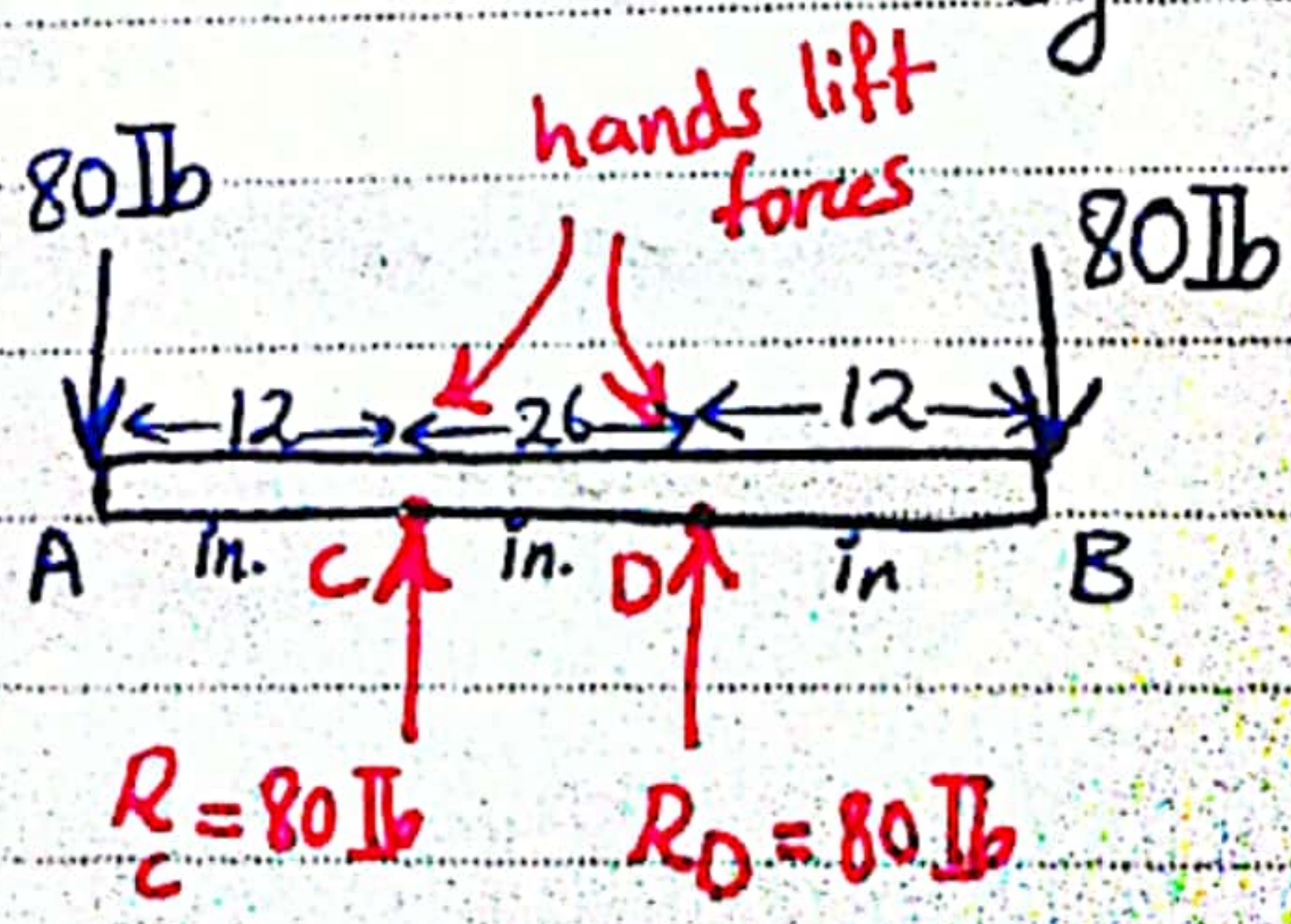
In this chapter, we'll analyze the stresses and strains in prismatic members subjected to bending.

In addition, it will be devoted to the analysis of prismatic members subjected to equal & opposite couples M and M' acting in the same longitudinal plane.

\therefore Members said to be in **PURE** bending.

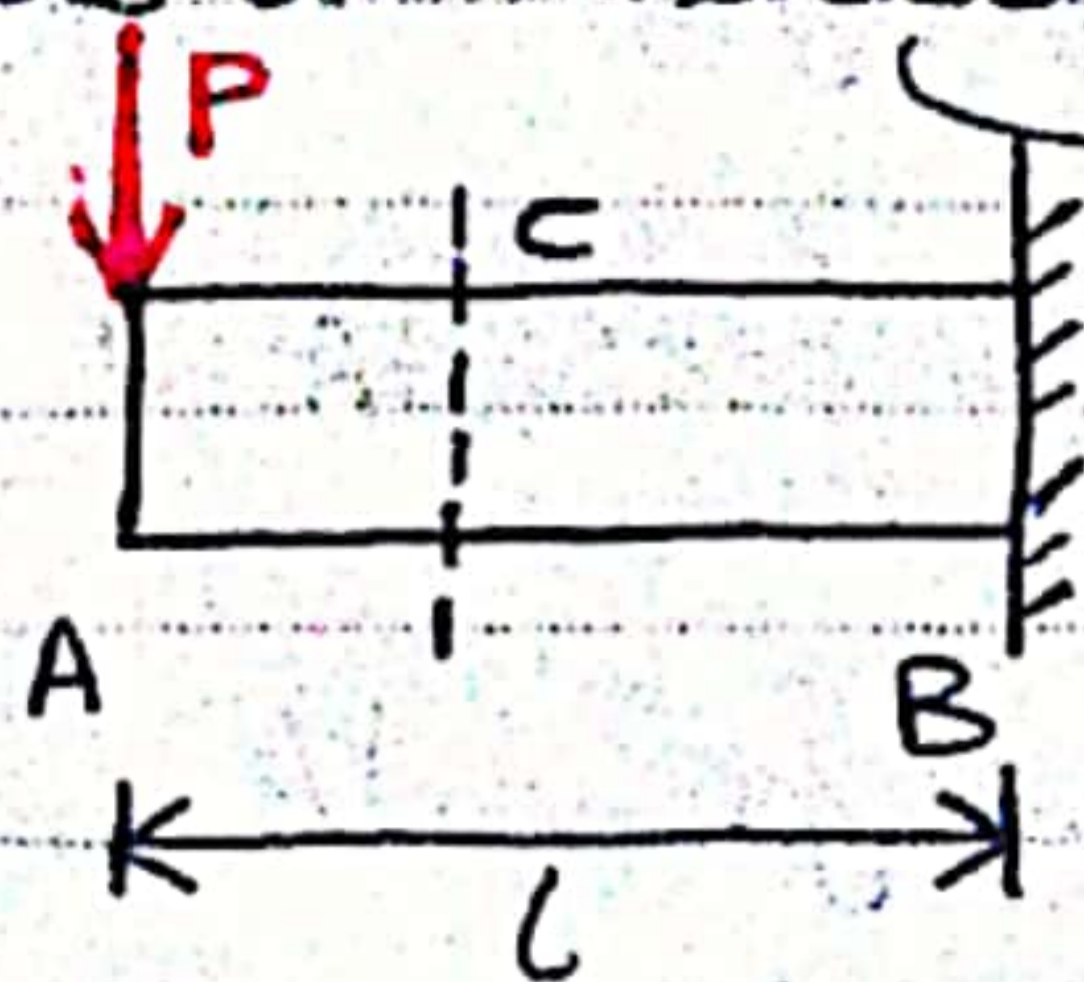


* An example of pure bending: the bar as it's held overhead by a weight lifter.



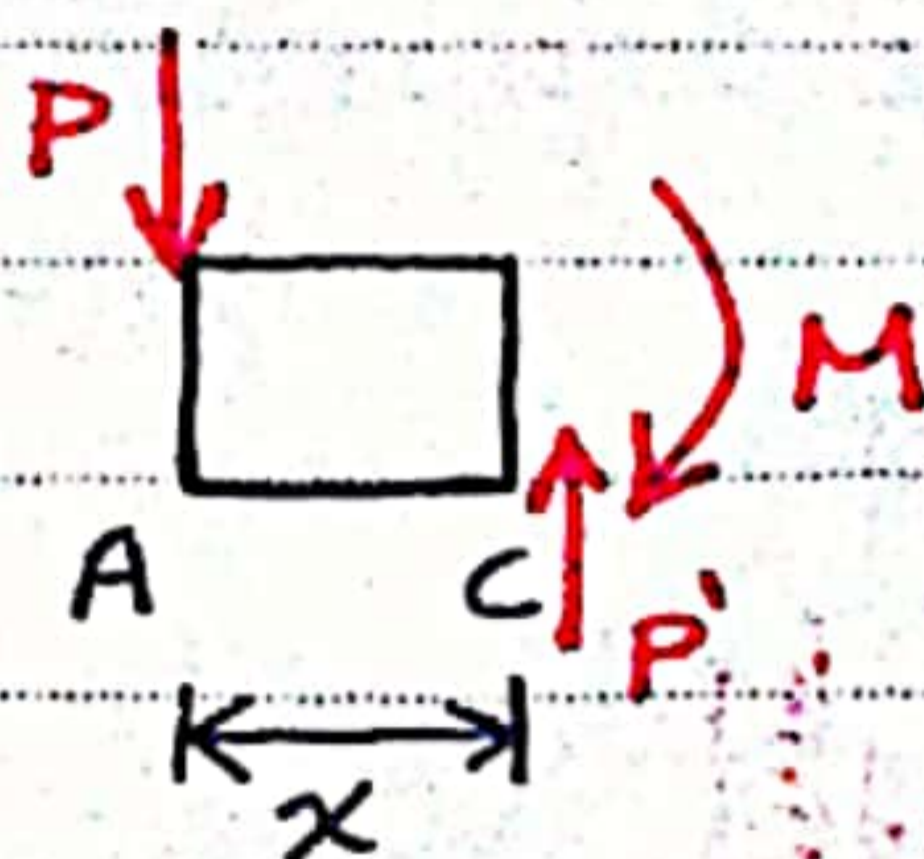
* study of pure bending \equiv study of beams (prismatic members subjected to various types of transverse loads)

لو أخذنا سكيناً من القعدة C
 من أجلها من الـ FBD للسكين هاد
 انه عند فيه (internal forces)

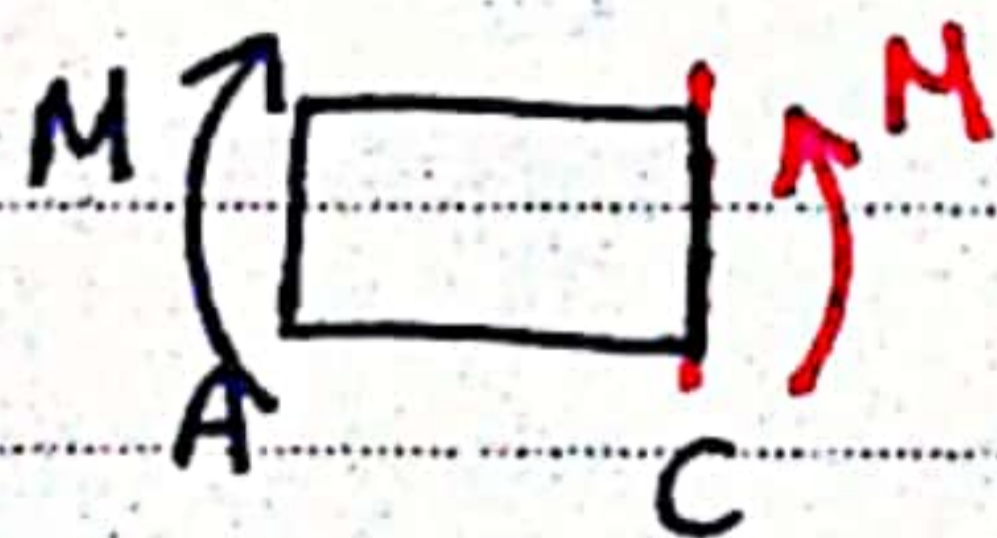
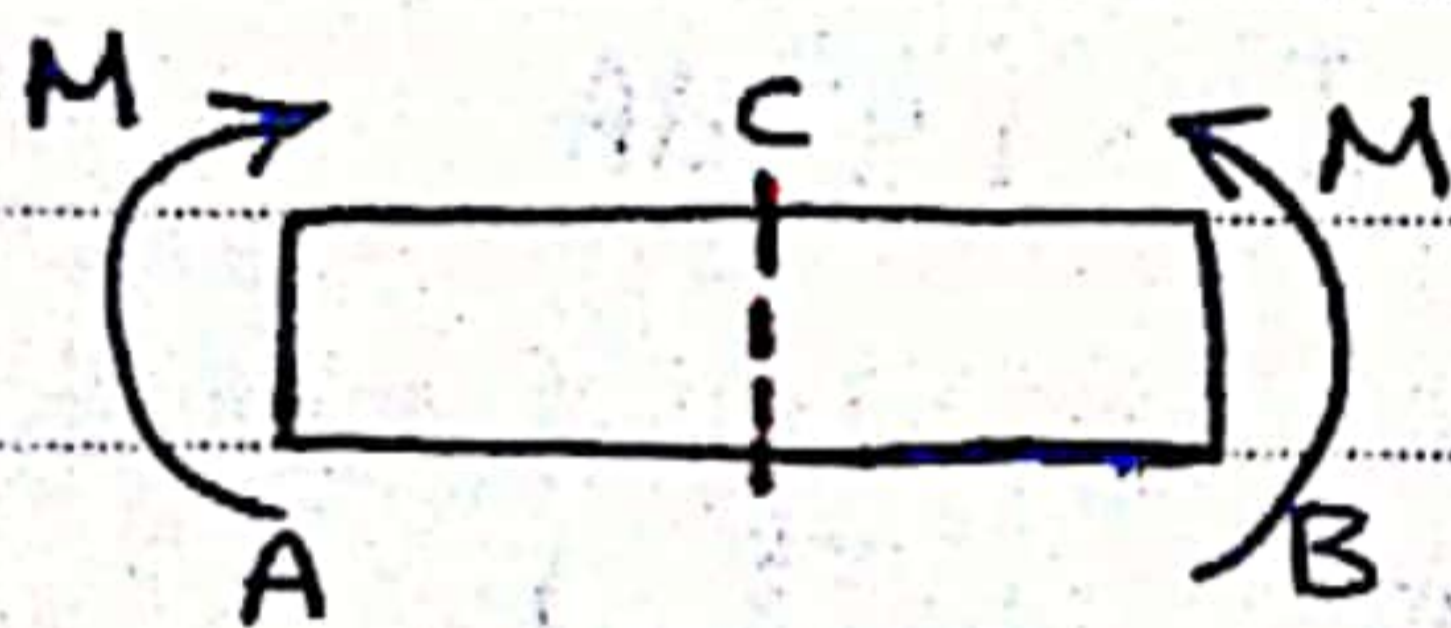


القوى التي
 فابتكون
 على الـ
 member
 axis

P' equal & opposite to P
 M of magnitude $M = Px$

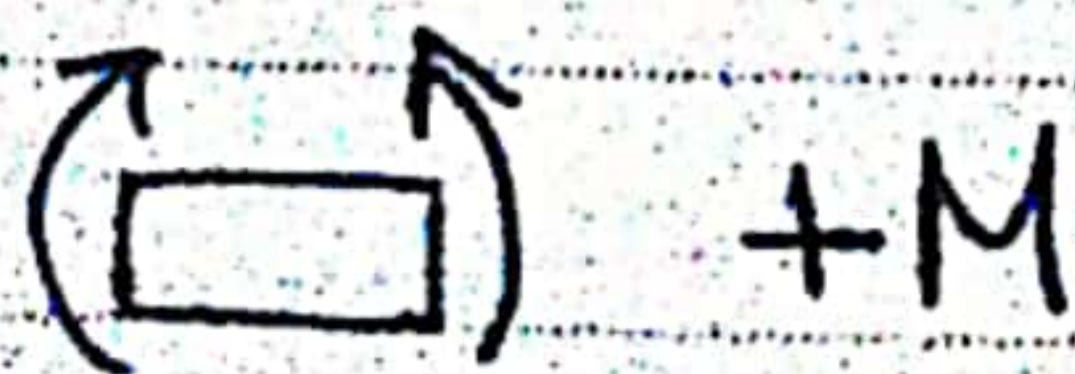


4.2 Symmetric Member in Pure Bending



the internal forces in any cross section of a symmetric member in pure bending are equivalent to a couple.

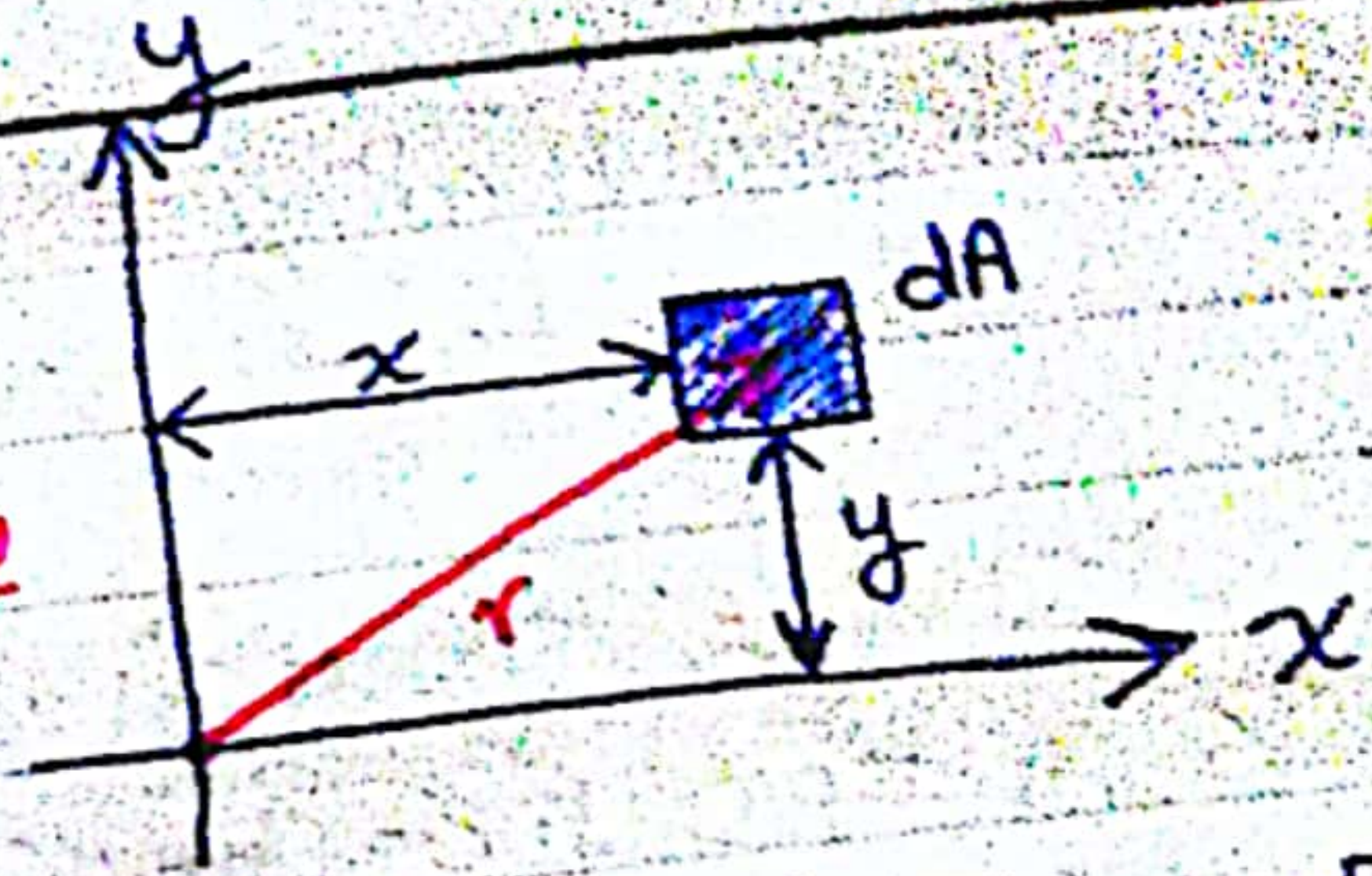
The moment M of that couple is referred to as \Rightarrow **bending moment**



$$I_x = \int y^2 \cdot dA$$

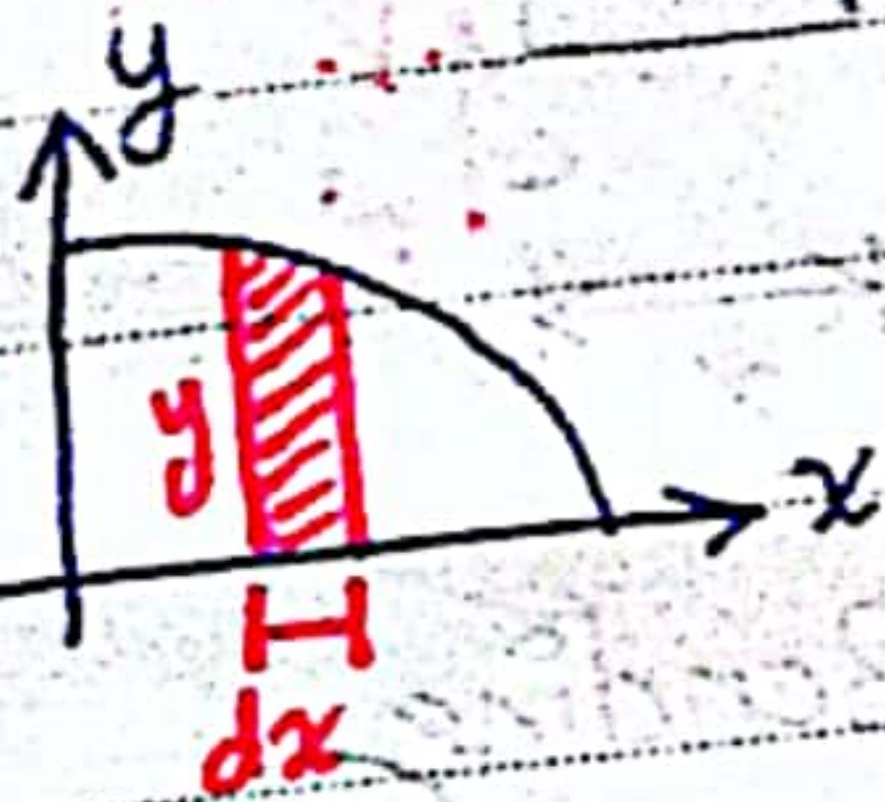
$$I_y = \int x^2 \cdot dA$$

Moment of inertia



[Polar moment of inertia] $J = \int r^2 \cdot dA = \int (x^2 + y^2) dA = I_x + I_y \quad [m^4]$

[Remarque]

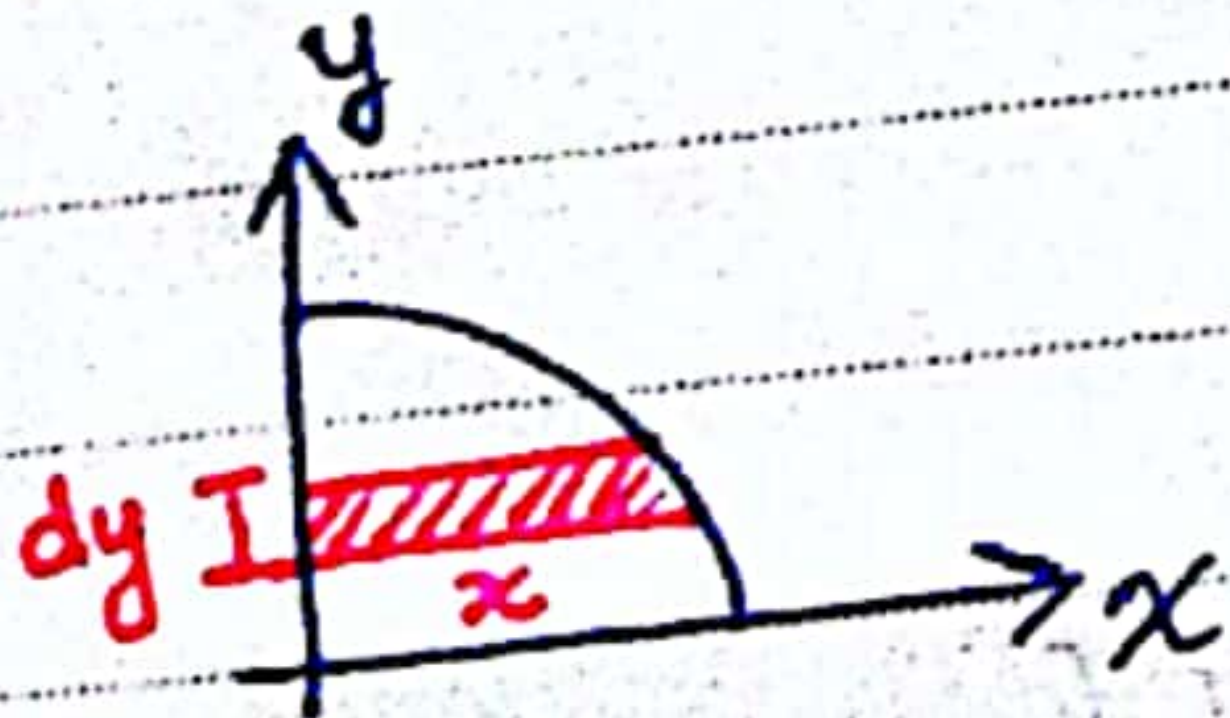


$y = ax + B$, $\omega_{23} \text{ } \omega_{30}$

$$\int dA = \int y \cdot dx$$

- $I_y = \int x^2 \cdot dA$

- $I_x = \int y^2 \cdot dA$



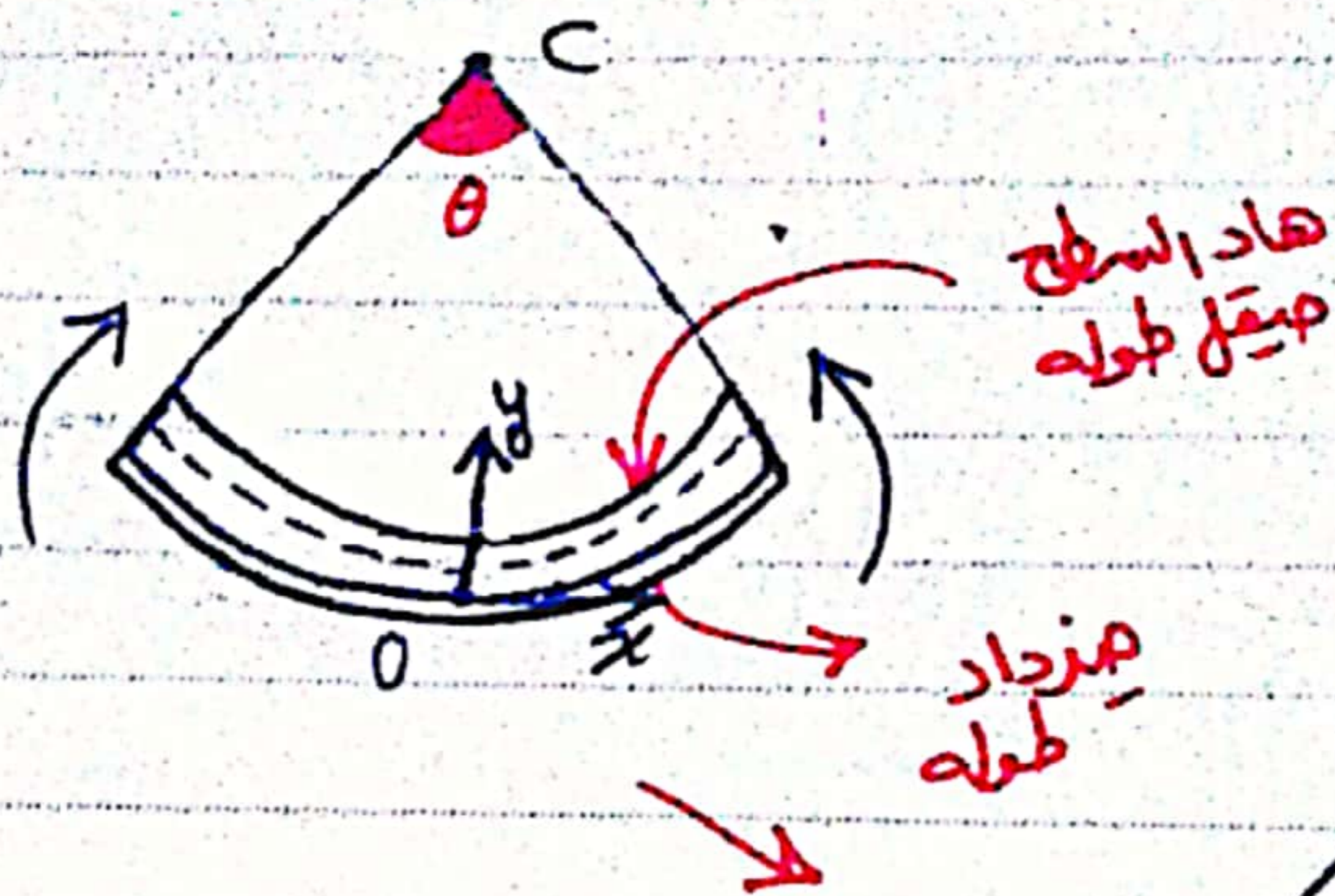
$x = ay + B$, $\omega_{30} \text{ } \omega_{23}$

$$\int dA = \int x \cdot dy$$

- $I_x = \int y^2 \cdot dA$

- $I_y = \int x^2 \cdot dA$

4.3 | Deformations in a Symmetric Member in Pure Bending



$$L = \rho\theta$$

$$L' = (\rho - y)\theta$$

$$\delta = L' - L \rightarrow \delta = (\rho - y)\theta - \rho\theta$$

$$\delta = -y\theta$$

$$\epsilon_x = \frac{\delta}{L} = \frac{-y\theta}{\rho\theta}$$

$$\therefore \left[\epsilon_x = -\frac{y}{\rho} \right]$$

لأنه يتغير مع $+M$

CONCAVE UP...

∴ the longitudinal normal strain ϵ_x varies linearly with the distance y from the neutral surface...

4.4 | Stresses and deformations in the elastic range

Assuming the material to be homogenous, and denoting by E its modulus of elasticity, we have in the longitudinal x direction:

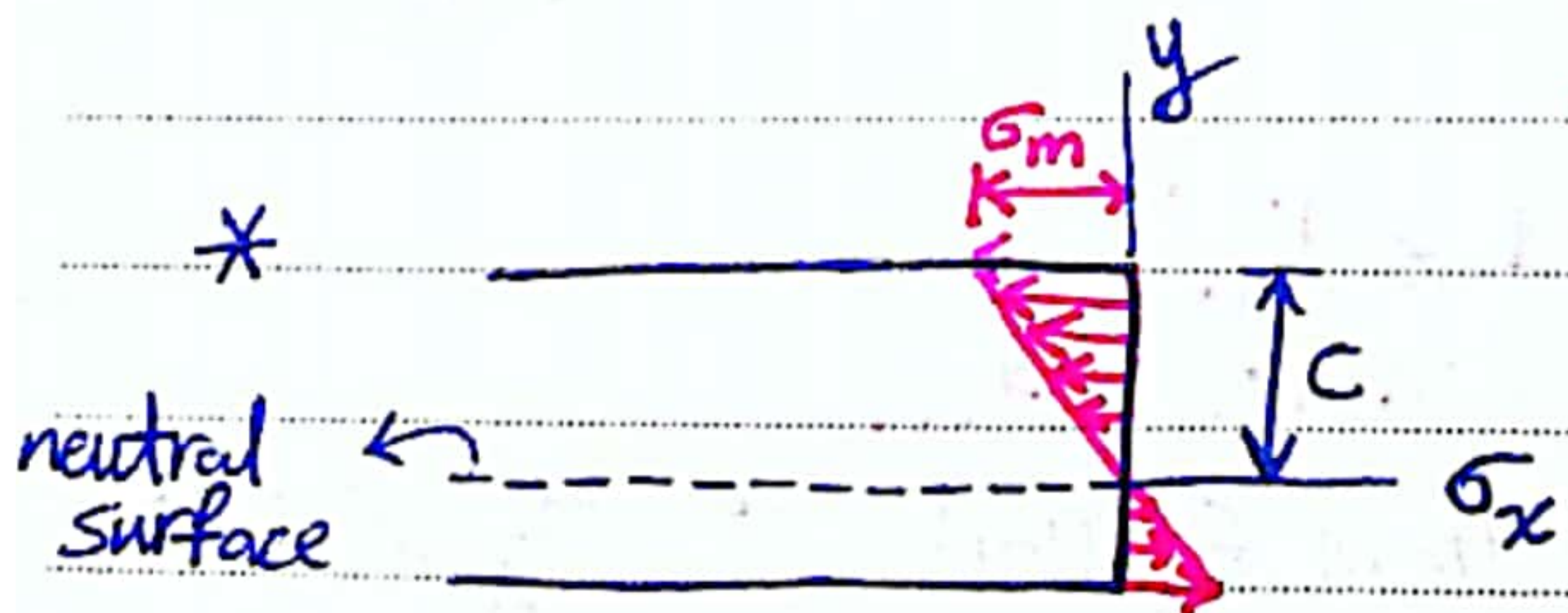
$$(\sigma_x = E \epsilon_x)$$

$$\Rightarrow \left[\epsilon_x = \frac{-y}{c} \epsilon_m \right] * E$$

$$\sigma_x = \frac{-y}{c} \sigma_m$$

→ the max absolute value of the stress

∴ This result shows that, in the elastic range, the normal stress varies linearly with the distance from the neutral surface.



⇒ Bending Stresses

$$\int \sigma_x dA = \int \frac{-y}{c} \sigma_m \cdot dA = 0$$

shows that the first moment of the cross section about its neutral axis must be zero.

∴ $\int y \cdot dA = 0$
 For a member subjected to pure bending, and as long as the stresses remain in the elastic range, the

neutral axis passes through the centroid of the section.

$$M = \int (-y \sigma_x) dA$$

$$M = \int \left(-y * \frac{-y}{c} \sigma_m\right) dA = \frac{\sigma_m}{c} \int y^2 dA$$

$$\therefore \sigma_m = \frac{M c}{I}$$

\Rightarrow in the case of pure bending the neutral axis passes through the centroid of the cross section.

$$\sigma_x = \frac{-M y}{I}$$

\Rightarrow the normal stress σ_x at any distance y from the neutral axis

(Elastic flexure formulas): $\sigma_x = \frac{M y}{I}$

and σ_x caused by the bending or "flexing" of the member \Rightarrow flexural stress

$\sigma_x < 0$, compressive above the neutral axis ($y > 0$)
when the bending moment is +ve
(M)

$\sigma_x > 0$, tensile when M is -ve

Elastic section modulus = $S = \frac{I}{c}$ → depends only upon the geometry of the cross section.

$$\therefore \left[\sigma_m = \frac{M}{S} \right]$$

The deformation of the member caused by M is measured by the curvature of the neutral surface

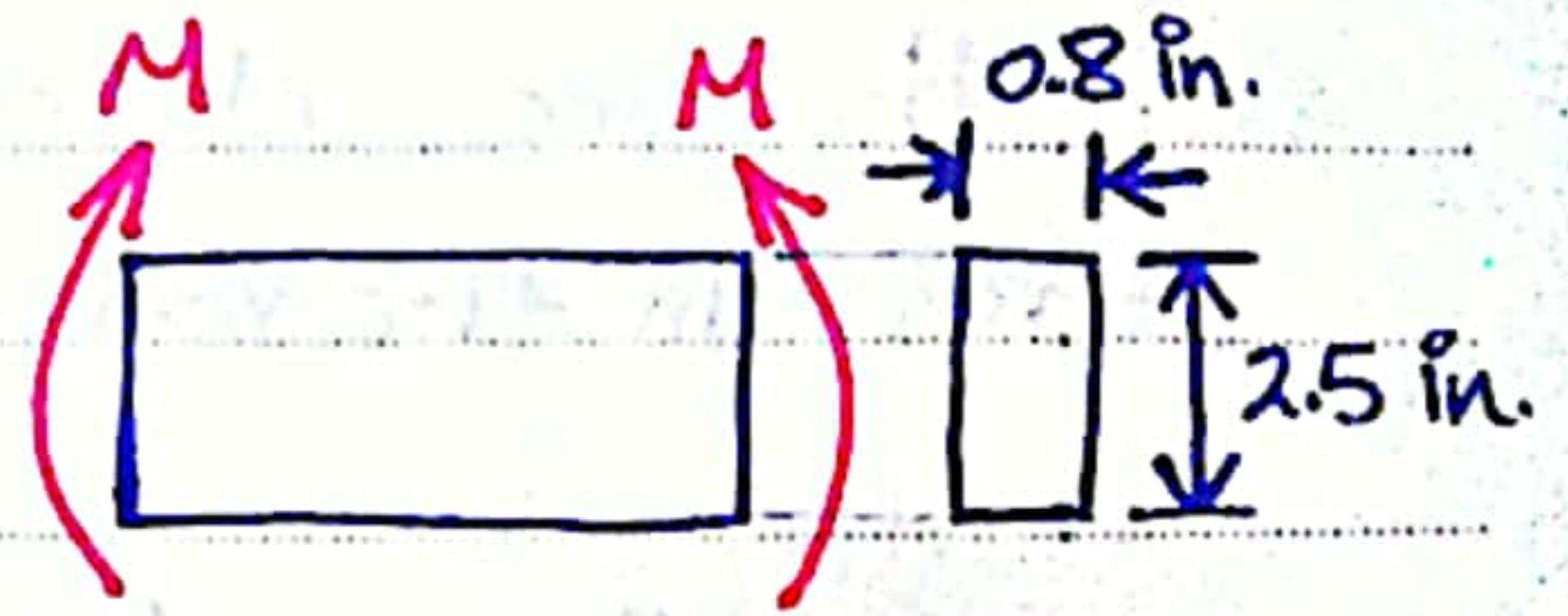
$$\left[\frac{1}{\rho} = \frac{\epsilon_m}{c} \right]$$

$$\left[\epsilon_m = \frac{\sigma_m}{E} \right] \leftarrow \text{elastic strain}$$

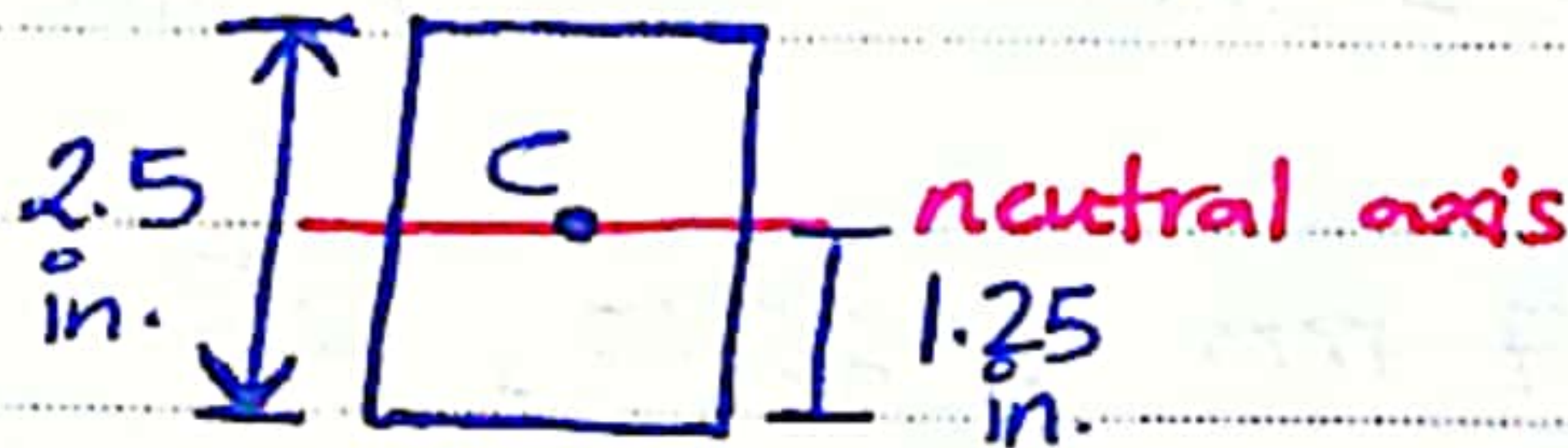
$$\therefore \frac{1}{\rho} = \frac{\sigma_m}{EC} = \frac{1}{EC} \left(\frac{Mc}{I} \right)$$

$$\left[\frac{1}{\rho} = \frac{M}{EI} \right]$$

Ex(4.01) A Steel bar of 0.8×2.5 in. rectangular cross sec. is subjected to two equal and opposite couples acting in the vertical plane of symmetry of the bar. Determine the value of the bending moment M that causes the bar to yield. Assume $\sigma_y = 36$ ksi



cross sec. \perp إلى محور التواء (neutral axis) \perp



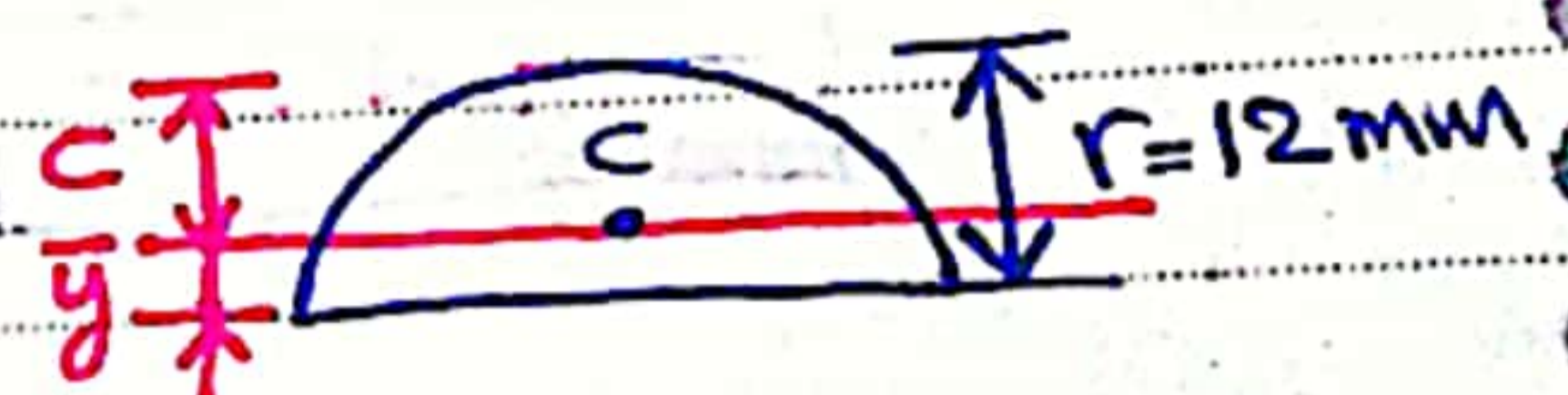
$$I = \frac{bh^3}{12} = \frac{(0.8)(2.5)^3}{12} = 1.042 \text{ [in}^4\text{]}$$

the centroidal moment of inertia of the rec. cross sec.

$$M = \frac{I}{c} \sigma_m = \frac{1.042 \times 36}{1.25} = 30 \text{ [kip.in]}$$

Example (4.02)

An aluminum rod with a semicircular cross section of radius $r = 12 \text{ mm}$ is bent into the shape of a circular arc of mean radius $\rho = 2.5 \text{ m}$. Knowing that the flat face of the rod is turned toward the center of curvature of the arc, determine the max. tensile and compressive stress in the rod. Use $E = 70 \text{ GPa}$

$$\rightarrow \bar{y} = \frac{4r}{3\pi} = \frac{4(12)}{3\pi} = 5.093 \text{ mm}$$


$$c = r - \bar{y} = 12 - 5.093 = 6.907 \text{ mm}$$

$$\epsilon_m = \frac{c}{\rho} = \frac{6.907 \text{ mm}}{2.5 \text{ m}} = 2.763 \times 10^{-3}$$

$$\sigma_m = E \epsilon_m = 70 \times 10^9 \times 2.763 \times 10^{-3} = 193.4 \text{ MPa}$$

since this side of the rod faces away from the center of curvature, the stress obtained is a tensile stress.

the max. comp stress occurs on the flat side of the rod

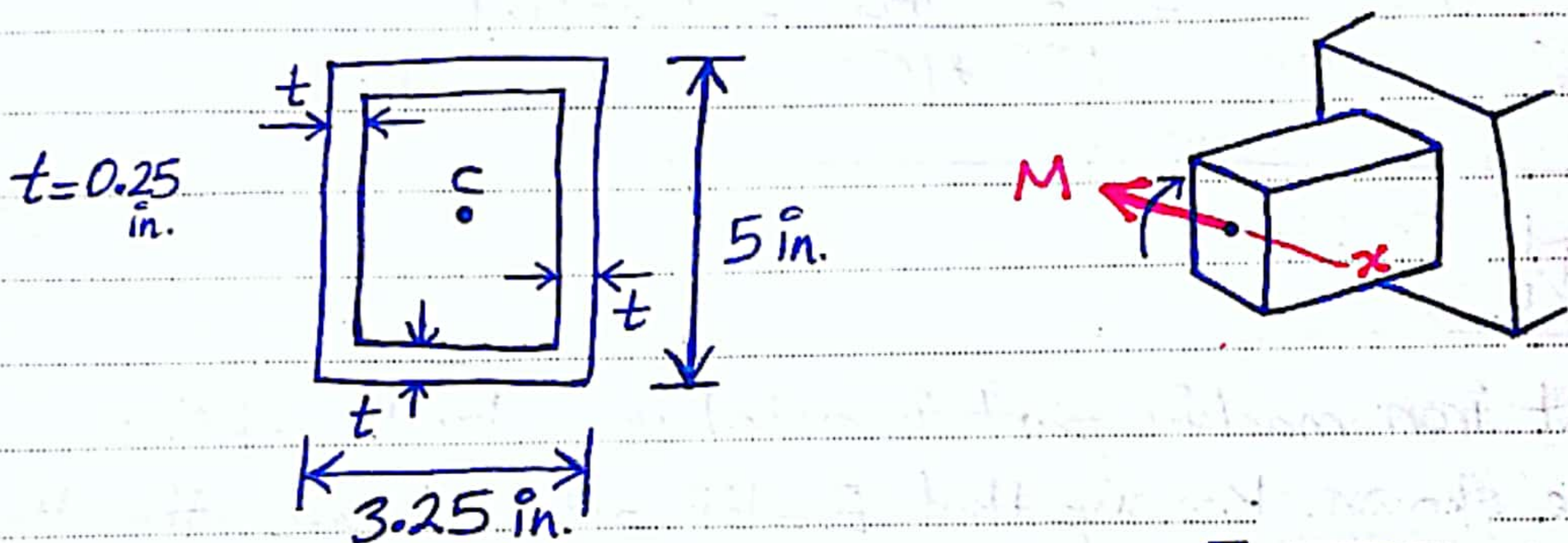
$$\begin{aligned} \sigma_{\text{comp}} &= -\frac{\bar{y}}{c} \sigma_m = -\frac{5.093}{6.907} (193.4 \times 10^6) \text{ Pa} \\ &= -142.6 \text{ MPa} \end{aligned}$$

Sample Problem (4.1)

The rectangular tube shown is extruded from an aluminum alloy for which $\sigma_y = 40 \text{ ksi}$, $\sigma_u = 60 \text{ ksi}$, and $E = 10.6 \times 10^6 \text{ psi}$.

Neglecting the fillets effect, determine

- the bending moment M for which the factor of safety will be 3?
- the corresponding radius of curvature of the tube?



→ the cross sec. area of the tube is

$$I = \frac{3.25(5)^3}{12} - \frac{2.75(4.5)^3}{12}$$

The diagram shows two rectangles. The outer rectangle has a height of 5 in. and a width of 3.25 in. The inner rectangle has a height of 4.5 in. and a width of 2.75 in. Dashed lines indicate the subtraction of the inner area from the outer area.

$$= 12.97 \text{ in}^4$$

$$\sigma_{\text{allowable}} = \frac{\sigma_{\text{ultimate}}}{\text{factor of safety (Fs)}} = \frac{60 \text{ ksi}}{3} = 20 \text{ ksi} < \sigma_y$$

elastic deformation of tube

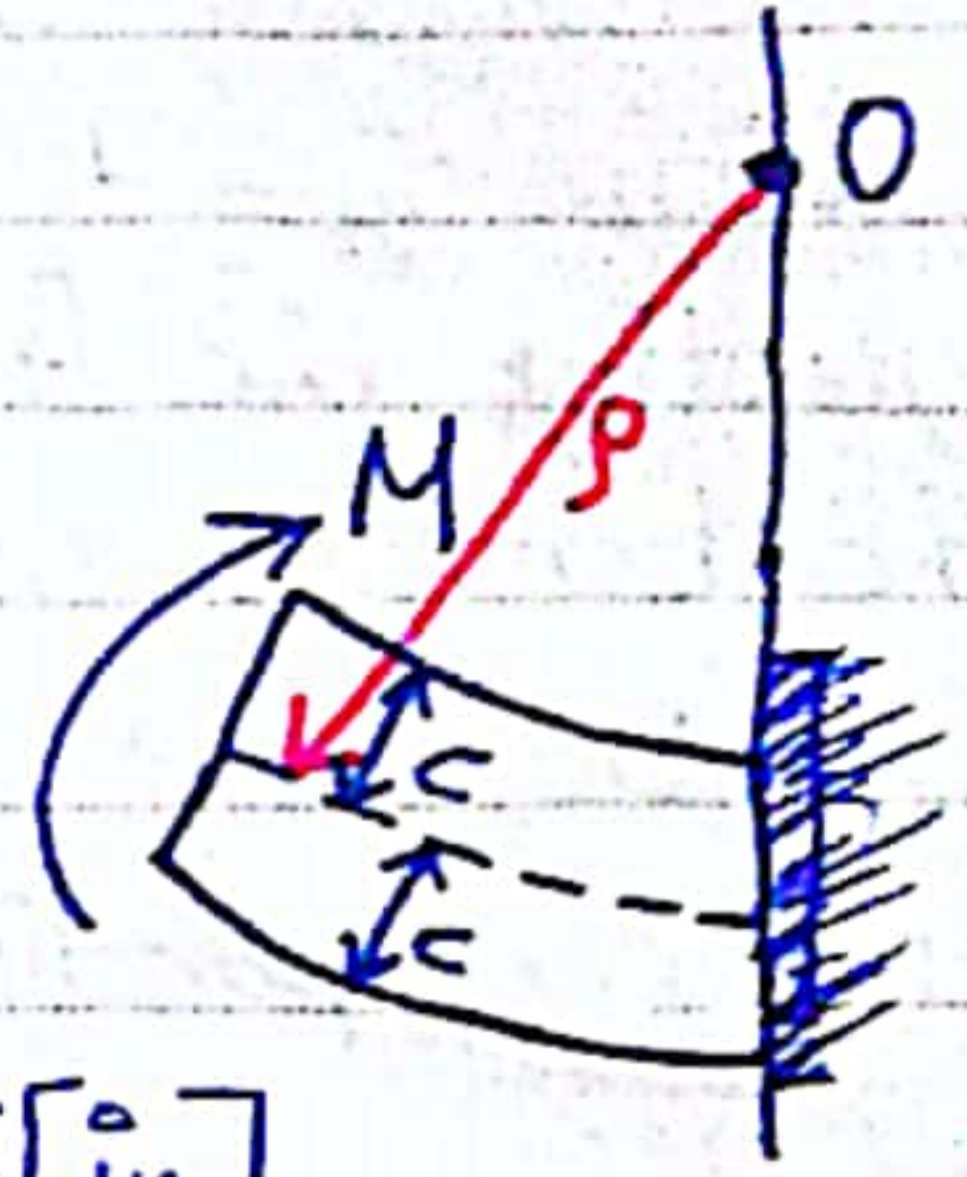
$$a. M = \frac{I \sigma_{\text{allow}}}{c} = \frac{12.97 \times 20 \text{ ksi}}{5 \times \frac{1}{2}} = 103.8 \text{ kip}\cdot\text{in}$$

$$b. \frac{1}{\rho} = \frac{M}{EI} = \frac{103.8 \times 10^3 \text{ [Ib}\cdot\text{in}]}{(10.6 \times 10^6 \text{ psi})(12.97)} = 0.755 \times 10^{-3} \text{ in}^{-1}$$

$$\rho = 1325 \text{ in.}$$

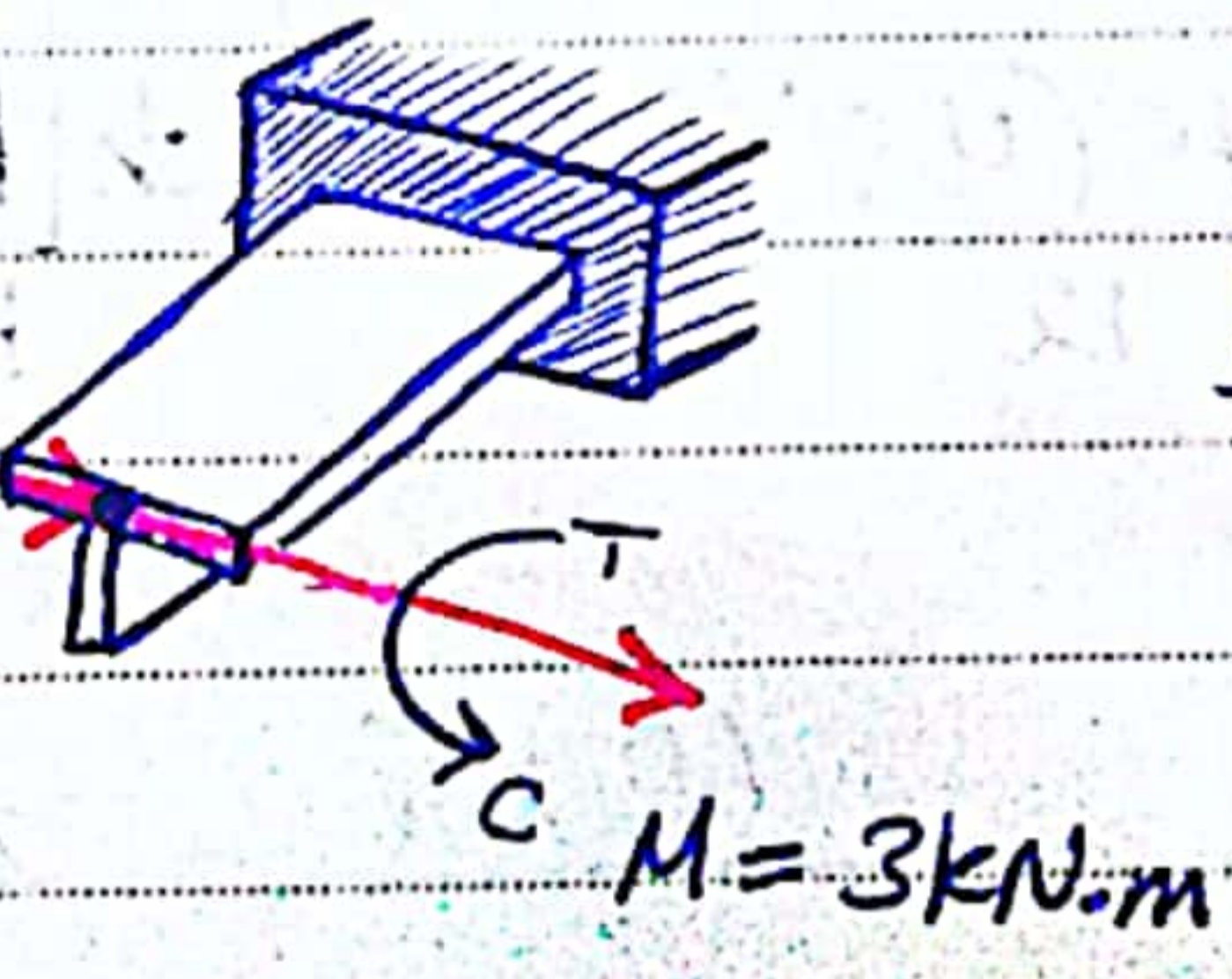
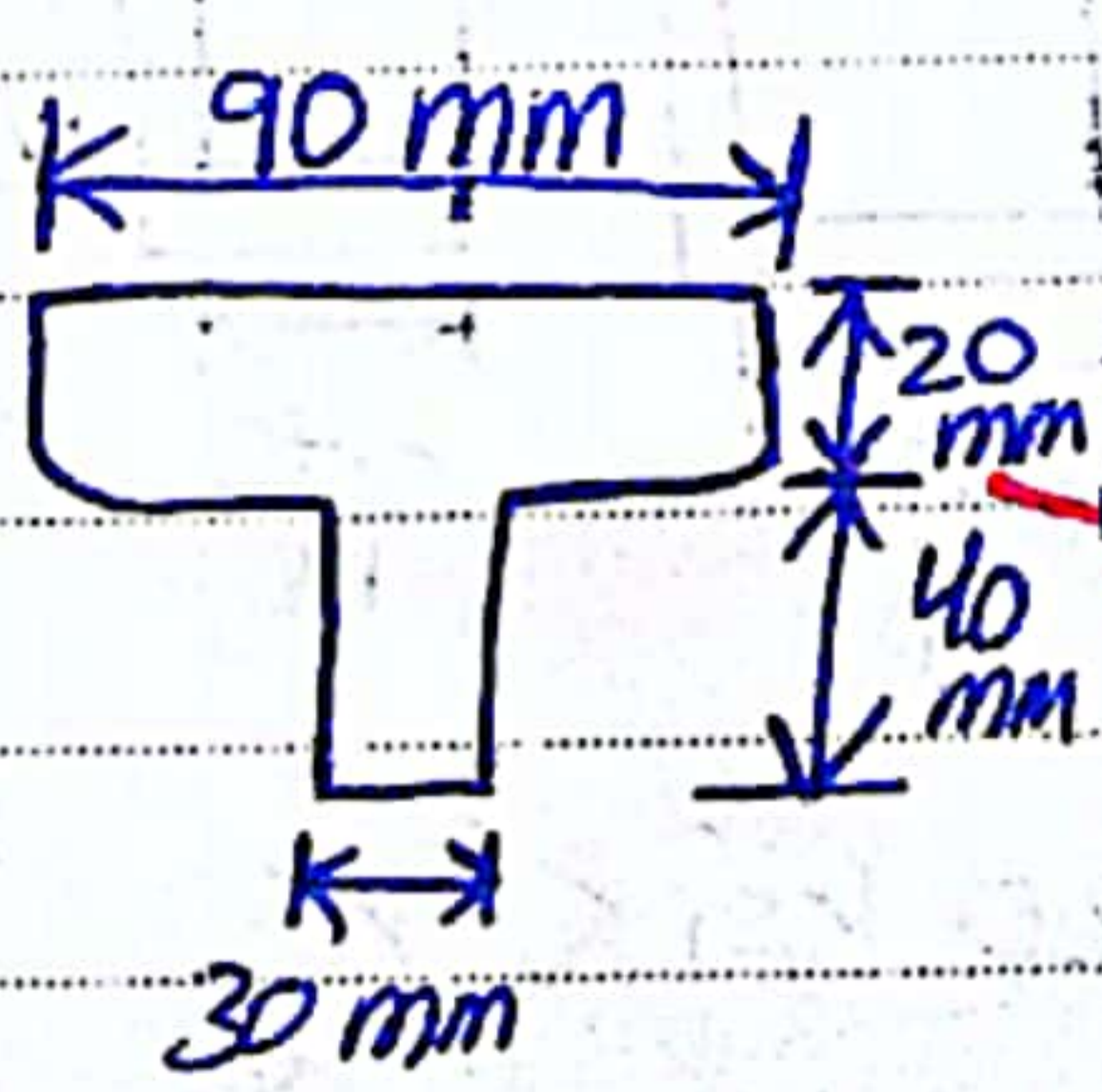
or
$$E_m = \frac{\sigma_{allow}}{E} = \frac{20 \text{ ksi}}{10.6 \times 10^6 \text{ psi}} = 1.887 \times 10^{-3}$$

$$\rho = \frac{c}{E_m} = \frac{2.5 \text{ [in.]}}{1.887 \times 10^{-3}} = 1325 \text{ [in.]}$$



Sample Problem (4.2)

A Cast iron machine part is acted upon by the 3 kN.m Couple shown. Knowing that $E = 165 \text{ GPa}$ and neglecting the effect of fillets, determine:



- the max. tensile and compressive stresses in the casting?
- the radius of curvature of the casting?

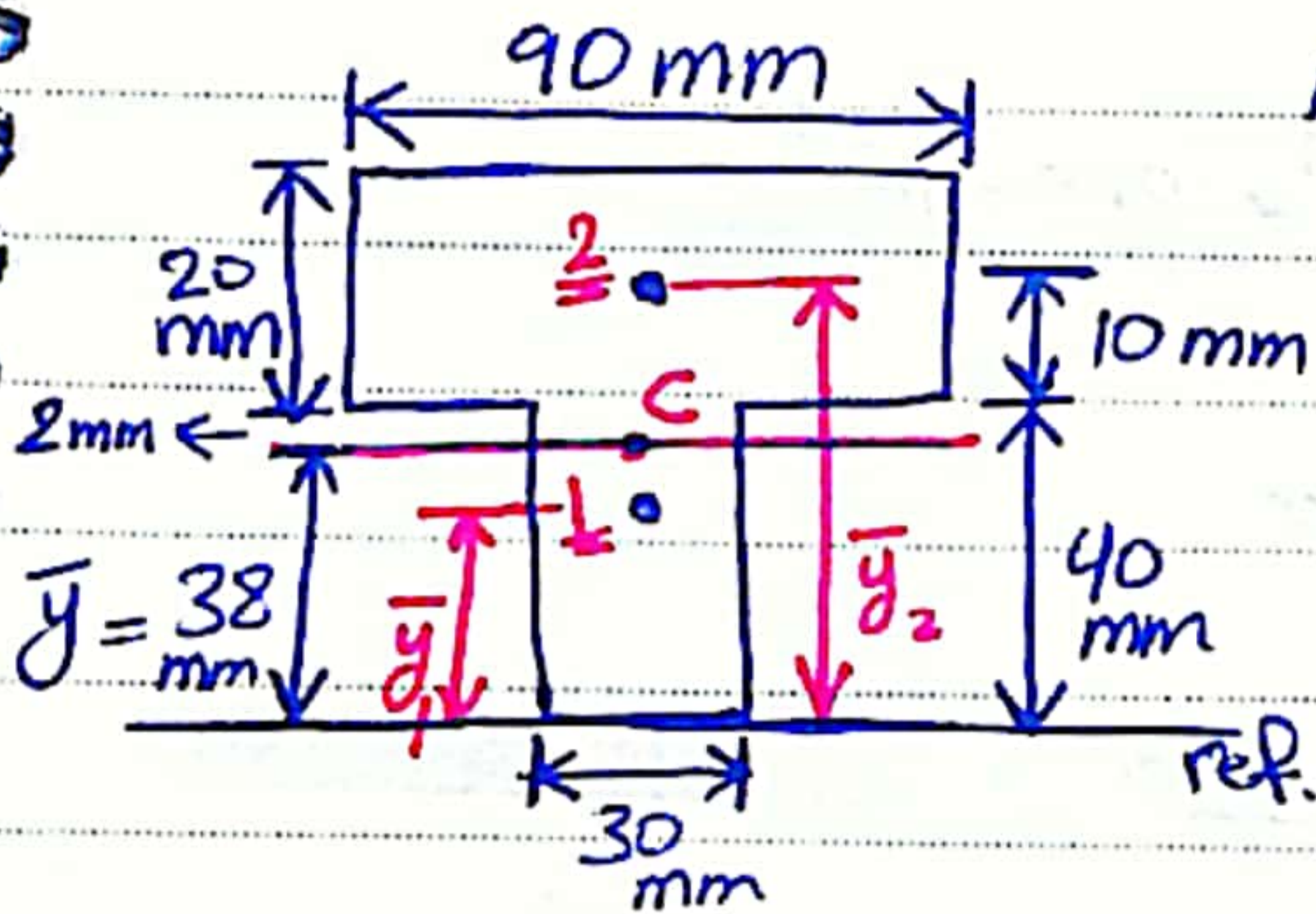
أول شيء نجد اتجاه الـ M (فيه نجد اتجاه الاسترود ومنطقة الـ tension والـ compression)

الليها مع رأس السهم ومركبة الأضلاع هي اتجاه الـ M والاسترود بيكون في اعتبار الليها

[الموصية (الأضلاع) بتدور هو اللي الاسترود (الليها)]



كان أطلع ك لازم أوجد I و y



$$\bar{y}_1 = 20 \text{ mm}$$

$$\bar{y}_2 = 50 \text{ mm}$$

$$\bar{y} = \frac{\sum A y_c}{\sum A} = \frac{(30 \times 40)(20) + (90 \times 20)(50)}{(30 \times 40) + (90 \times 20)} = 38 \text{ mm}$$

بإنا نستخدم
عن شكل فنظم

$$I = I_1 + I_2$$

$$I_1 = \frac{30(40)^3}{12} + (30 \times 40)(38 - 20)^2$$

$$I_2 = \frac{90(20)^3}{12} + (90 \times 20)(12)^2$$

$$\therefore I = 868 \times 10^{-9} \text{ m}^4$$

$$I_x = I_x + Ad^2$$

المسافة بين مركز كل شكل والاسترود للشكل كاملًا

من الشد في السطح،

$$\sigma_{\max} |_{\text{tension}} = \frac{M y}{I} = \frac{3000 (22 \times 10^{-3})}{868 \times 10^{-9}} = 76 \text{ MPa}$$

تكون في سطح الشد في الـ tension (في الأعلى للشد)

$$\sigma_{\max} |_{\text{comp}} = \frac{M y}{I} = \frac{-3000 (38 \times 10^{-3})}{868 \times 10^{-9}} = -131.3 \text{ MPa}$$

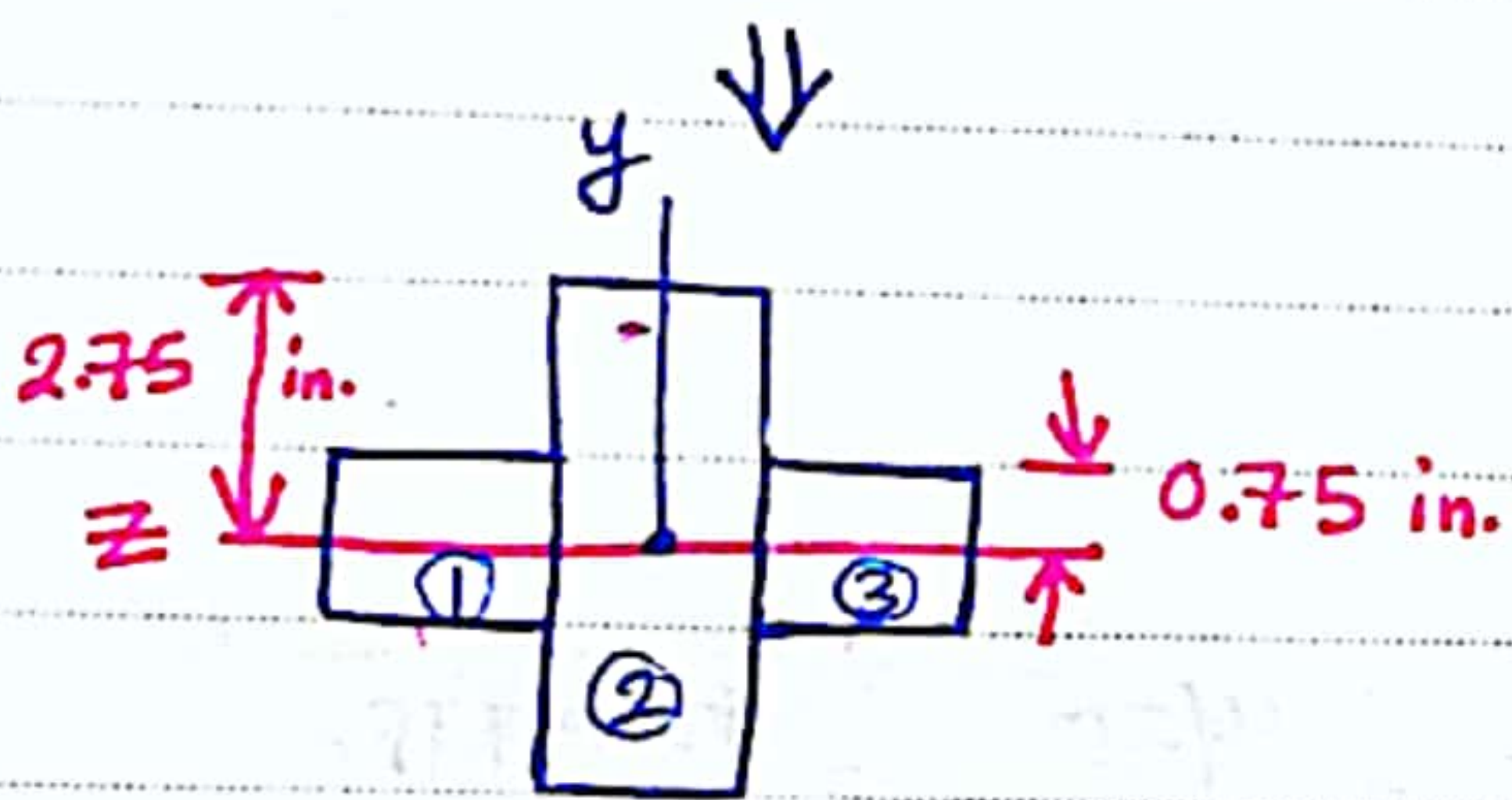
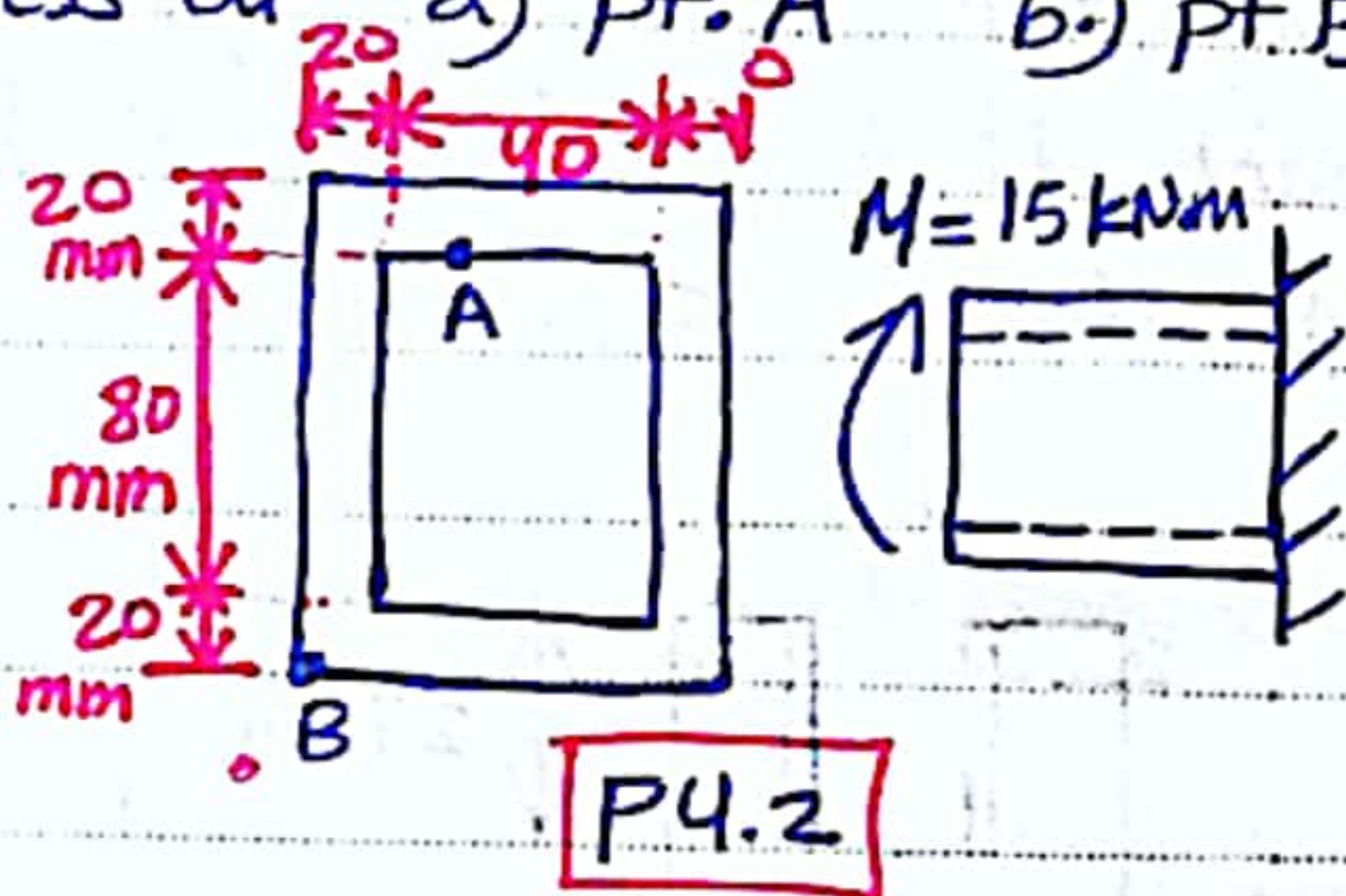
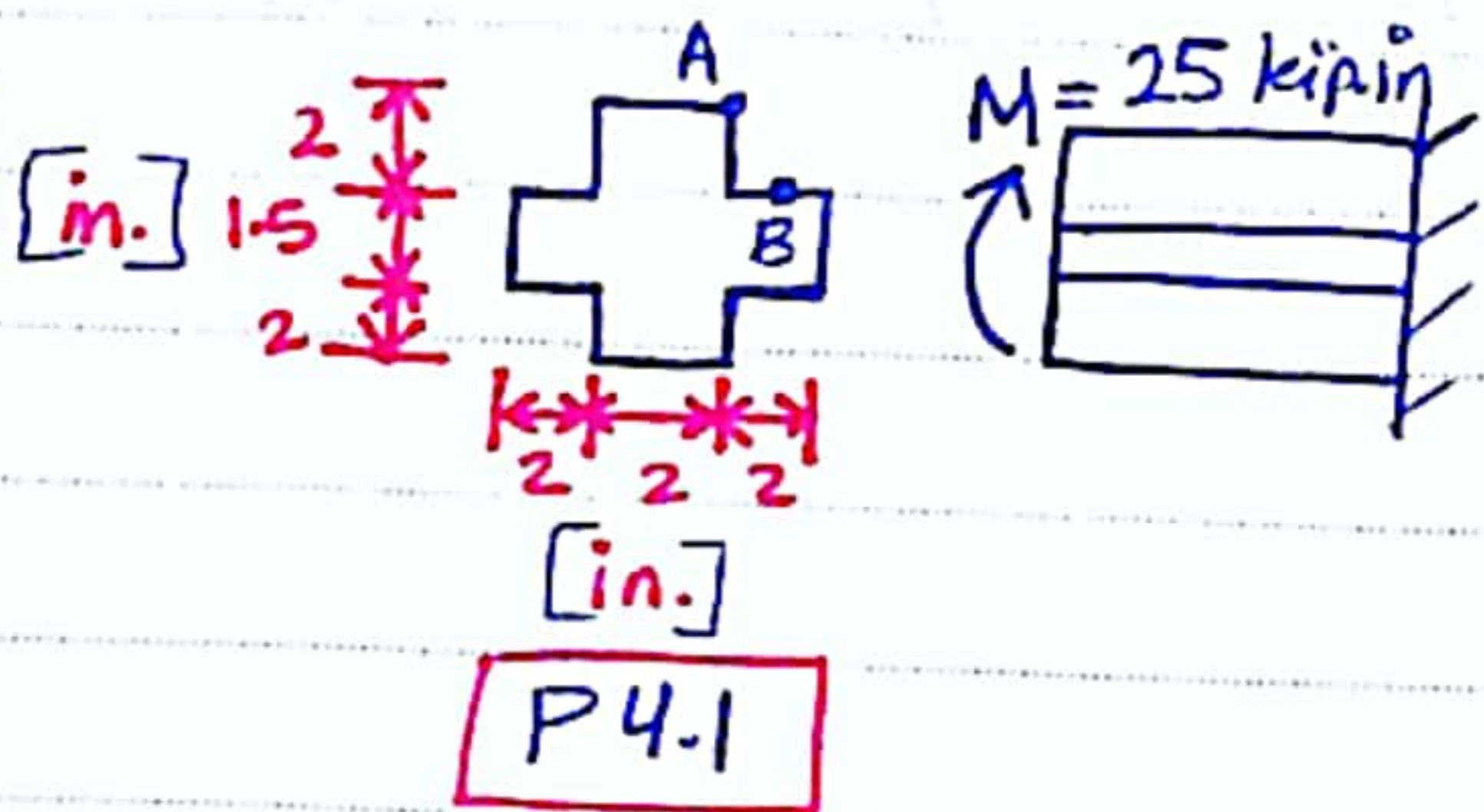
تكون في سطح الشد في الـ comp. (في الأسفل للشد)

$$b. \rho = \frac{E I}{M} = \frac{165 \times 10^9 \times 868 \times 10^{-9}}{3000} = 47.7 \text{ m}$$

Suggested Problems

Ch. 4

4.1 & 4.2) knowing that the couple shown acts in a vertical plane, determine the stress at a) pt. A b) pt. B



$$I = I_1 + I_2 + I_3$$

$$= \frac{2(1.5)^3}{12} + \frac{2(5.5)^3}{12} + \frac{2(1.5)^3}{12}$$

$$= 28.854 \text{ in}^4$$

a) $y_A = 2.75 \text{ in.} \rightarrow \sigma_A = \frac{-My_A}{I}$

$$= \frac{-25(2.75)}{28.854}$$

$$= -2.38 \text{ ksi}$$

b) $y_B = 0.75 \text{ in.} \rightarrow \sigma_B = \frac{-My_B}{I}$

$$= \frac{-(25)(0.75)}{28.854}$$

$$= -0.65 \text{ ksi}$$

For the section $I = I_1 - I_2$

$$I = \frac{80(120)^3}{12} - \frac{40(80)^3}{12} = 9.813 \times 10^6 \text{ mm}^4$$

$$I = 9.8133 \times 10^{-6} \text{ m}^4$$

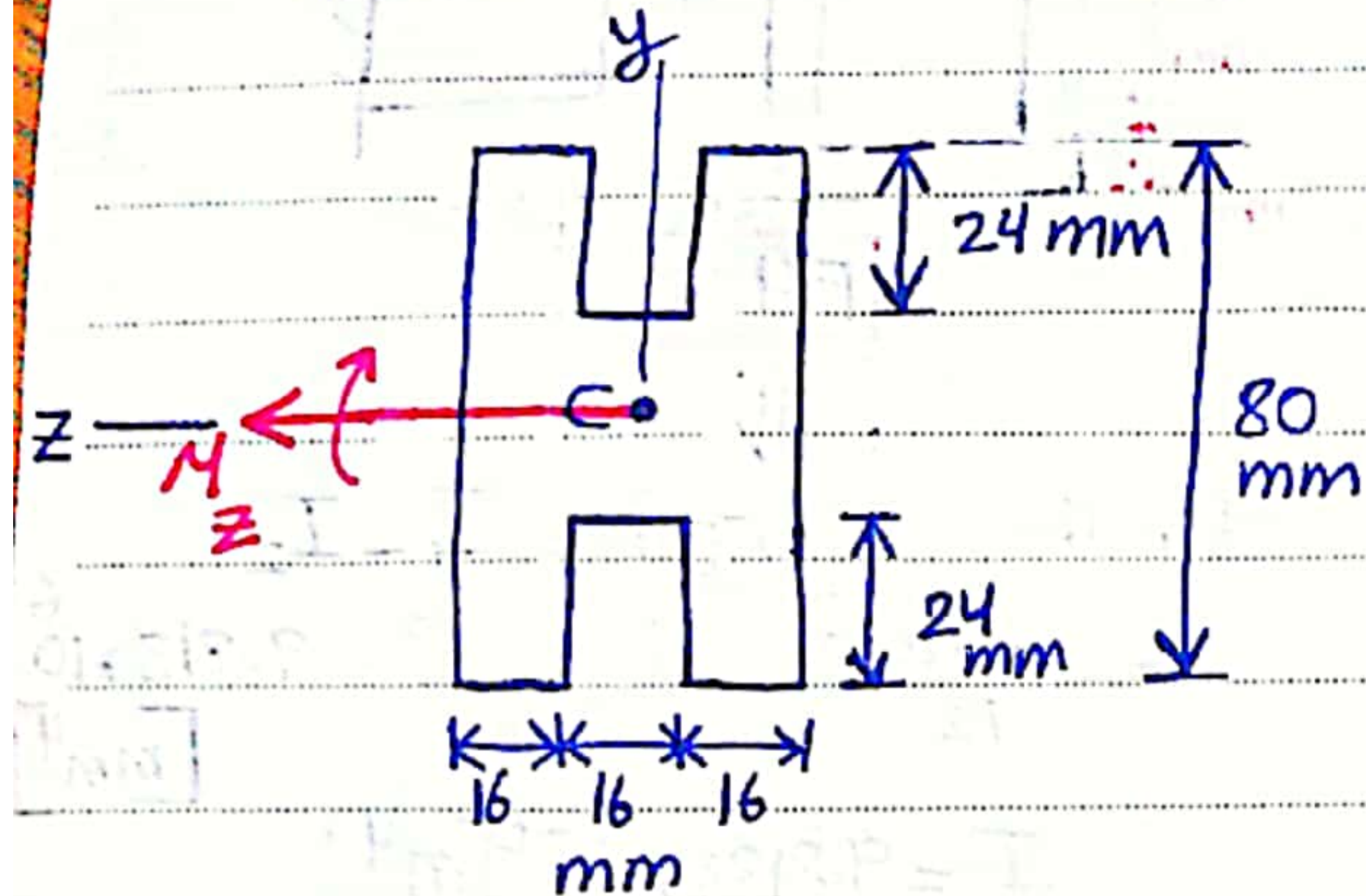
a) $y_A = 0.04 \text{ m} \rightarrow \sigma_A = \frac{-My_A}{I}$

$$\sigma_A = \frac{-(15 \times 10^3)(0.04)}{9.8133 \times 10^{-6}} = -61.6 \text{ MPa}$$

b) $y_B = -0.06 \text{ m} \rightarrow \sigma_B = \frac{-My_B}{I}$

$$\sigma_B = \frac{-(15 \times 10^3)(-0.06)}{9.8133 \times 10^{-6}} = 91.7 \text{ MPa}$$

4.5) A beam of the cross section shown is extruded from an aluminum alloy for which $\sigma_y = 250 \text{ MPa}$ and $\sigma_u = 450 \text{ MPa}$. Using a factor of safety of 3, determine the largest couple that can be applied to the beam when it's bent about the Z axis?

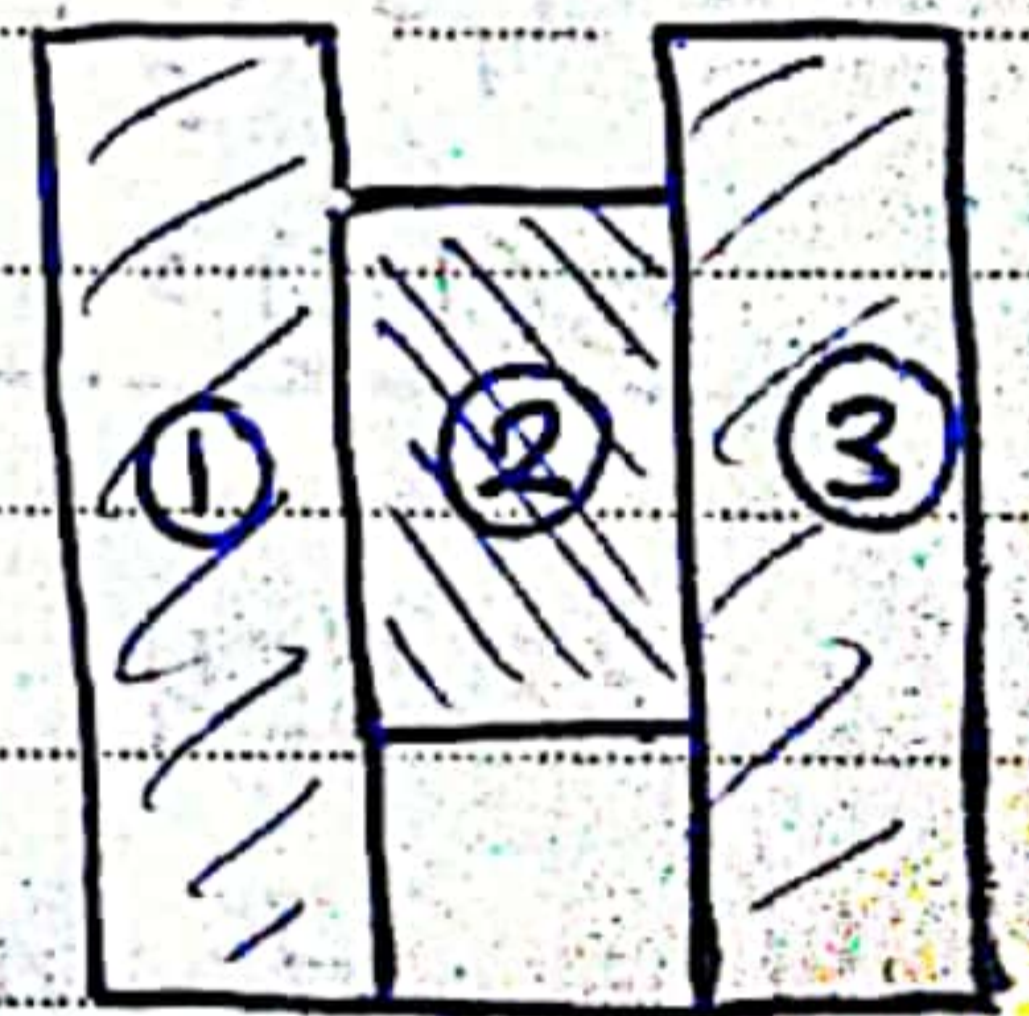


$$\text{allowable stress} \Rightarrow = \frac{\sigma_{ult}}{F.S} = \frac{450}{3} = 150 \text{ MPa}$$

Moment of inertia about Z axis:

$$I_1 = \frac{16(80)^3}{12} = 682.67 \times 10^3 [\text{mm}^4]$$

$$I_2 = \frac{16(32)^3}{12} = 43.69 \times 10^3 [\text{mm}^4]$$

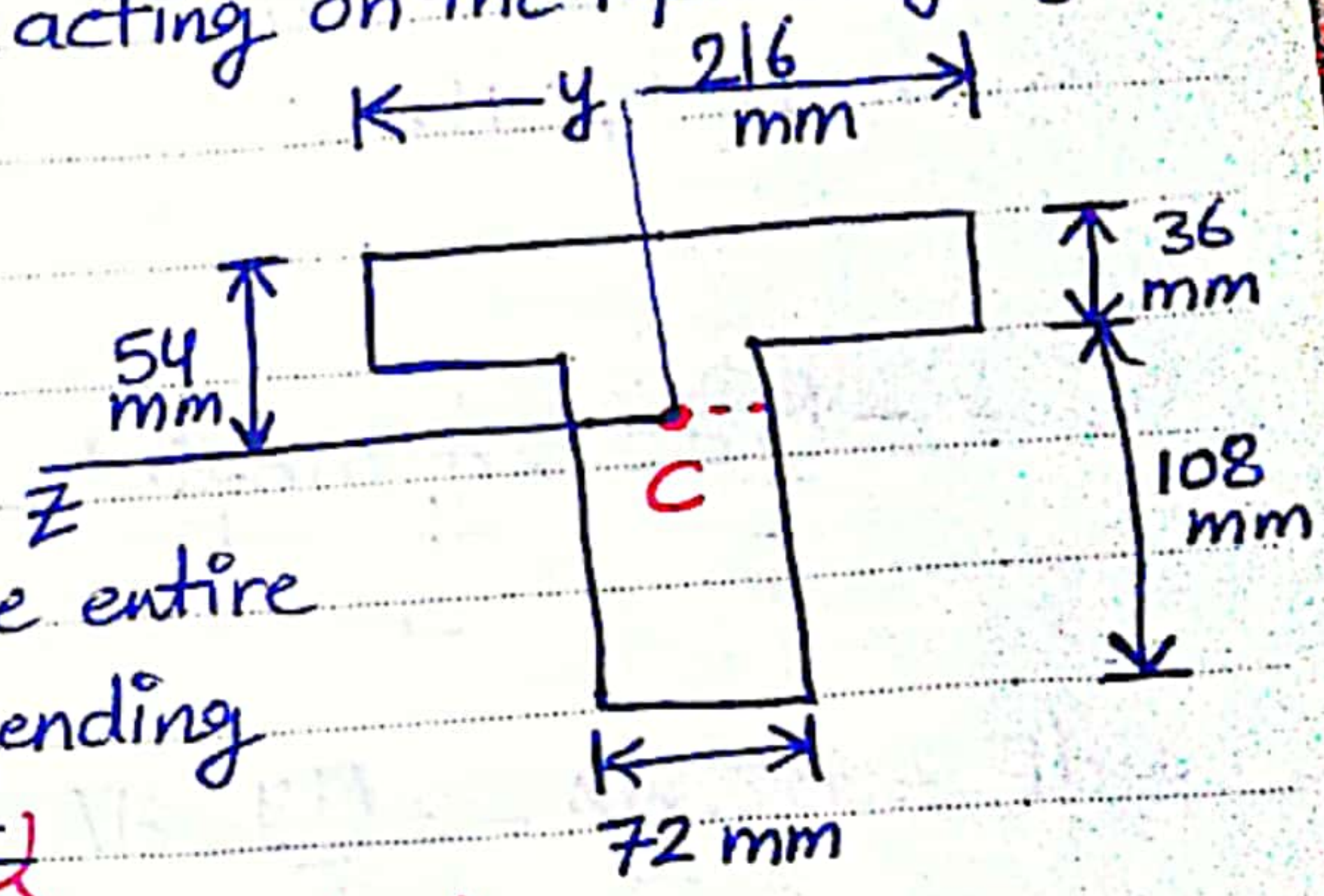


$$I_3 = I_1 = 682.67 \times 10^3 [\text{mm}^4] \Rightarrow I = I_1 + I_2 + I_3 = 1.409 \times 10^{-6} \text{ m}^4$$

$$\begin{aligned} * \sigma &= \frac{M C}{I} \Rightarrow M = \frac{150 \times 10^6 \times 1.409 \times 10^{-6} [\text{m}^4]}{\frac{1}{2}(80) \times 10^{-3} [\text{m}]} \\ &= 5.28 \times 10^3 [\text{Nm}] \end{aligned}$$

#

4.12) Knowing that a beam of the cross sec. shown is bent about a horizontal axis and that the bending moment is 6 kNm, determine the total force acting on the top flange?



the stress distribution over the entire cross section is given by the bending stress formula

$$\sigma_x = -\frac{My}{I}$$

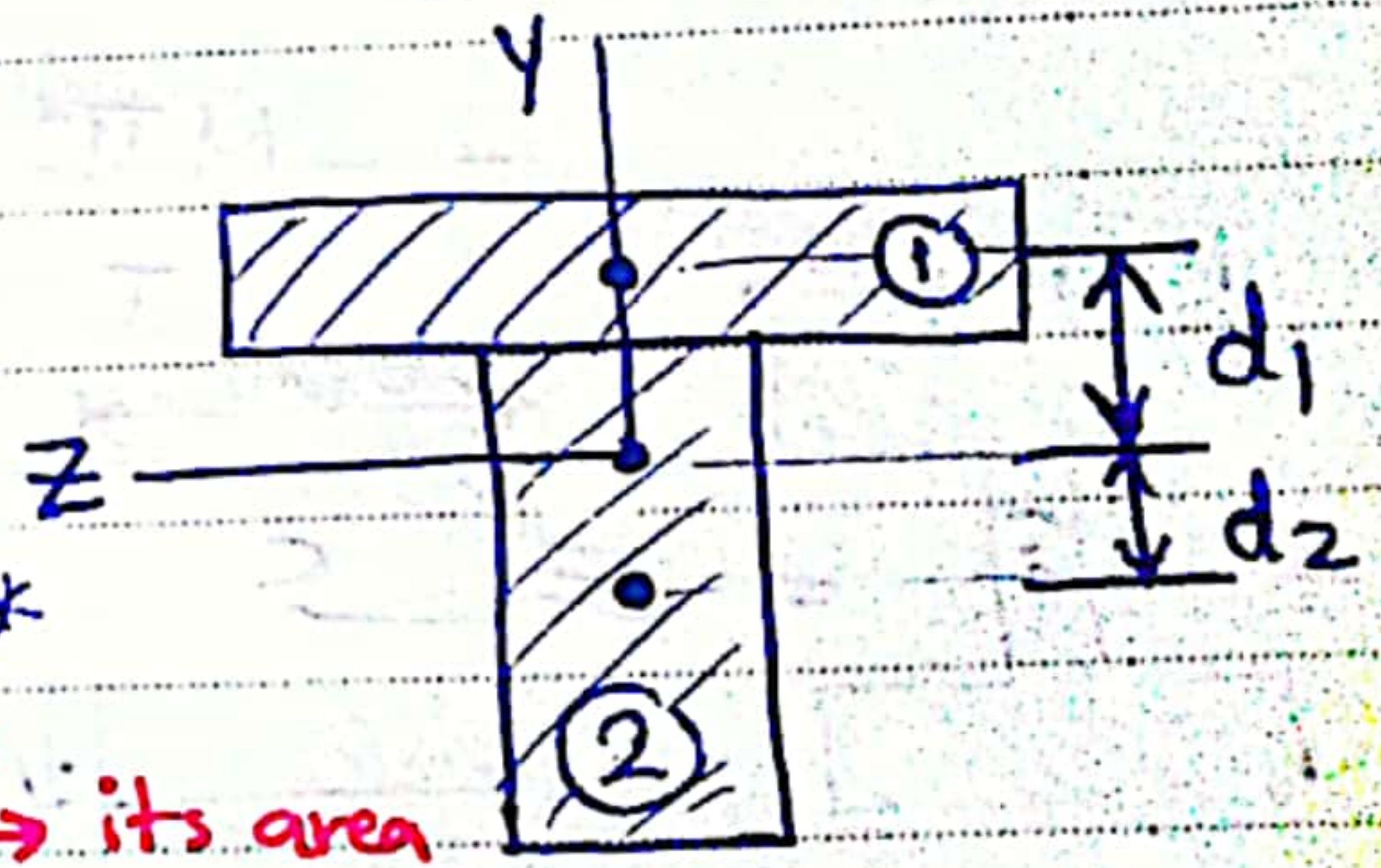
moment of inertia (pointing to I)
coordinate with its origin on the neutral axis (pointing to y)
dis (cross section) (pointing to y)

the force on the shaded area

$$dF = \sigma_x dA = -\frac{My}{I} dA$$

$$F = \int -\frac{My}{I} dA = -\frac{M}{I} \int y^* dA^*$$

its area (pointing to dA^*)
centroidal coordinate of the shaded portion (pointing to y^*)



$$d_1 = 54 - 18 = 36 \text{ mm}$$

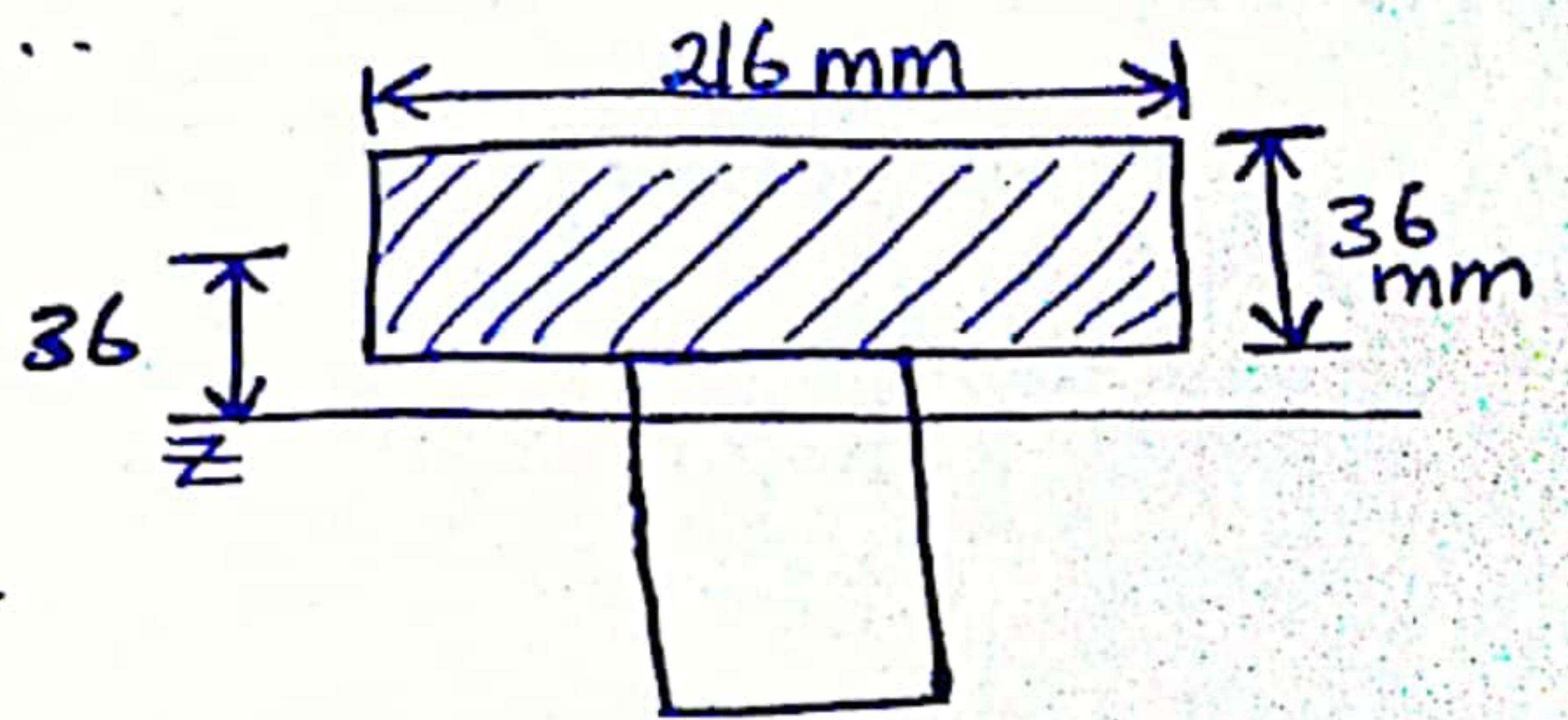
$$d_2 = 54 + 36 - 54 = 36 \text{ mm}$$

$$I_1 = \frac{216(36)^3}{12} + 216(36)(36)^2 = 10.9175 \times 10^6 \text{ mm}^4$$

$$I_2 = \frac{72(108)^3}{12} + 72(108)(36)^2 = 17.636 \times 10^6 \text{ mm}^4$$

$$I = I_1 + I_2 = 28.55 \times 10^6 \text{ mm}^4$$

for the shaded area...



$$A^* = 216(36) = 7776 \text{ mm}^2$$

$$\bar{y}^* = 36 \text{ mm}$$

$$A^* \bar{y}^* = 279.936 \times 10^{-6} \text{ m}^3$$

$$F = - \left| \frac{MA^* \bar{y}^*}{I} \right| = \frac{6 \times 10^3 \times 279.936 \times 10^{-6}}{28.5535 \times 10^{-6}}$$

$$F = 58.8 \text{ kN}$$

#

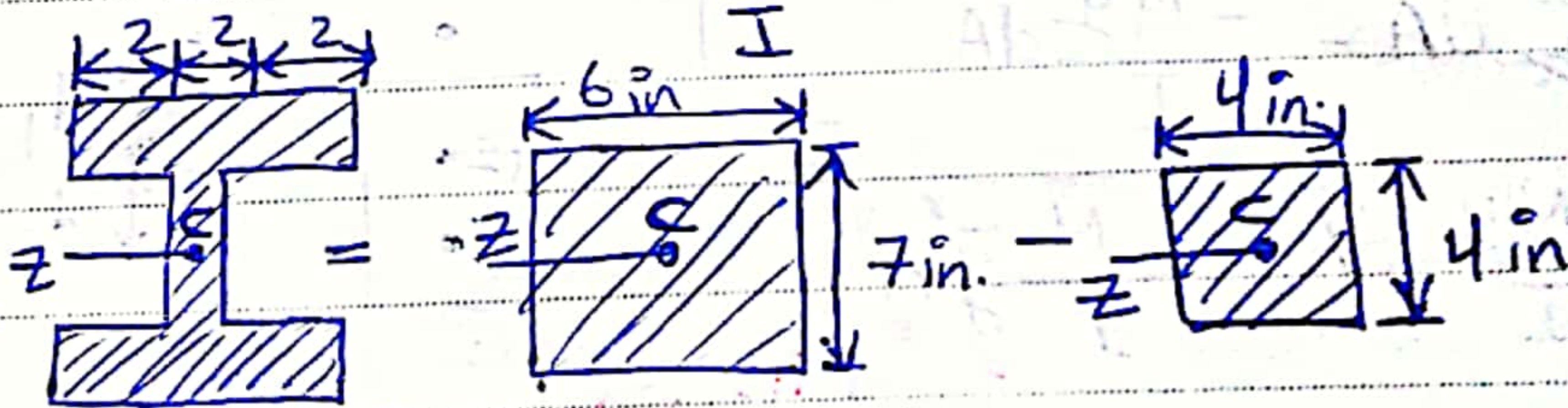
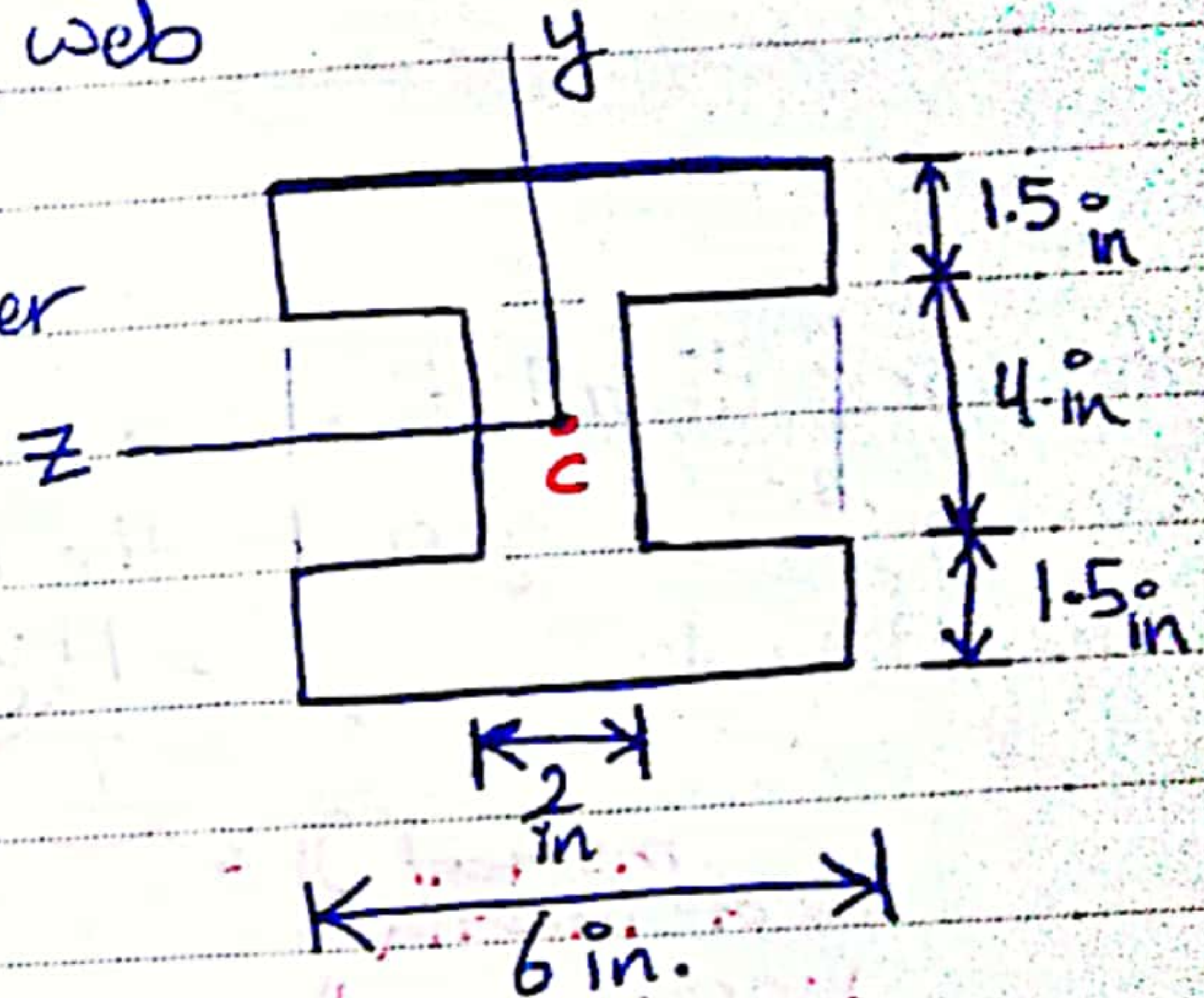
4.14) Knowing that a beam of the cross section shown is bent about a horizontal axis z that the bending moment is $50 \text{ kip}\cdot\text{in}$, determine the total force acting
 a) on the top flange b) on the shaded portion of the web

$$\sigma_x = \frac{-My}{I} \quad \text{distributed over the entire cross section}$$

$$dF = \sigma_x dA = \frac{-My}{I} dA$$

$$F = \int dF = \frac{-M}{I} \int y dA$$

$$= \frac{-M \bar{y}^* A^*}{I}$$



$$I = \frac{6(7^3)}{12} - \frac{4(4)^3}{12} = 150.17 \text{ [in}^4\text{]}$$

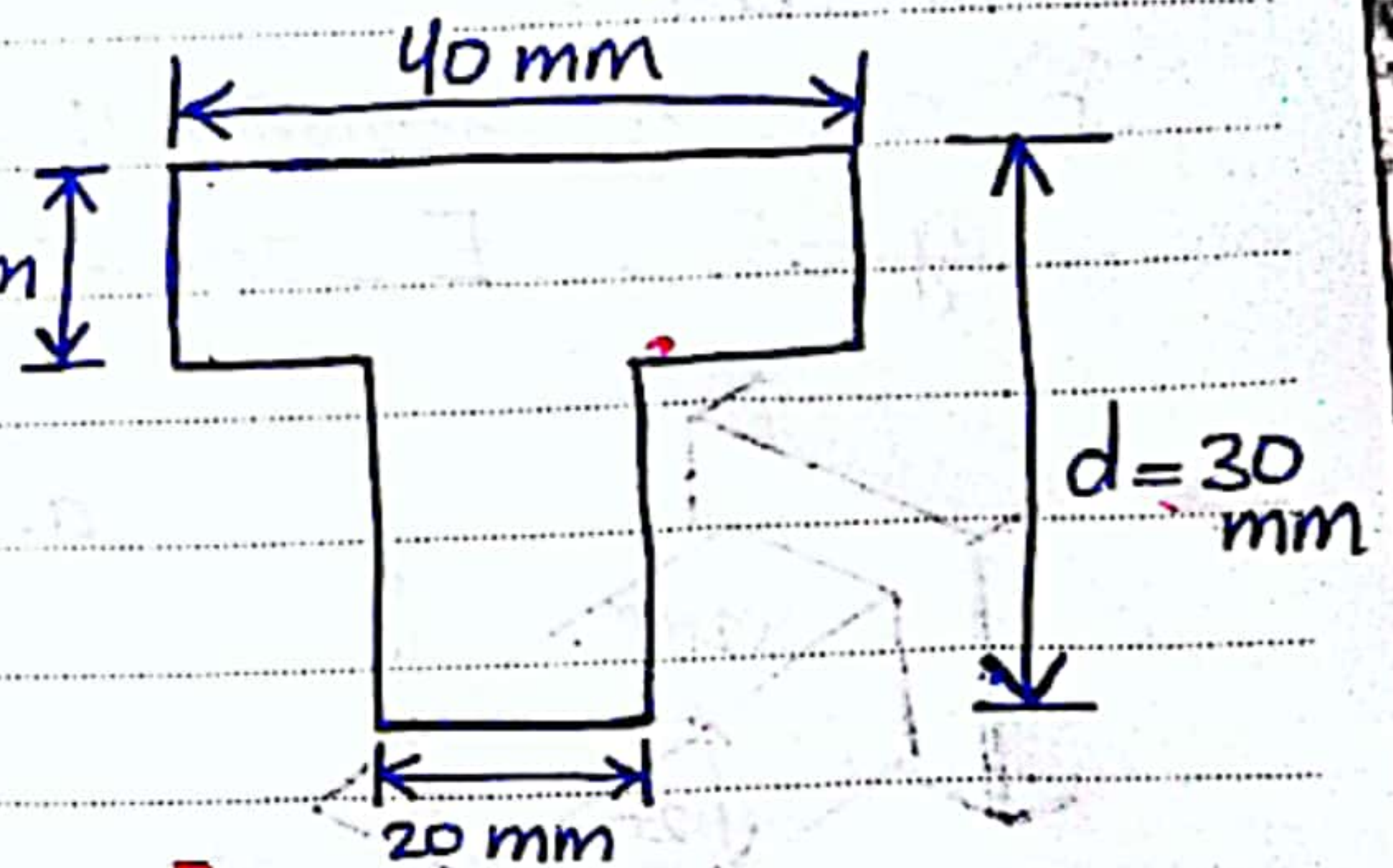
$$M = 50 \text{ kip}\cdot\text{in}$$

$$a) A^* = 6(1.5) = 9 \text{ in}^2, \quad \bar{y}^* = 2 + 0.75 = 2.75 \text{ in.}$$

$$F = \frac{50 * 9 * 2.75}{150.17} = 8.24 \text{ kips}$$

$$b) F = \frac{50 * (2 * 2) * (1)}{150.17} = 1.332 \text{ kips}$$

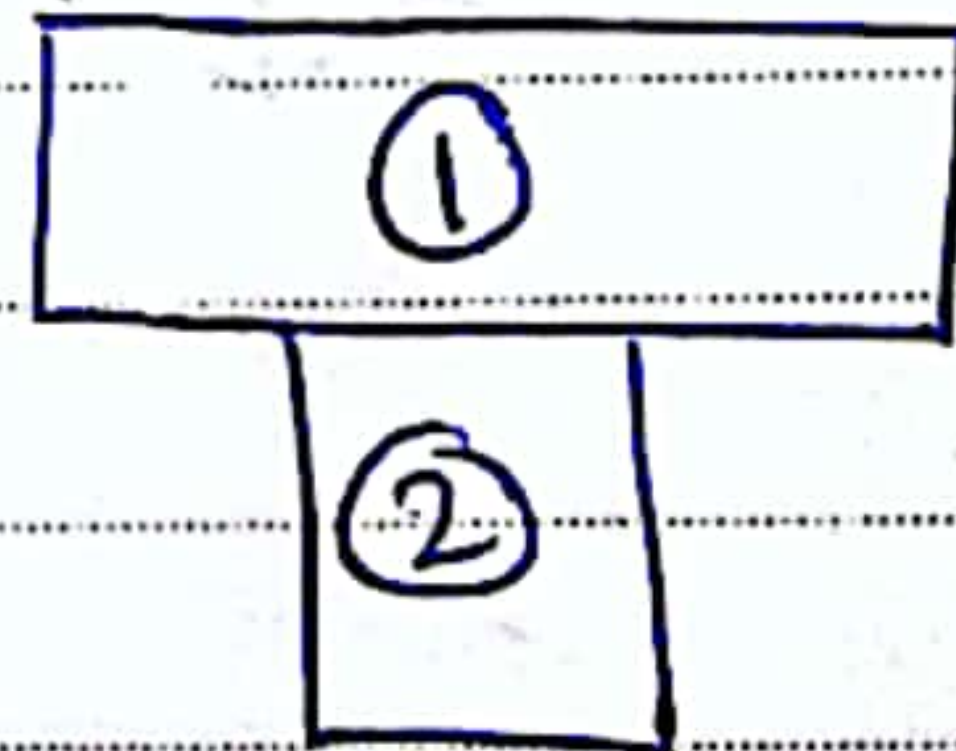
4.15) The beam shown is made of a nylon for which the allowable stress is 24 MPa in tension and 30 MPa in comp. Determine the largest couple M that can be applied to the beam



| | A [mm ²] | \bar{y}_0 [mm] | $A\bar{y}_0$ [mm ³] |
|----------|------------------------|------------------|---------------------------------|
| 1 | 600 | 22.5 | 13.5×10^3 |
| 2 | 300 | 7.5 | 2.25×10^3 |
| Σ | 900 | | 15.75×10^3 |

$$* \bar{y}_0 = \frac{15.75 \times 10^3}{900} = 17.5 \text{ mm}$$

* the neutral axis lies 17.5 mm above the bottom



$$y_{\text{top}} = 30 - 17.5 = 12.5 \text{ mm}$$

$$y_{\text{bot}} = -17.5 \text{ mm}$$

$$I_1 = \frac{b_1 h_1^3}{12} + A_1 d_1^2 = \frac{40 (15)^3}{12} + 600 (5)^2 = 26.25 \times 10^3 \text{ [mm}^4\text{]}$$

$$I_2 = \frac{20 (15)^3}{12} + 300 (10)^2 = 35.625 \times 10^3 \text{ mm}^4$$

$$I = I_1 + I_2 = 61.875 \times 10^3 \text{ [mm}^4\text{]}$$

Top σ

$$M = \frac{24 \times 10^6 \times 61.875 \times 10^{-9}}{0.0125} = 118.8 \text{ N}\cdot\text{m} \quad \text{--- tension side}$$

$$\text{Bottom } \sigma \quad M = \frac{30 \times 10^6 \times 61.875 \times 10^{-9}}{0.0175} = 106.1 \text{ N}\cdot\text{m} \quad \text{--- compression side}$$

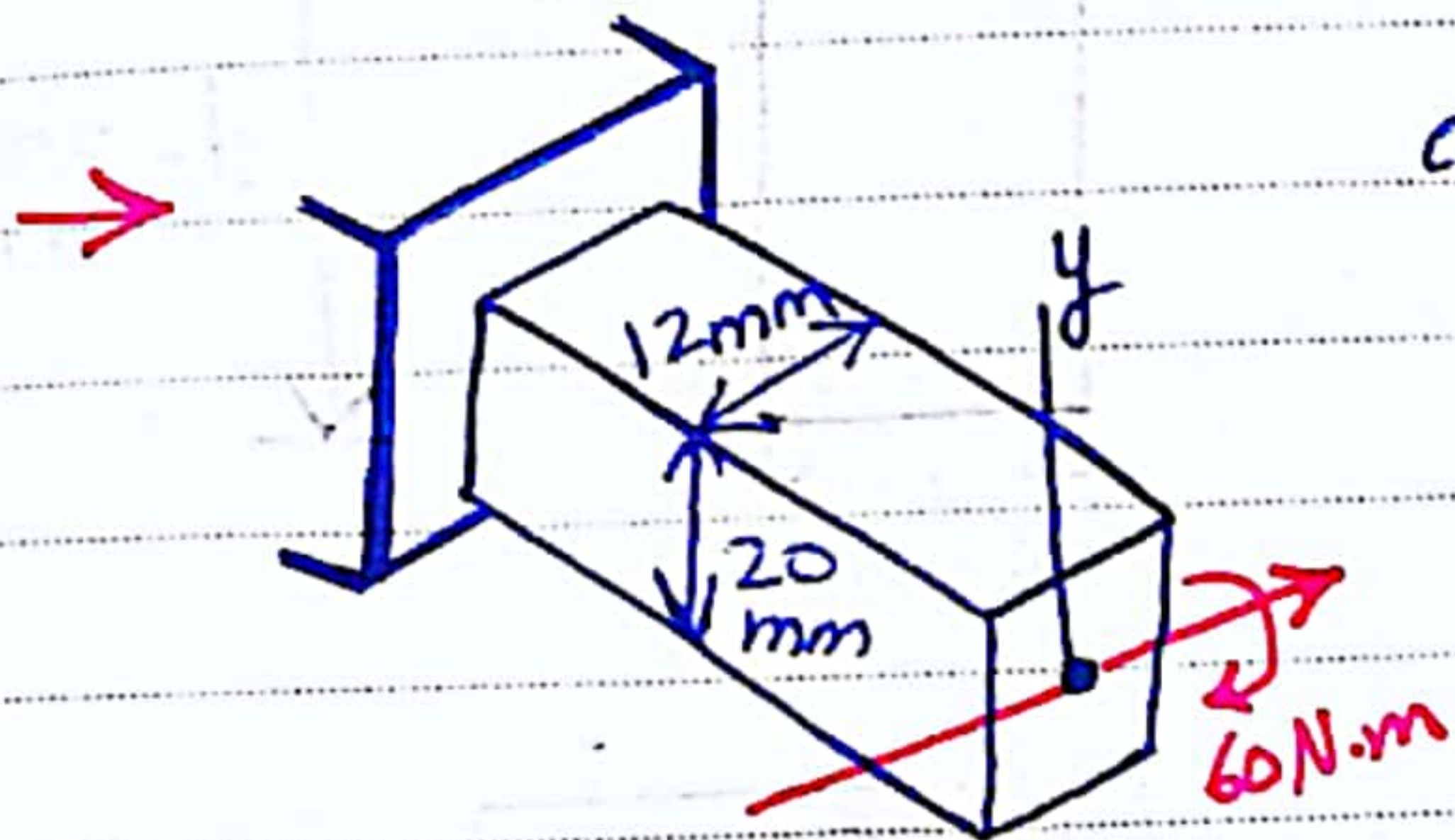
$$M = 106.1 \text{ N}\cdot\text{m}$$

→ the smallest value

4.24) A 60 N.m couple is applied to the steel bar shown.

a) Assuming that the couple is applied about the z-axis as shown determine the max. stress and the radius of curvature of the bar?

b) solve ρ , assuming that the couple is applied about the y-axis. $E = 200 \text{ GPa}$



a- Bending about z-axis →

$$\sigma = \frac{M \cdot c}{I} = \frac{60 \times \left(\frac{20}{2} \times 10^{-3}\right)}{\left[\frac{12(20)^3}{12} \times 10^{-12}\right]} = 75 \text{ MPa}$$

$$\frac{1}{\rho} = \frac{M}{EI} = \frac{60}{200 \times 10^9 \times 8 \times 10^{-9}} = 37.5 \times 10^{-3} \text{ m}^{-1}$$

$$\rho = 26.7 \text{ m}$$

b- Bending about y-axis →

$$\sigma = \frac{60 \times \left(12 \times \frac{1}{2} \times 10^{-3}\right)}{\frac{20(12)^3}{12} \times 10^{-12}} = 125 \text{ MPa}$$

$$\frac{1}{\rho} = \frac{60}{200 \times 10^9 \times 2.88 \times 10^{-9}} = 104.17 \times 10^{-3} \text{ m}^{-1}$$

$$\rho = 9.6 \text{ m}$$

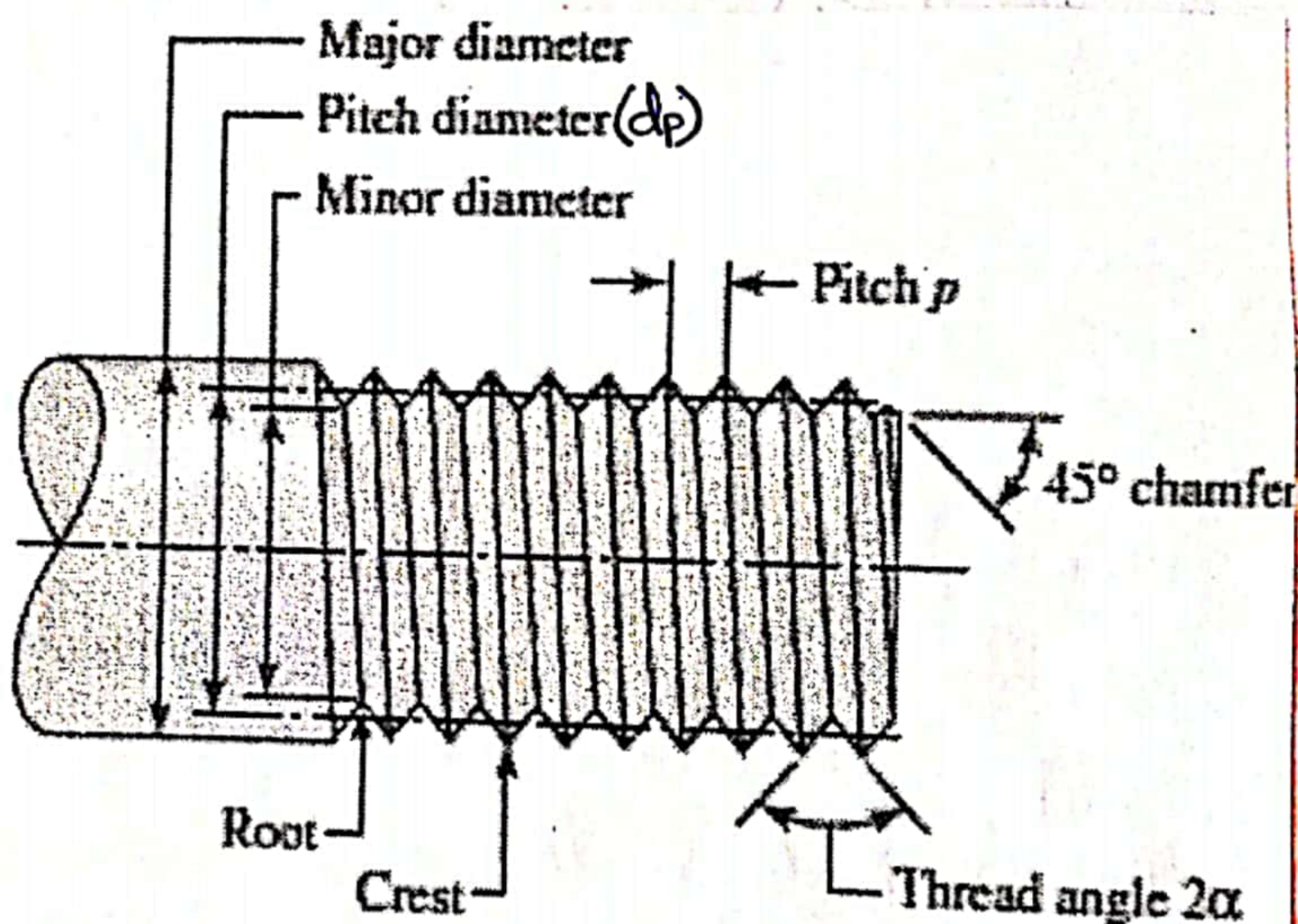
Ch.8 Screws, Fasteners, and the Design of Non permanent joints

Type of joints → permanent "welding"
 → Non permanent "Screws"

8.1 Thread Standards and Definitions

• The terminology of screw threads, illustrated in the figures below ∞

Sharp vee threads → shown for clarity; the crests and roots are actually flattened or rounded during the forming operation.



pitch \equiv distance between adjacent thread forms measured parallel to the thread axis.

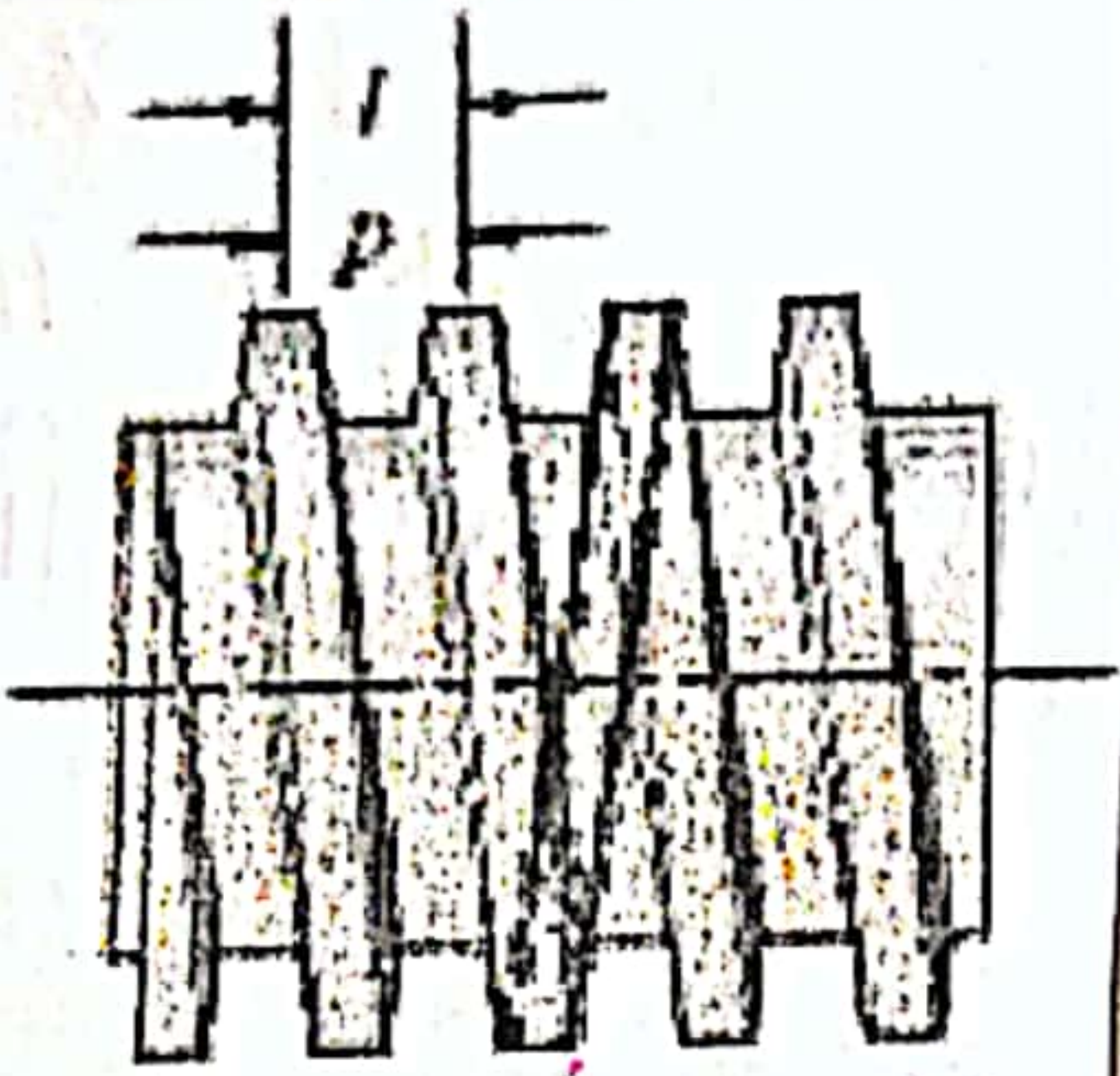
(d) major diameter \equiv the largest diameter of a screw thread.

(dr) minor diameter \equiv the smallest diameter of a screw thread.

pitch diameter \equiv is a theoretical diameter between the major and minor diameters.

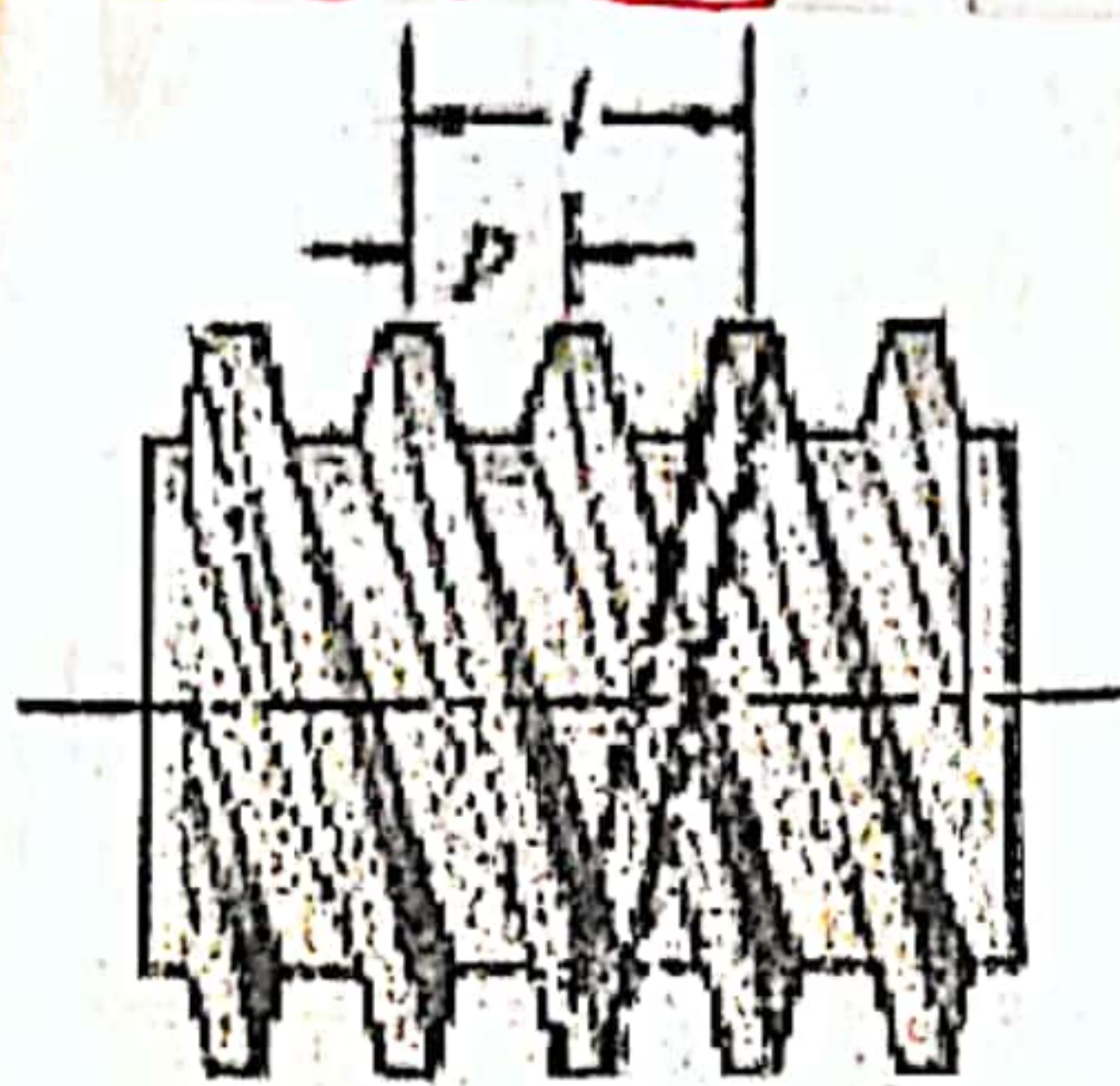
or
 (root diameter)

$$L = P$$



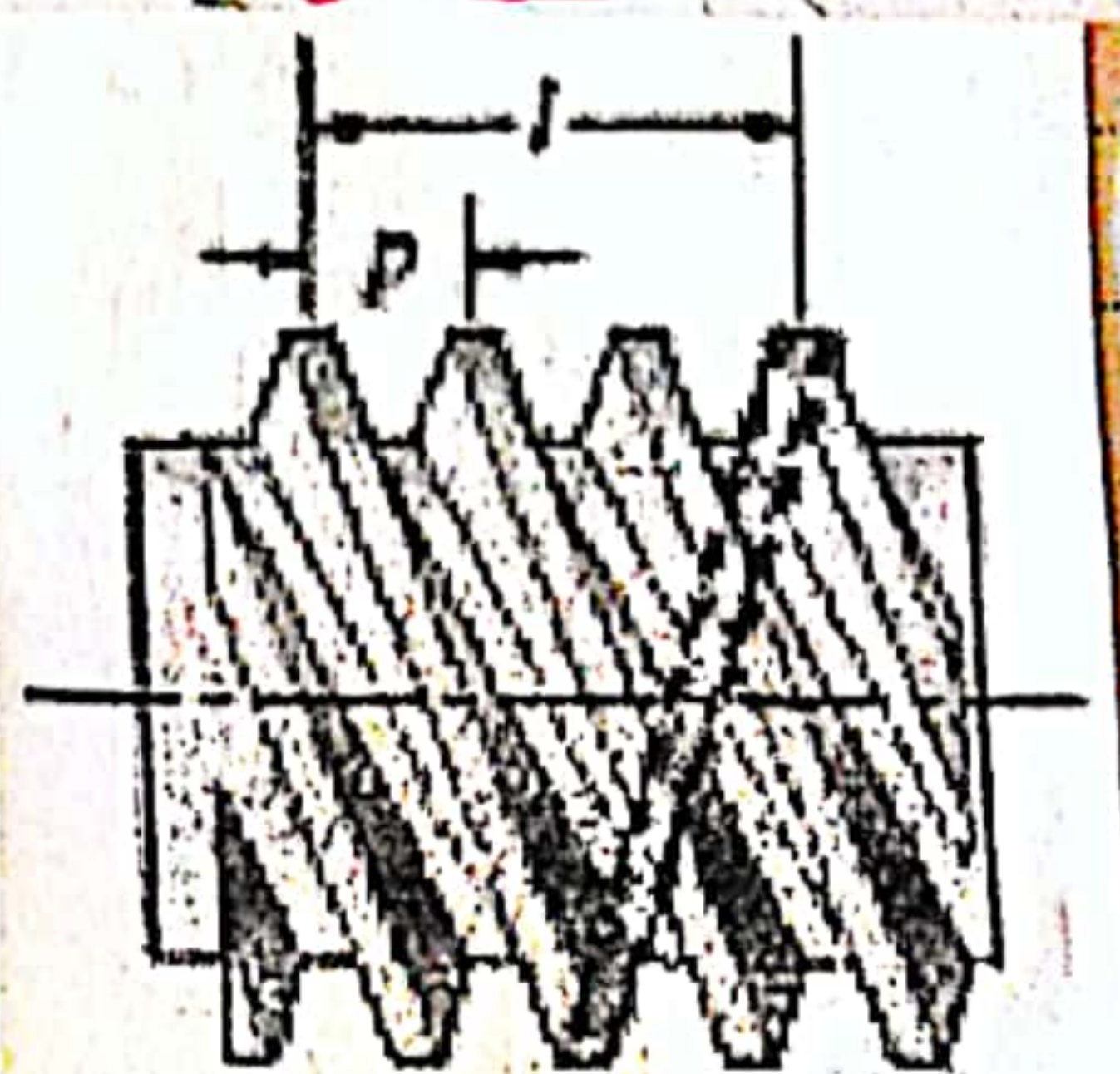
Single thread screw

$$L = 2P$$



Double thread screw

$$L = 3P$$



Three threads screw

Lead (L) \equiv the distance the nut moves parallel to the screw axis when the nut is given one turn.

الارتفاع بين الخيوط

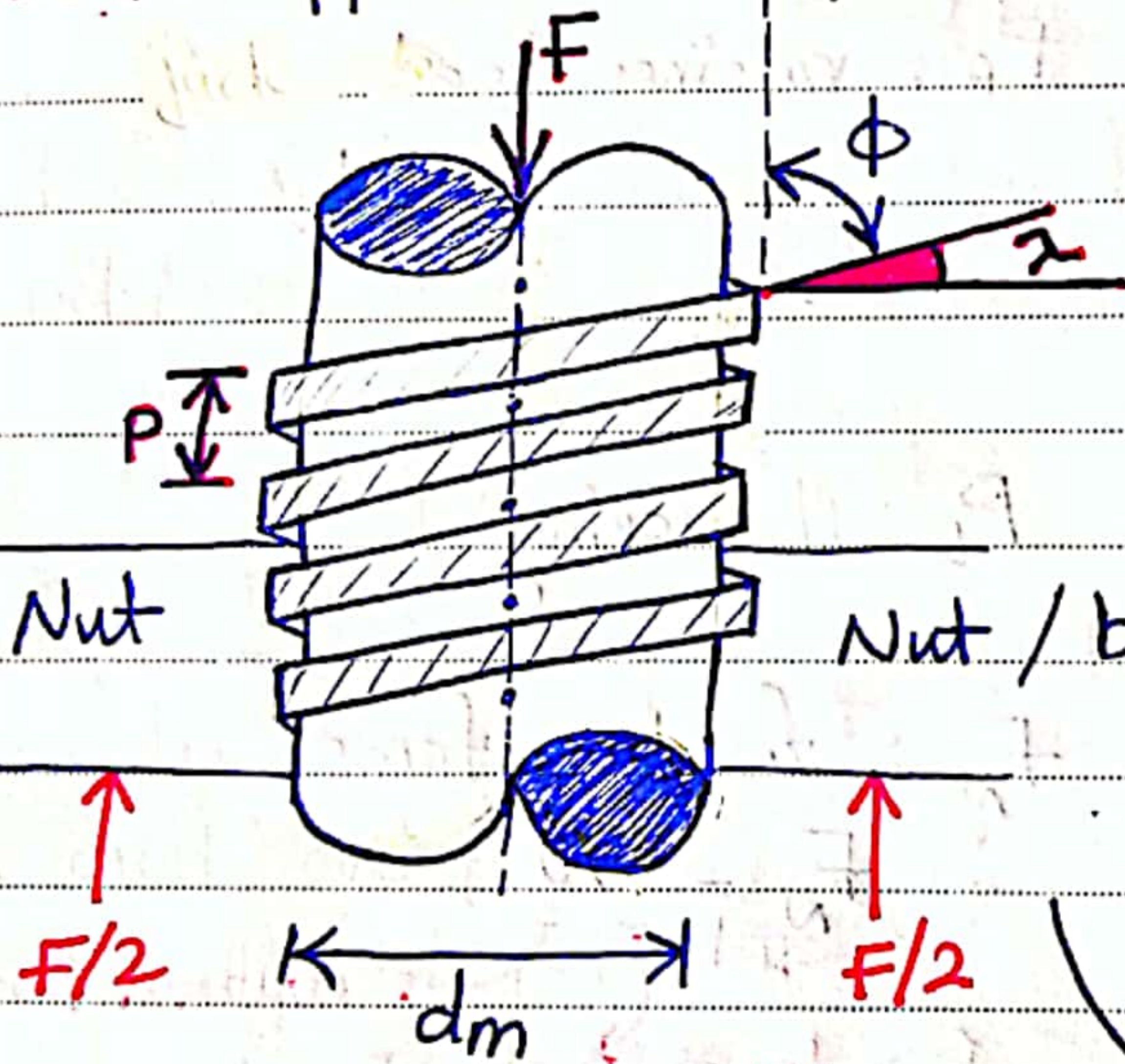
Tables (8-1) & (8-2) for specifying and designing threaded parts.

The thread size is specified by giving the pitch (P) for metric sizes and by giving the no. of threads per inch N . The screw sizes in (8-2) with diameter under $1/4$ in are numbered or gauge sizes. The second column in (8-2) shows that a No. 8 screw has a nominal major diameter of 0.1640 [in.]

8.2 | The Mechanics of Power Screws

Power Screw \equiv is a device used in machinery to change angular motion into linear motion, and, usually, to transmit power.

An application of power screws to a power-driven jack



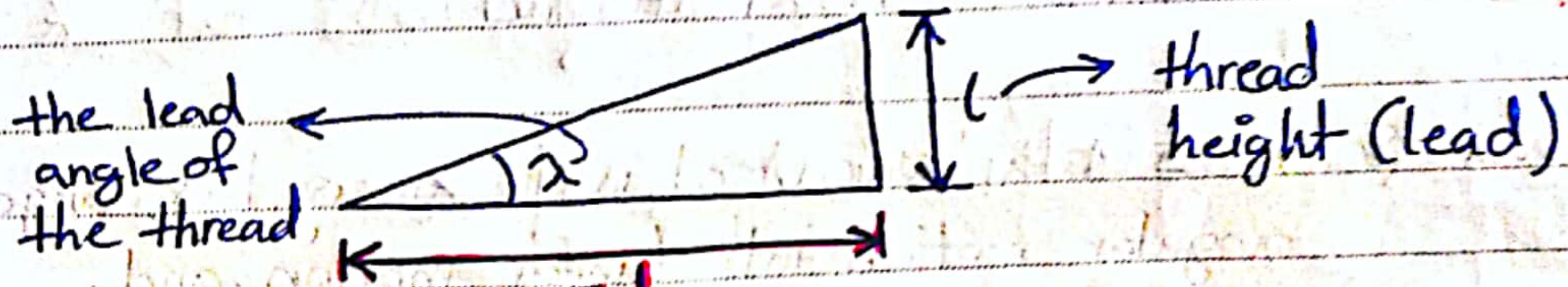
- F : Comp. axial force
- λ : Lead angle
- ϕ : helix angle
- d_m : mean diameter

جزء من الجسم
تاج ال Jack

النسبة مربع

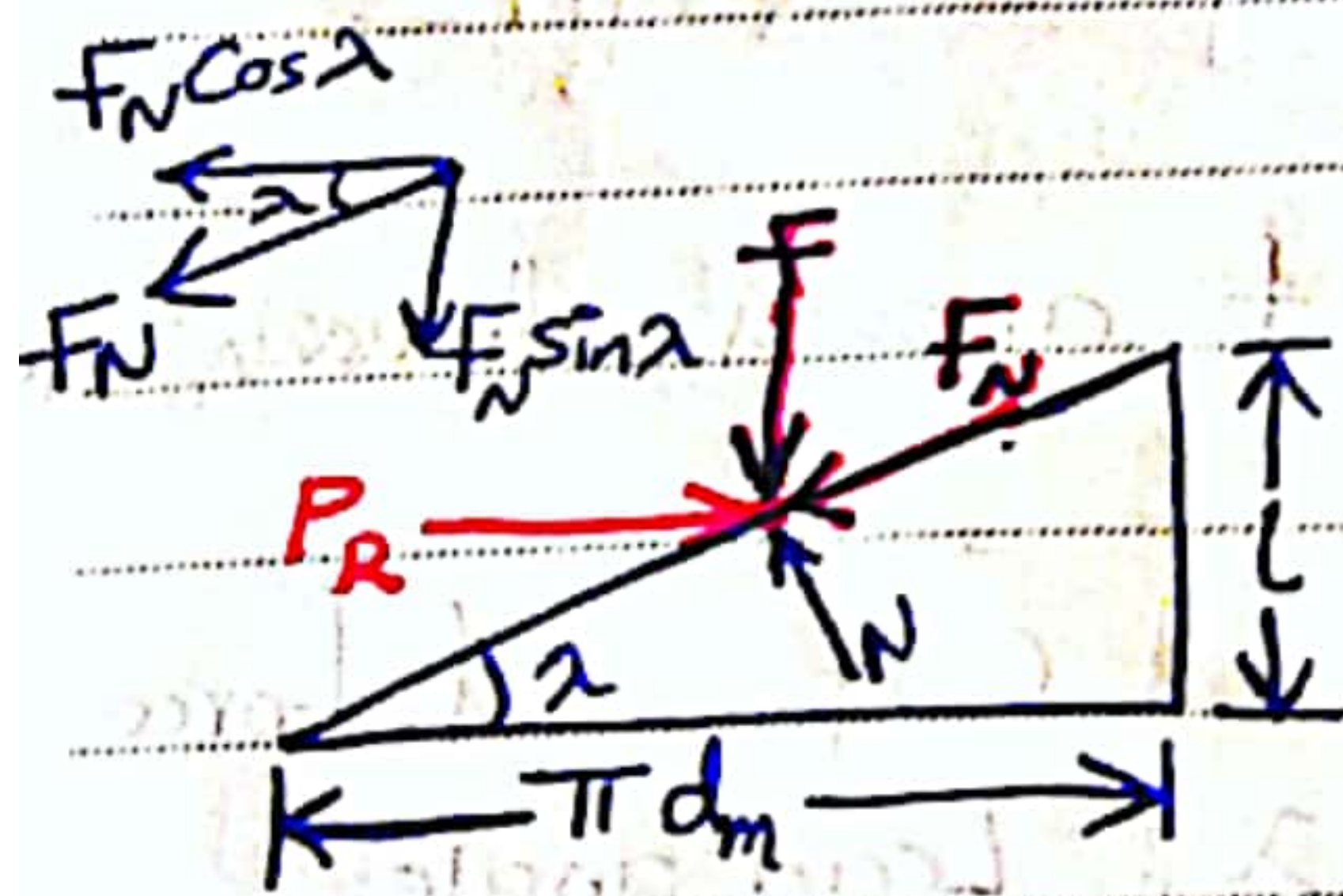
square threaded
power screw
with single thread

* بينا نوجد التورك الى رفع ختابة كتي
نرفع هاد اللود و التورك كتي ننزله
بعضه

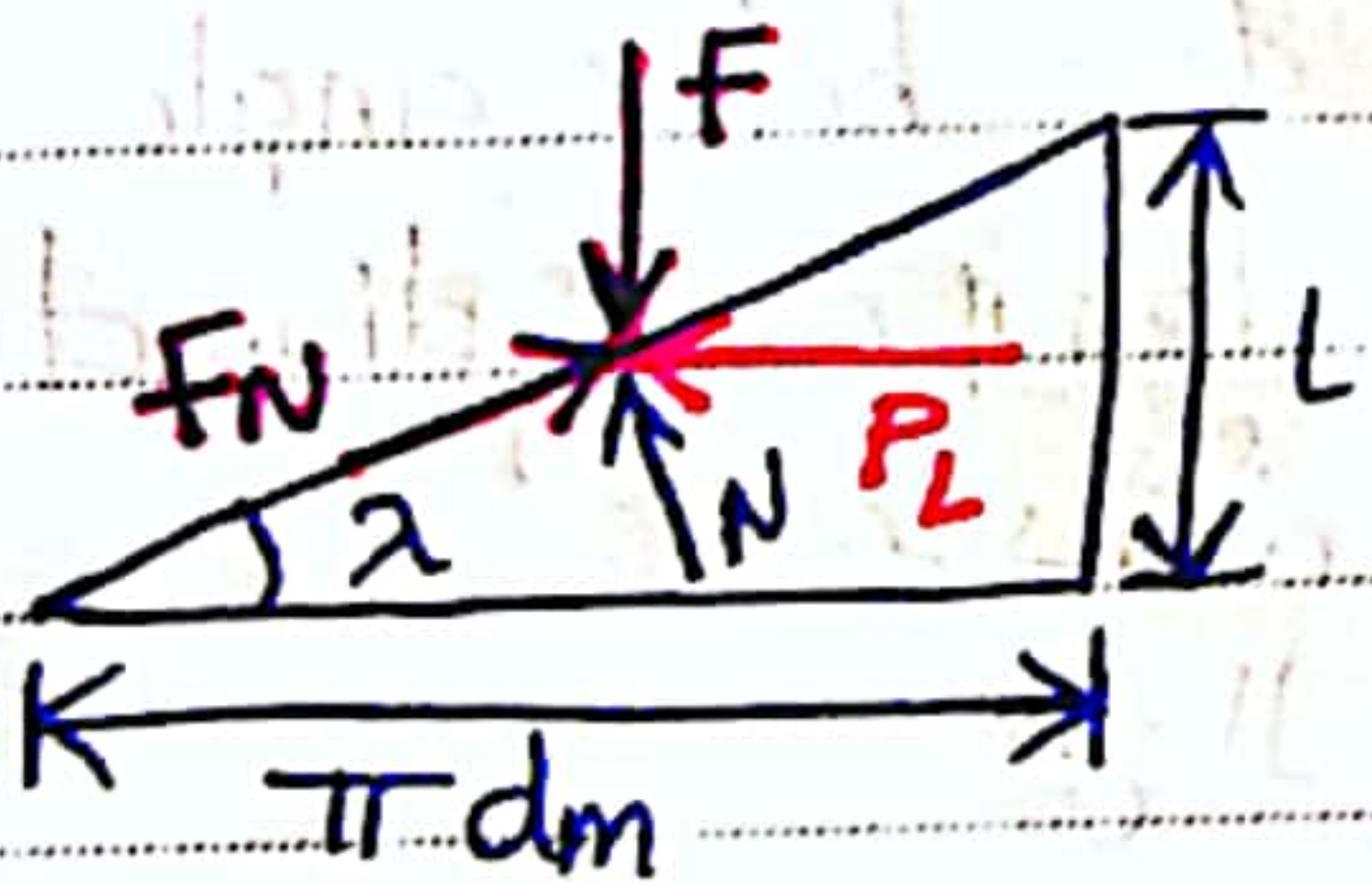


πd_m

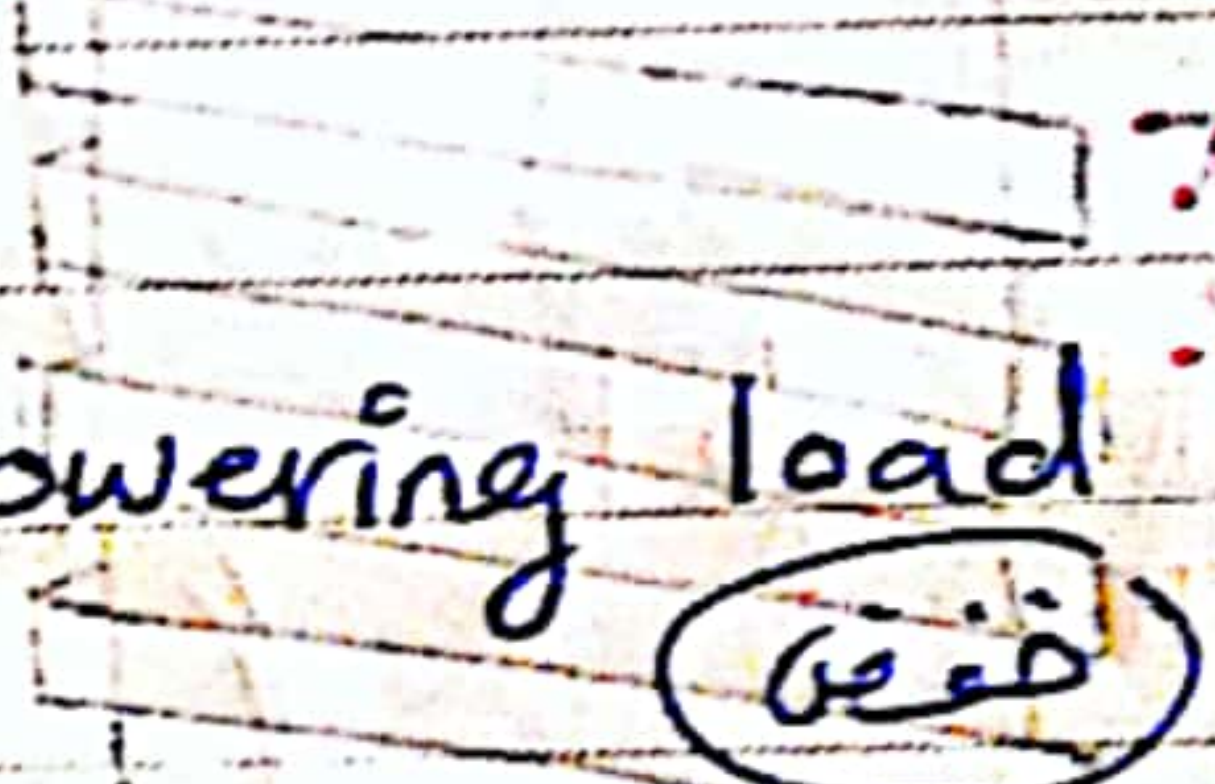
the circumference of the mean thread diameter circle



$\Rightarrow P_R$: raising load



$\Rightarrow P_L$: lowering load



F_N : friction force

$$F_N = fN$$

normal force

كسوف و ل

friction coefficient

* For raising the load :

$$\sum F_x = 0 \Rightarrow [P_R - N \sin \lambda - fN \cos \lambda = 0]$$

$$\sum F_y = 0 \Rightarrow [-F - fN \sin \lambda + N \cos \lambda = 0]$$

dividing P_R by F :

$$\frac{P_R}{F} = \frac{N \sin \lambda + f N \cos \lambda}{-f N \sin \lambda + N \cos \lambda} \Rightarrow P_R = F \frac{(\sin \lambda + f \cos \lambda)}{(\cos \lambda - f \sin \lambda)}$$

cos λ کی صورت میں

$$\text{Let } \tan \lambda = \frac{L}{\pi dm}$$

$$P_R = F \frac{\left(\frac{L}{\pi dm} + f \right)}{\left(1 - \frac{f L}{\pi dm} \right)}$$

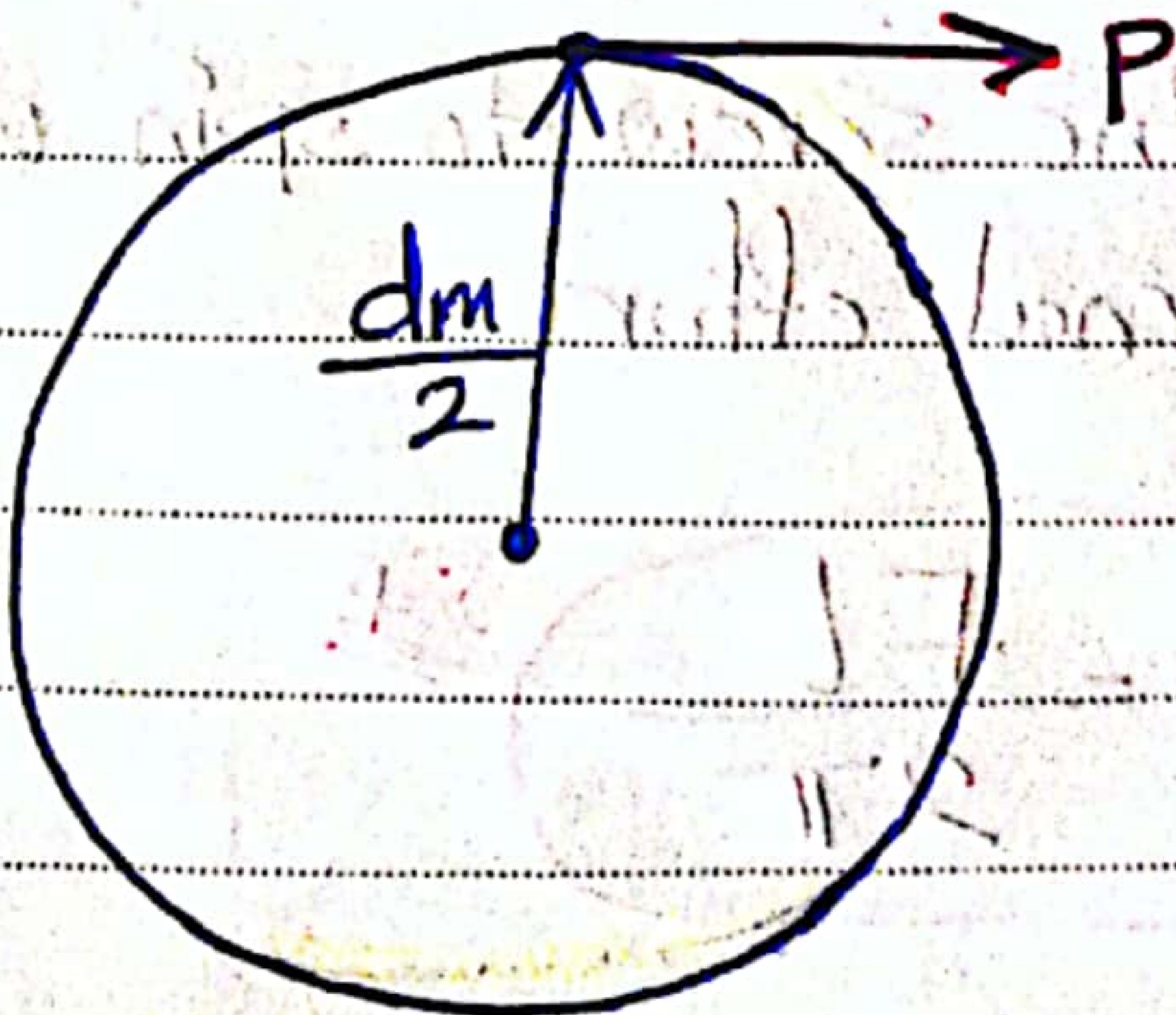
نیپ

f : دالہ
الاصول

• The torque for raising the load is

$$T_R = P \left(\frac{dm}{2} \right)$$

$$T_R = F dm \left(\frac{\pi f dm + L}{\pi dm - f L} \right)$$



* For Lowering the Load \circ

$$\sum F_x = 0 \Rightarrow [-P_L' + N \sin \alpha + fN \cos \alpha = 0]$$

$$\sum F_y = 0 \Rightarrow [-F + fN \sin \alpha + N \cos \alpha = 0]$$

$$P_L = F \frac{\left(f - \frac{L}{\pi d_m}\right)}{\left(1 + f \frac{L}{\pi d_m}\right)}$$

tip

the torque required to overcome a part of the friction in lowering the load.

$$T_L = F \frac{d_m}{2} \frac{\left(f - \frac{L}{\pi d_m}\right)}{\left(1 + f \frac{L}{\pi d_m}\right)} = F \frac{d_m}{2} \frac{(\pi f d_m - L)}{(\pi d_m + fL)}$$

$$\Rightarrow \text{If } \left\{ \pi f d_m > 1 \right\} \rightarrow f > \frac{L}{\pi d_m}$$

When a +ve torque obtained from the above equation

السكرو لا يسحب

$$f > \tan \alpha$$

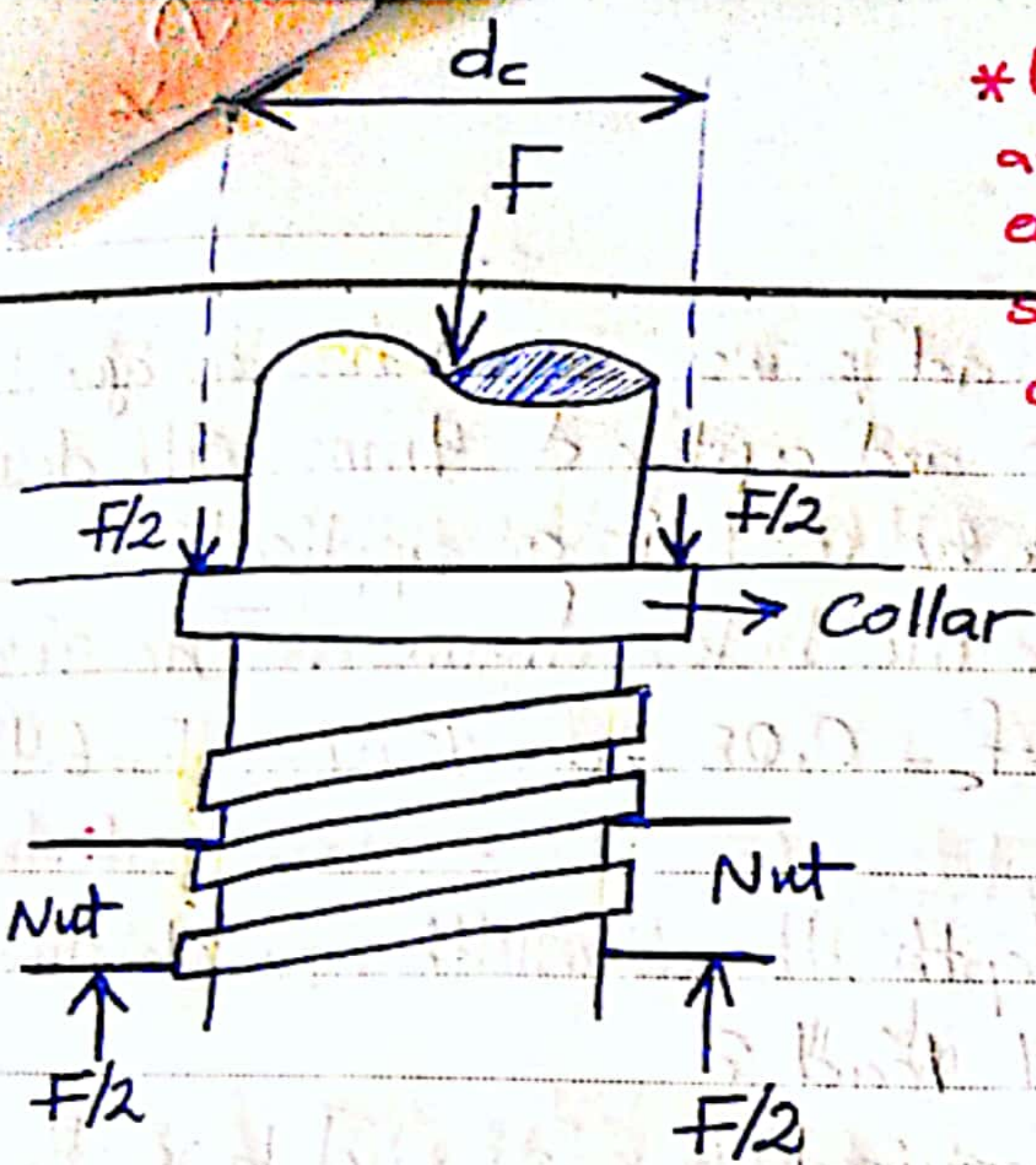
Self locking: the load will lower itself by causing the screw to spin without any external effort

$$\text{if } f = 0 \rightarrow T_0 = \frac{FL}{2\pi}$$

tip

$$\eta = \frac{T_0}{T_R} = \frac{FL}{2\pi T_R}$$

* When the screw is loaded axially, a thrust or collar bearing must be employed between the rotating and stationary members to carry the axial component..



f_c : the coefficient of collar friction
 d_c : the mean collar diameter

$$f_c \cdot d_c \Rightarrow T_c = \frac{F f_c d_c}{2}$$

then, $T_{total} = T_R + T_c$

or $T_{total} = T_L + T_c$

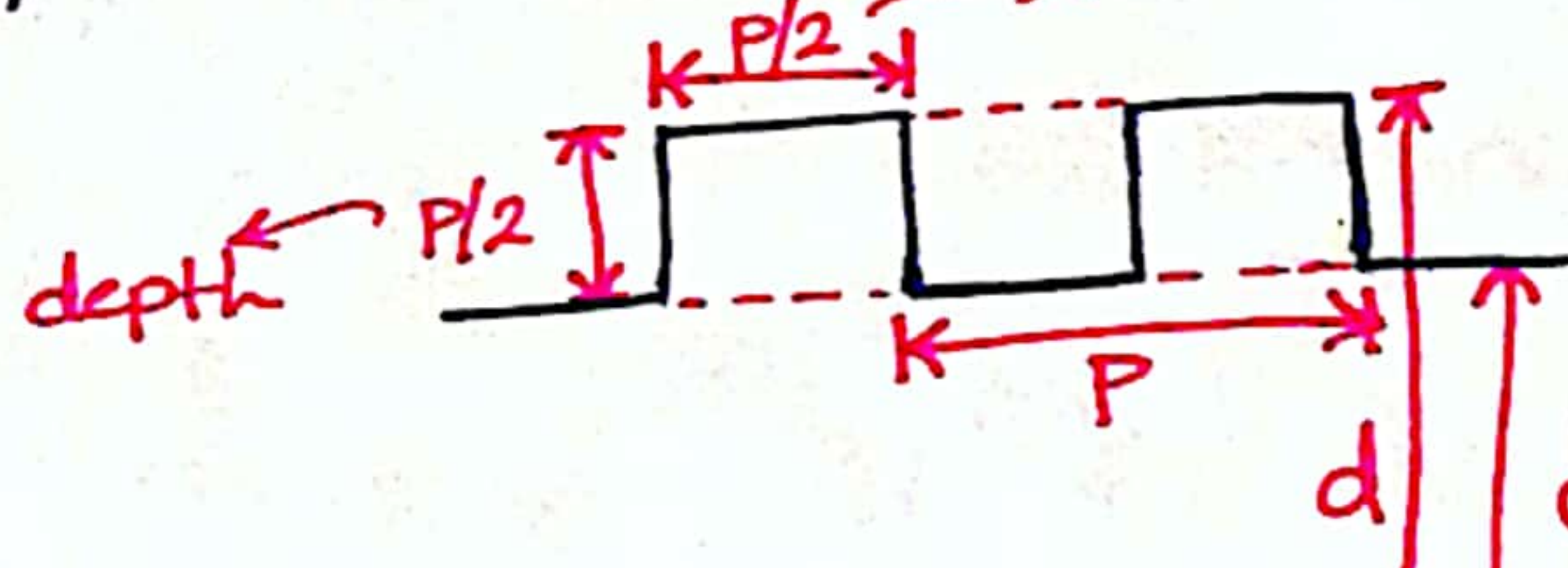
to calculate stresses for the above structure :-

$$\sigma_{normal} = \frac{F}{A} = \frac{4F}{\pi (dr)^2}$$

$$\tau_{max} = \frac{16 T}{\pi (dr)^3} \rightarrow \text{for a maximum shear stress in torsion of the screw body}$$

τ_{max}
direction

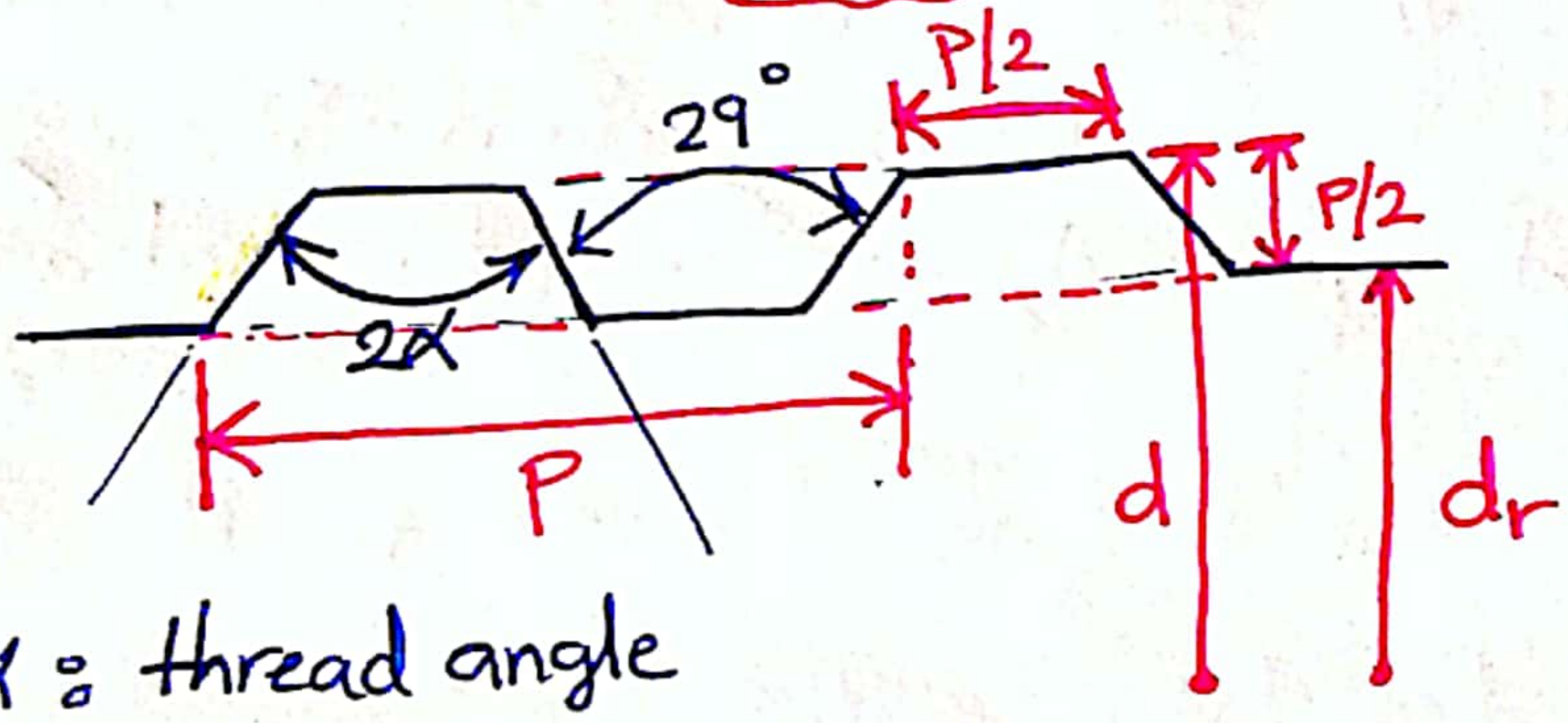
* Square Thread →



P : pitch
 $P/2$: width (depth)

* Acme Thread → the normal thread load is inclined to the axis because of the thread angle 2α and the lead angle λ

- For power screws, square better than Acme



2α : thread angle

... of the screw ...
 ... of the screw ...
 ... of the screw ...



Ex(8.1) A square thread power screw has a major diameter of 32 mm and a pitch of 4 mm with double threads, and it's to be used in an application similar to that in the below figure. The given data include $f = f_c = 0.08$, $d_c = 40$ mm, $F = 6.4$ kN per screw.

↳ mean collar diameter

- Find the thread depth, thread width, pitch diameter, minor diameter, and lead?
- Find the torque required to raise and lower the load?
- Find the efficiency during lifting the load?
- Find the body stresses, torsional and compressive?

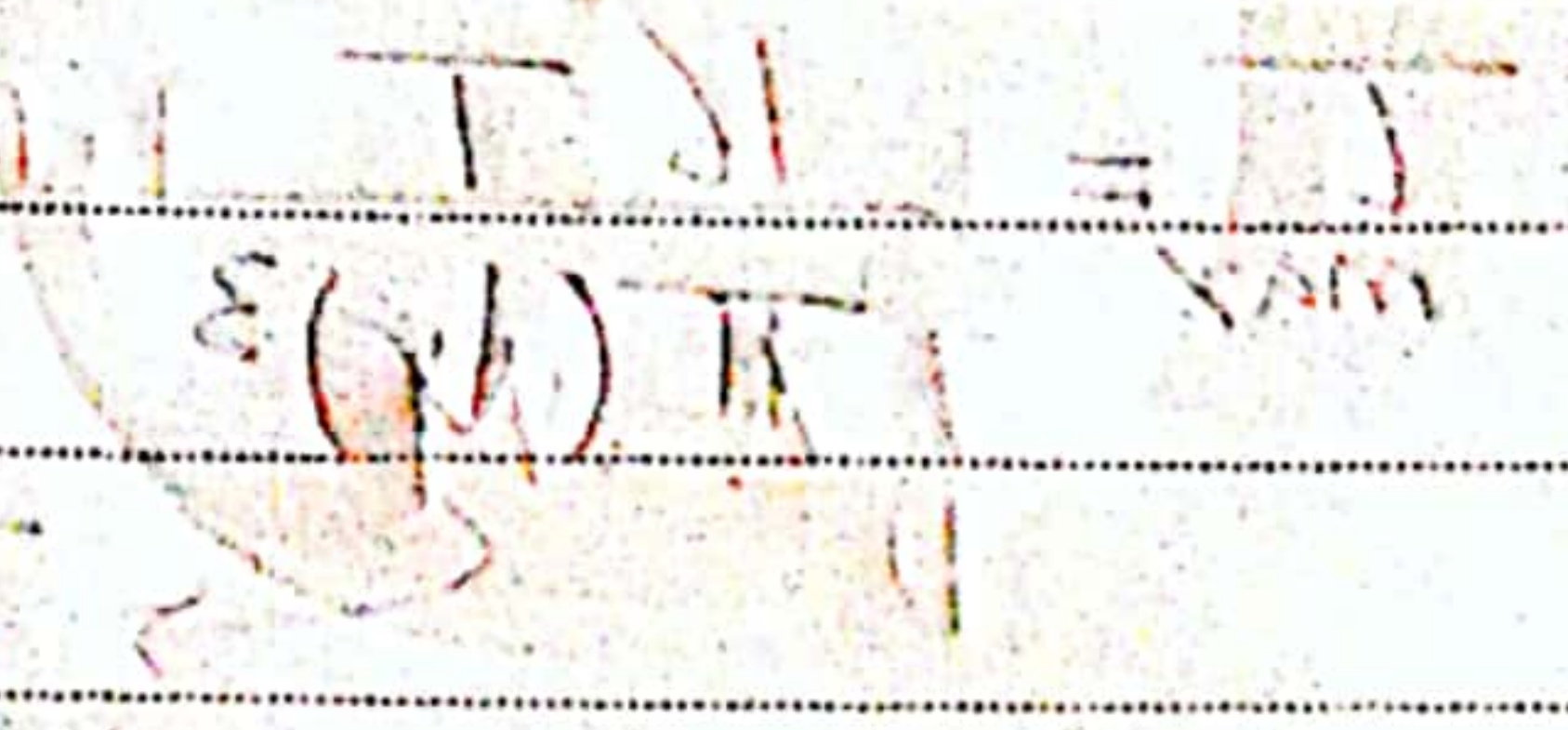
→ From the fig. shown previous for the square thread

a- mean diameter (d_m) = $d - (P/2) = 32 - \frac{4}{2} = 30$ mm

thread depth = thread width = $\frac{\text{Pitch}}{2} = \frac{4}{2} = 2$ mm

minor diameter (d_r) = $d - P = 32 - 4 = 28$ mm

Lead (L) = $n \cdot P = 2(4) = 8$ mm



b. the torque required to turn the screw against the load is :

$$T_R = \frac{F d_m}{2} \left(\frac{L + \pi f d_m}{\pi d_m - f L} \right) + \frac{F f_c d_c}{2} T_c$$

$$T_R = \frac{6.4(30)}{2} \left(\frac{8 + \pi(0.08)(30)}{\pi(30) - 0.08(8)} \right) + \frac{6.4(0.08)(40)}{2}$$

$$= 15.94 + 10.24 = 26.18 \text{ N}$$

the load - lowering load torque is :

$$T_L = \frac{F d_m}{2} \left(\frac{\pi f d_m - L}{\pi d_m + f L} \right) + \frac{F f_c d_c}{2}$$

$$= \frac{6.4(30)}{2} \left(\frac{\pi(0.08)(30) - 8}{30\pi + 0.08(8)} \right) + \frac{6.4(0.08)(40)}{2}$$

$$= -0.466 + 10.24$$

$$= 9.77 \text{ N.m}$$

indicates that the screw alone isn't self locking and would rotate under the action of the load except for the fact that the collar friction is present and must be overcome, too. Thus, the torque required to rotate the screw "with" the load < torque required to overcome collar friction alone.

C- The overall efficiency in raising the load is:

$$e = \frac{FL}{2\pi T_R} = \frac{6.4(8)}{2\pi(26.18)} = 0.311$$

d- The body shear stress (τ) due to torsional moment T_R at the outside of the screw body is:

$$\tau = \frac{16 T_R}{\pi d_r^3} = \frac{16 * 26.18 * 10^3}{\pi (28)^3} = 6.07 \text{ MPa}$$

$$\sigma_{\text{axial normal}} = \frac{-4F}{\pi d_r^2} = \frac{-4(6.4)(10^3)}{\pi (28)^2} = -10.39 \text{ MPa}$$

Bearing Stress Calculation:

$$\text{Bearing: } \sigma_B = \frac{-2F}{\pi d_m n_t P}, \quad n_t = \text{no. of engaged threads}$$

$$\text{bending: } \sigma_b = \frac{M}{Z} = \frac{6F}{\pi d_r n_t P} \rightarrow \text{the bending stress at the root of the thread}$$

$$M = \frac{FP}{4}$$

$$Z = \frac{\pi}{24} d_r n_t P^2$$

The transverse τ at the centre of the root of the thread due to load F is \propto

$$\tau = \frac{3V}{2A} = \frac{3F}{2\left(\frac{\pi d_r n_t P}{2}\right)}$$

$$\tau = \frac{3F}{\pi d_r n_t P}$$

and at the top of the root it's zero.

* The screw thread form is complicated from an analysis viewpoint.

A power screw lifting a load is in compression and its thread pitch is shortened by elastic deformation.

Its engaging nut is in tension and its thread pitch is lengthened.

The engaged threads can't share the load equally

* at $F \Rightarrow 0.38F$ (the first engaged thread)

$$n_t = 1$$

will give the largest level of stresses in the thread nut combination.

e) Find the bearing stress?

→ σ_B with one thread carrying $0.38F$

$$\sigma_B = \frac{-2(0.38F)}{\pi d_m (1) P} = \frac{-2(0.38)(6.4 \times 10^3)}{\pi (30)(1)(4)}$$

$$\sigma_B = -12.9 \text{ MPa}$$

f) Find the thread bending stress at the root of the thread?

$$\sigma_b = \frac{6(0.38F)}{\pi d_r (1) P} = \frac{6(0.38)(6.4 \times 10^3)}{\pi (28)(1)(4)}$$

$$\sigma_b = 41.5 \text{ MPa}$$

F for $n_t = 1 \rightarrow 0.38F$

$n_t = 2 \rightarrow 0.25F$

$n_t = 3 \rightarrow 0.18F$

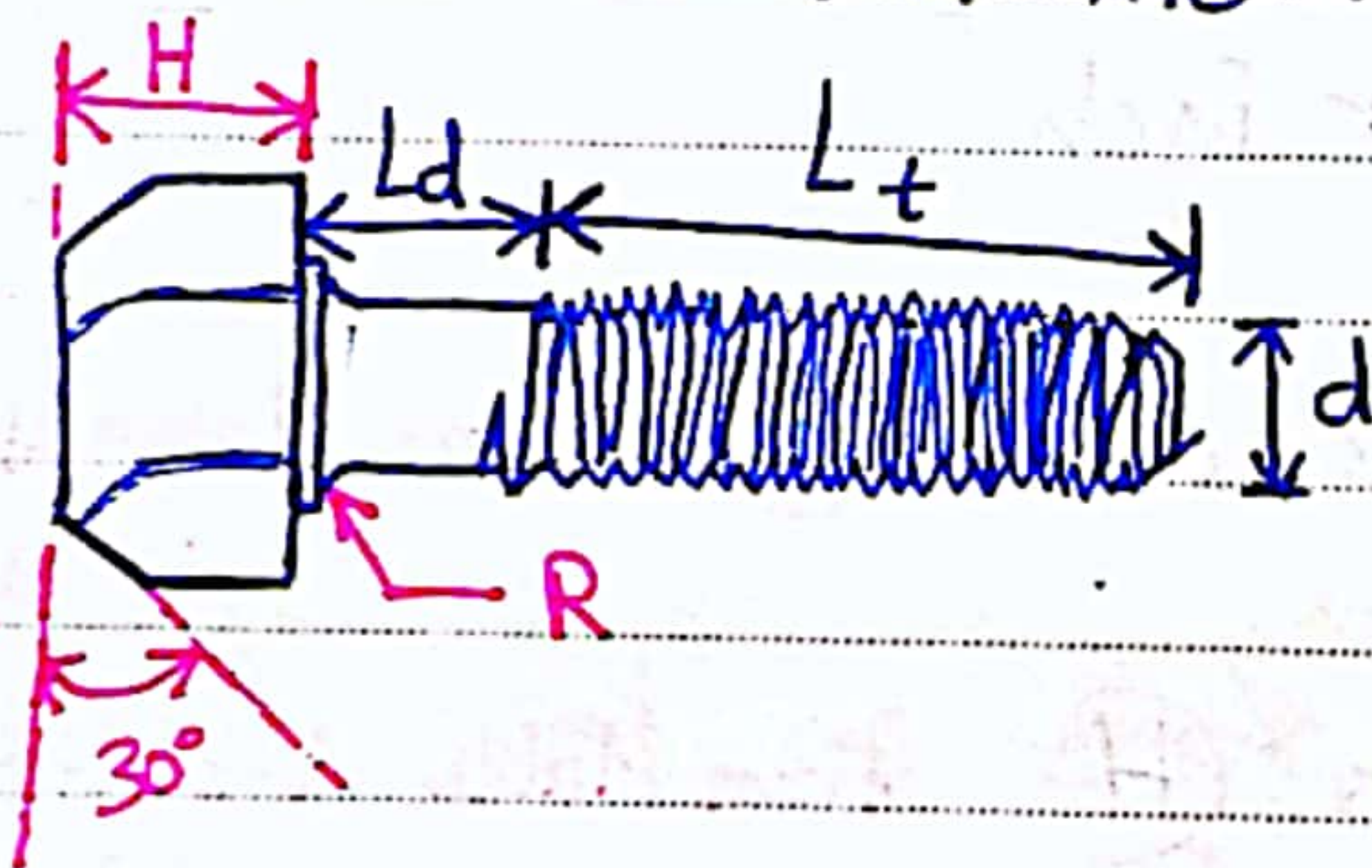
→ 3.30

8.3 Threaded Fasteners

The purpose of a bolt is to clamp 2 or more parts together. The clamping load stretches, or elongates the bolt; the load is obtained by twisting the nut until the bolt has elongated almost to the elastic limit.

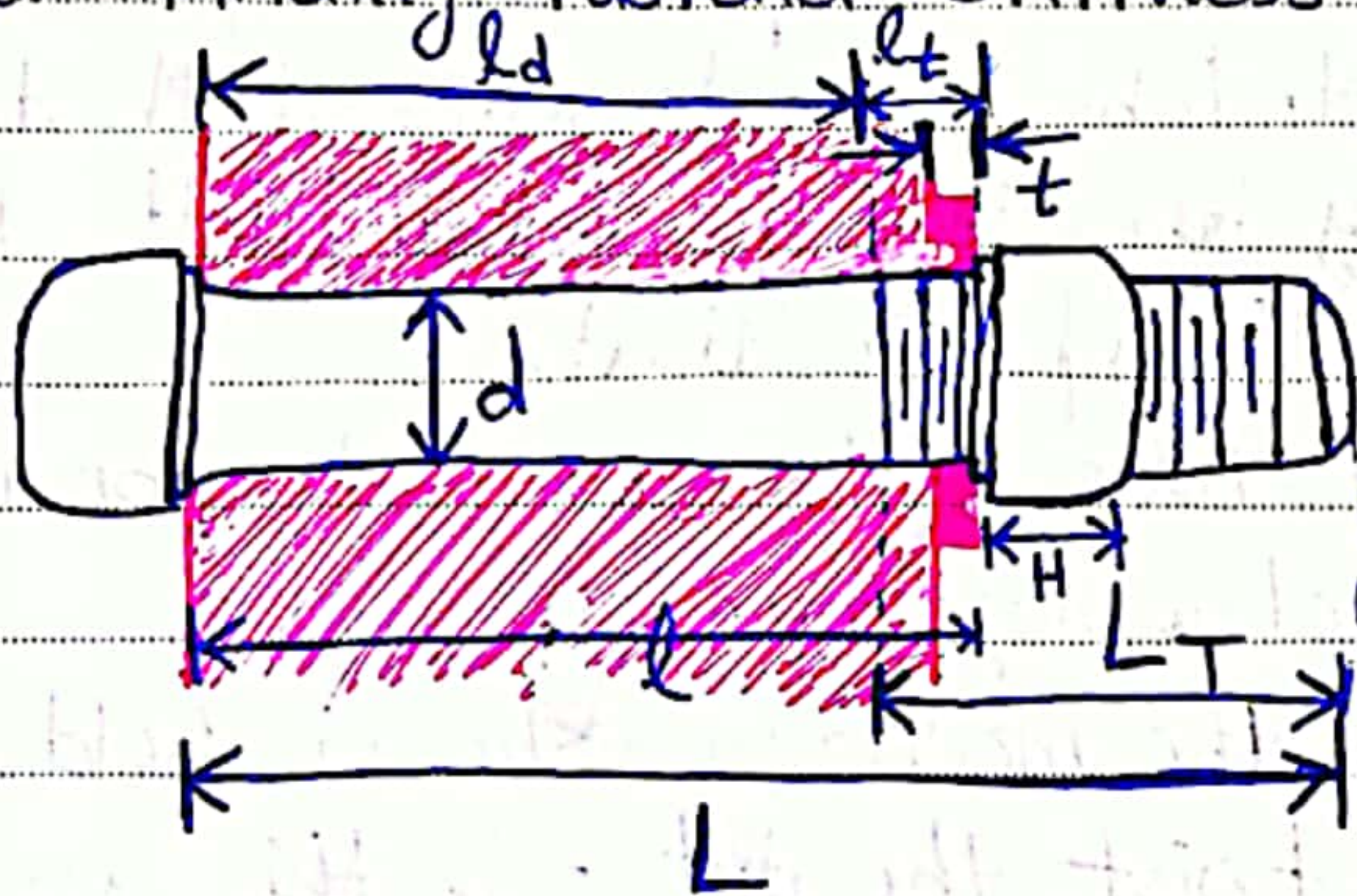
If the nut doesn't loosen, this bolt tension remains as the preload or clamping force.

When tightening, the mechanic should hold the bolt head stationary and twist the nut; in this way the bolt shank will not feel the thread-friction torque.



8.4 Joints - Fastener Stiffness

- For finding fastener stiffness \rightarrow



From
table
8-7
P.426

\Rightarrow Fig(a)

- Given fastener diameter d and pitch P in [mm] or no. of threads per inch

- Washer Thickness: (t) From table A-32 (P.1056) or A-33 (P.1057)
- Nut Thickness: (H) From table A-31 (P.1055)

- Fastener stiffness: $k_b = \frac{A_d A_t E}{A_d l_t + A_t l_d}$

where A_t : area of threaded portion

\hookrightarrow from table 8-1
8-2

A_d : area of unthreaded portion

$$A_d = \frac{\pi}{4} d^2$$

table(8-7) مودول و دین

- Length of unthreaded portion in grip (l_d)

$$l_d = L - L_T$$

- Length of threaded portion in grip (l_t)

$$l_t = L - l_d$$

- Threaded Length (L_T)

$$L_T = \begin{cases} 2d + 6\text{mm} & ; L \leq 125\text{mm}, d \leq 48\text{mm} \\ 2d + 12\text{mm} & ; 125 < L \leq 200\text{mm} \\ 2d + 25\text{mm} & ; L > 200\text{mm} \end{cases}$$

- Fastener length (L)

Fig(a)

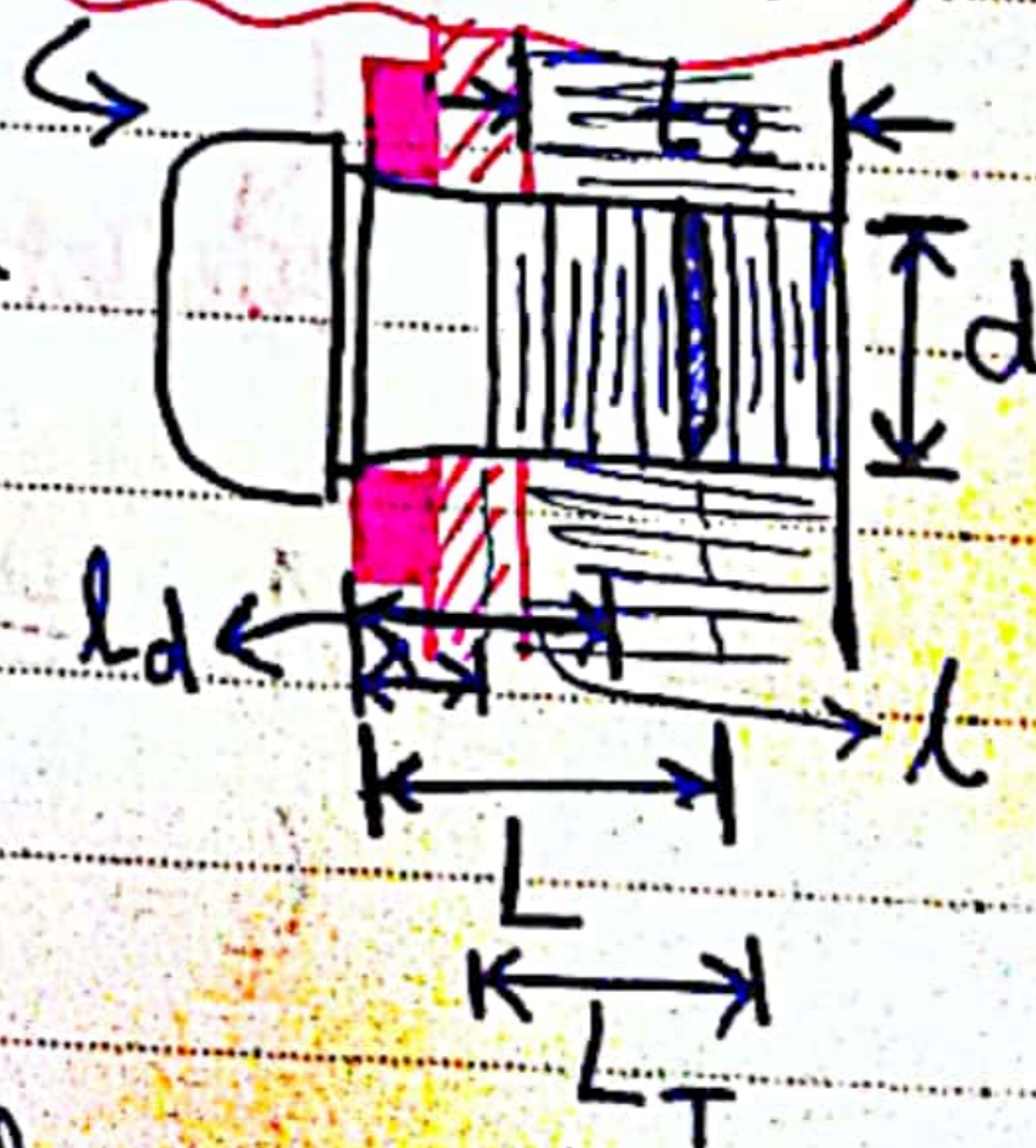
$$L > l + H$$

or

$$L > h + 1.5d$$

Fig(b)

Fig(b) \Rightarrow



- Grip length (l)

Fig(a) $\Rightarrow l \equiv$ thickness of all material squeezed between face of bolt and face of nut.

$$Fig(b) \Rightarrow l = \begin{cases} h + t_2/2 & ; t_2 < d \\ h + d/2 & ; t_2 \geq d \end{cases}$$

- The stiffness of the portion of a bolt or screw within the clamped zone will generally consist of 2 parts, that of the unthreaded shank portion and that of the threaded portion.

Thus, the stiffness constant of the bolt is equivalent to the stiffnesses of 2 springs in series.

$$\frac{1}{k} = \frac{1}{k_1} + \frac{1}{k_2}$$

$$k = \frac{k_1 k_2}{k_1 + k_2}$$

- The spring rates of the threaded and unthreaded portions of the bolt in the clamped zone are :-

$$k_t = \frac{A_t E}{l_t} \quad ; \quad k_d = \frac{A_d E}{l_d}$$

↓ threaded
↓ unthreaded

where ; $A_t \equiv$ tensile stress Area

$l_t \equiv$ length of threaded portion of grip

$A_d \equiv$ major diameter area of fastener

$l_d \equiv$ length of unthreaded portion in grip

$$\text{so, } k_b = \frac{A_d A_t E}{A_d l_t + A_t l_d}$$

$k_b \equiv$ estimated effective stiffness of the bolt or cap screw in the clamped zone.

Problems

8.11) An M14 x 2 hex-head bolt with a nut is used to clamp together two 15 mm steel plates.

- a- determine a suitable length for the bolt, rounded up to the nearest 5 mm
 b- determine the bolt stiffness

→ (a) From table A-31 → $H = 12.8$

$$L = l + H$$

$$= 2(15) + 12.8 = 42.8 \text{ mm} \approx 45 \text{ mm}$$

لأنه السؤال لم يذكر

قريباً لعينه أقرب 5 mm

(b) $k_b = \frac{A_d A_t E}{A_d l_t + A_t l_d}$

$$l_d = L - L_T = 45 - 34 = 11 \text{ mm}$$

$$l_t = l - l_d = 2(15) - 11 = 19 \text{ mm}$$

$$A_d = \frac{\pi}{4} d^2 = \frac{\pi}{4} (14^2) = 153.9 \text{ mm}^2$$

$$A_t = 115 \text{ mm}^2 \rightarrow \text{From table 8-1}$$

$$E = 207 \text{ GPa} \rightarrow \text{From table 8-8}$$

$$k_b = \frac{(153.9)(115)(207)}{(153.9)(19) + (115)(11)}$$

$$= 874.6 \text{ MN/m}$$

8.12) Repeat the previous question with the addition of one 14R metric plain washer under the nut?

$$\rightarrow \text{From table A-31} \rightarrow H = 12.8 \text{ mm}$$

$$L = l + H + h \quad \leftarrow \text{nut thickness}$$

$$= 2(15) + 12.8 + 3.5$$

$$= 46.3 \text{ mm}$$

$$\approx 50 \text{ mm}$$

$$L_T = 2(14) + 6 = 34 \text{ mm} \quad \bullet \quad L < 125$$

$$50 < 125$$

$$l_d = L - L_T = 50 - 34 = 16 \text{ mm}$$

$$2d + 5 \text{ mm}$$

$$l_t = l - l_d = 30 - 16 = 14 \text{ mm}$$

$$\text{So} \Rightarrow k_b = 808.2 \text{ MN/m}$$